CCR Groundwater Monitoring System Report, Revision 1

ECOM

Laramie River Station Wheatland, Wyoming

Basin Electric Power Cooperative

Project number: 60506860

January 16, 2025

Quality information

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Revision History

Revision 1	1/16/2025	Updated well Network		Jeremy Hurshman	Project Manager
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Table of Contents

1.	Introdu	ction	1
2.	Backgr	ound	2
3.	Geolog	ical and Hydrogeological Setting	3
4.	Monitor	ing Well System Selection and Installation	4
	4.1	Monitoring Well Installation in 2016	4
	4.2	Drilling and Well Construction	4
	4.3	Well Development	4
	4.4	Aquifer Testing	5
	4.5	Slug Test Analysis	5
	4.6	Pumping Test Analysis	6
	4.7	Addition of Existing Monitoring Wells in November 2016	6
	4.8	Addition of New and Existing Monitoring Wells in 2017	7
	4.9	Addition of New Monitoring Wells in 2019	7
	4.10	Replacement of Existing Monitoring Wells in 2024	8
	4.11	Recovery Well Installation	8
5.	System	Evaluation1	0
6.	Statistic	cal Methodology1	2
	6.1	Regulatory Guidance	2
	6.2	Statistical Analysis Approach1	3
	6.3	Interwell Statistical Approach	3
	6.4	Proposed Statistical Methods for Appendix III Analytes1	5
7.	Limitati	ons 1	6
8.	Referer	nces1	7

Figures

Figure 1-1	Site Location Map
Figure 4-1	LRS CCR Monitoring Well Network
Figure 5-1	Potentiometric Surface Map – June 4-5, 2024
Figure 5-2	Potentiometric Surface Map – September 26-27, 2024

Tables

Table 4-1	Monitoring Well Construction Details
Table 4-2	Aquifer Test Results
Table 5-1	Groundwater Elevations – June 4-5, 2024
Table 5-2	Groundwater Elevations – September 26-27, 2024
Table 6-1	Proposed Statistical Methods for Appendix III Constituents in Background Wells

Appendices

Appendix A – Boring Logs and Well Construction Diagrams

Appendix B – Aquifer Test Procedures, Data and Analysis

List of Acronyms

AECOM	AECOM Technical Services, Inc.				
ANOVA	analysis of variance				
Basin	Basin Electric Power Cooperative				
bgs	below ground surface				
CCR	Coal Combustion Residuals				
CFR	Code of Federal Regulations				
EPA	United States Environmental Protection Agency				
FGD	Flue Gas Desulfurization				
ft	feet				
ft/d	feet per day				
ID	internal diameter				
LRS	Laramie River Station				
MW	megawatt				
PVC	polyvinyl chloride				
RCRA	Resource Conservation and Recovery Act				
SSI	statistically significant increase				
SU	standard units				
U.S.	United States				
USGS	U.S. Geological Survey				

Monitoring System Certification

Basin Electric Power Cooperative Laramie River Station CCR units: Ash Pond 1, Ash Pond 2, Ash Pond 3, Ash Landfill, Emergency Holding Ponds

AECOM Technical Services, Inc. ("Consultant") was retained by Basin Electric Power Cooperative to prepare the attached documentation of the updated groundwater monitoring systems for the above-referenced coal combustion residuals ("CCR") surface impoundments and landfill, and to certify whether they meet the design and construction requirements set out in the CCR Rule under 40 Code of Federal Regulations (CFR) § 257.91. Some of the systems have been modified since the original certification dated October 17, 2017 to account for the results of groundwater monitoring and corrective action at the Site. This current certification applies to the systems as they exist as of December 31, 2024.

Background

Pursuant to 40 CFR § 257.90(b), owners and operators of new and existing CCR landfills, and new and existing CCR surface impoundments, and all lateral expansions of a CCR unit must install a groundwater monitoring system, compliant with 40 CFR § 257.91, which requires that said system consist of a sufficient number of wells, installed at appropriate locations and depths, to yield groundwater samples from the uppermost aquifer that accurately represent the quality of background groundwater that has not been affected by leakage from the CCR unit and accurately represent the quality of groundwater passing the waste boundary of the CCR unit.

Pursuant to 40 CFR § 257.91(f), the owner or operator must obtain a certification from a qualified professional engineer stating that the groundwater monitoring system has been designed and constructed to meet the requirements of 40 CFR § 257.91, including the performance standards specified in 40 CFR § 257.91(a), based on the site-specific information specified in 40 CFR § 257.91(b). If the groundwater monitoring system includes only the minimum number of monitoring wells specified in 40 CFR § 257.91(c)(1), the certification must document the basis supporting this determination. In support of Consultant's assessment, Consultant completed an evaluation of the groundwater monitoring system for the above-referenced CCR units and determined that sufficient information is available to make the certification required under 40 CFR § 257.91(f).

Limitations

The signature of Consultant's authorized representative on this document represents that to the best of Consultant's knowledge, information, and belief in the exercise of its professional judgment, it is Consultant's professional opinion that the aforementioned information is accurate as of the date of such signature. Any opinion or decisions by Consultant are made on the basis of Consultant's experience, qualifications, and professional judgment and are not to be construed as warranties or guaranties. In addition, opinions relating to environmental, geologic, and geotechnical conditions or other estimates are based on available data, and actual conditions may vary from those

encountered at the times and locations where data are obtained, despite the use of due care.

Certification

I, Patrick Clem, being a Registered Professional Engineer in the State of Wyoming, certify to the best of my knowledge, information, and belief, that the groundwater monitoring system for the CCR units that are the subject of this certification has been designed and constructed to meet the requirements of 40 CFR § 257.91, and that this certification is true and correct and has been prepared in accordance with generally accepted good engineering practices.

SIGNATURE: Intrick M. Clem

DATE: 01.16,2025



Statistical Method Certification

Basin Electric Power Cooperative Laramie River Station CCR units: Ash Pond 1, Ash Pond 2, Ash Pond 3, Ash Landfill, Emergency Holding Ponds

AECOM Technical Services, Inc. ("Consultant") was retained by Basin Electric Power Cooperative to prepare the attached documentation of the groundwater monitoring systems for the above-referenced coal combustion residuals ("CCR") surface impoundments and landfill, and to certify the appropriate statistical methods for evaluation of the monitoring systems data. Some of the groundwater monitoring systems have been modified and significant groundwater monitoring data have been generated and evaluated since the original certification dated October 17, 2017. Accordingly, the statistical methods were re-evaluated to account for the results of these modifications and monitoring. This current certification applies to the statistical methods selected for use as of December 31, 2024.

Background

Pursuant to 40 Code of Federal Regulations (CFR) § 257.90(b), owners and operators of new and existing CCR landfills, and new and existing CCR surface impoundments, and all lateral expansions of a CCR unit must install a groundwater monitoring system, compliant with 40 CFR § 257.91, which requires that said system consist of a sufficient number of wells, installed at appropriate locations and depths, to yield groundwater samples from the uppermost aquifer that accurately represent the quality of background groundwater that has not been affected by leakage from the CCR unit and accurately represent the quality of groundwater passing the waste boundary of the CCR unit.

Pursuant to 40 CFR § 257.93(f), the owner or operator of the CCR unit must select one of the statistical methods specified in paragraphs (f)(1) through (5) of this section to be used in evaluating groundwater monitoring data for each specified constituent. The statistical test chosen shall be conducted separately for each constituent in each monitoring well, and shall comply with the performance standards specified in 40 C.F.R. § 257.93(g). Per 40 CFR § 257.93(f)(6), the owner or operator must obtain a certification from a qualified professional engineer stating that the statistical method for the evaluation of groundwater monitoring data for the groundwater monitoring system meets the requirements of 40 CFR § 257.93(f)(6), including the performance standards specified in 40 CFR § 257.91(a), based on the site-specific information specified in 40 CFR § 257.91(b). In support of Consultant's assessment, Consultant completed an evaluation of the groundwater monitoring system for the above-referenced CCR units and determined that sufficient information is available to make the certification required under 40 CFR § 257.93(f).

Limitations

The signature of Consultant's authorized representative on this document represents that to the best of Consultant's knowledge, information, and belief in the exercise of its professional judgment, it is Consultant's professional opinion that the aforementioned information is accurate as of the date of such signature. Any opinion or decisions by Consultant are made on the basis of Consultant's experience, qualifications, and professional judgment and are not to be construed as warranties or guaranties. In addition, opinions relating to environmental, geologic, and geotechnical conditions or other estimates are based on available data, and actual conditions may vary from those

encountered at the times and locations where data are obtained, despite the use of due care.

Certification

I, Patrick Clem, being a Registered Professional Engineer in the State of Wyoming, certify to the best of my knowledge, information, and belief, that the statistical methods selected to evaluate groundwater monitoring data for the CCR units that are the subject of this certification, as identified in **Table 6-1** of this report, are appropriate and comply with the performance standards specified in 40 CFR § 257.93(g), and that this certification is true and correct and has been prepared in accordance with generally accepted good engineering practices.

SIGNATURE: Lotrick, M. Clan





1. Introduction

On behalf of Basin Electric Power Cooperative (Basin), AECOM Technical Services, Inc. (AECOM) prepared this revised report documenting the Coal Combustion Residuals (CCR) groundwater monitoring systems for the CCR units at Basin's Laramie River Station (LRS) located east of Wheatland, Wyoming (see **Figure 1-1**). This report addresses the requirement under Chapter 40 Code of Federal Regulations (CFR) Part 257.105(h) to provide in the Operating Record, as it becomes available, "documentation of the design, installation, development, and decommissioning of any monitoring wells, piezometers and other measurement, sampling, and analytical devices."

Pursuant to 40 CFR § 257.90(b)(1), by October 17, 2017, an owner and operator of a CCR unit must install a groundwater monitoring system that meets the requirements of 40 CFR § 257.91. The groundwater monitoring system must meet the CCR Rule's performance standard, which requires the system to consist of a sufficient number of wells, installed at appropriate locations and depths, to yield groundwater samples from the uppermost aquifer that accurately represent the quality of:

- 1. Background groundwater that has not been affected by leakage from a CCR unit; and
- Groundwater passing the waste boundary of the CCR unit—the downgradient monitoring system must be
 installed at the waste boundary that ensures detection of potential groundwater contamination in the uppermost
 aquifer and must monitor all potential contaminant pathways.

This report summarizes the procedures and field activities associated with drilling and installation of monitoring wells that comprise the CCR monitoring networks at LRS, as well as the results of testing and monitoring of wells to evaluate the network against the requirements of the Final Rule.

This report is organized as follows:

- Section 1 includes a brief introduction to this report;
- Section 2 provides a brief background with historical information concerning LRS and associated CCR units;
- Section 3 describes the geological and hydrogeological setting of LRS;
- Section 4 describes selection and installation of the CCR monitoring well networks for all CCR units at LRS, including the drilling and installation of monitoring wells to supplement existing monitoring wells at LRS;
- Section 5 presents an evaluation of the LRS CCR monitoring compared to the requirements of the CCR Rule;
- Section 6 describes the statistical methodology that will be used to evaluate CCR groundwater monitoring data;
- Section 7 describes the professional limitations that apply to this report; and
- Section 8 lists the references cited in this report.

Certifications pertaining to the design and construction of the groundwater monitoring system and selection of the statistical method for evaluating data acquired using the groundwater monitoring system, are presented before **Section 1**.

2. Background

The LRS is one of the largest consumer-operated, regional, joint power supply ventures in the United States (U.S.). LRS is a coal-based generating station located in Platte County east of Wheatland, Wyoming, and has a total power output capacity of 1,710 megawatts (MW) from three coal-based units:

- Unit 1, with a rating of 570 MW, which began operating in 1980;
- Unit 2, with a rating of 570 net MW, which began operating in 1981; and
- Unit 3, with a rating of 570 net MW, which began operating in 1982.

Coal ash is disposed at LRS in the following CCR units:

- Ash Landfill
- Ash Pond 1
- Ash Pond 2
- Ash Pond 3
- Emergency Storage Ponds

The three ash ponds and the landfill are located west of the generating units and office complex, near the western edge of the Site. The two emergency holding ponds are located north of the generating units in the northeastern part of the Site. The landfill and ash ponds were permitted in 1978 and began receiving coal ash in 1980. The emergency holding ponds were subsequently incorporated due to disposal of flue gas desulfurization (FGD) materials. Basin Electric reported that in 2014 the landfill received 284,119 tons of solid waste, including fly ash, FGD waste, and a minor contribution of solid debris. The landfill is currently accessed via a haul road running generally east to west along the south side of the landfill.

Due to the presence of CCR, the LRS ash ponds, landfill, and emergency holding ponds are regulated by the CCR Rule, promulgated by the U.S. Environmental Protection Agency (EPA) under Chapter 40 CFR Part 257, Subtitle D of the Resource Conservation and Recovery Act (RCRA). The CCR Rule establishes requirements for existing CCR landfills and surface impoundments, including groundwater monitoring and corrective action. The groundwater monitoring provisions of the CCR Rule require the installation of a system of monitoring wells, the specification of procedures for sampling these wells, and analysis of the resulting data to detect the presence of hazardous constituents. A corrective action process is required in the event that hazardous constituents are detected above background concentrations at levels exceeding groundwater protection standards.

3. Geological and Hydrogeological Setting

The geological and hydrogeological setting is important to understanding the groundwater environment in the vicinity of the LRS. The geologic history of Platte County is similar to most areas within the Front Range of the Rocky Mountains. Platte County is underlain by marine and continental deposits of limestone, conglomerate, sandstone, siltstone, shale, and unconsolidated sediments. Deposits range in thickness over the Laramie Range, Hartville uplift, and related features up to 10,000 feet (ft) in the east central and southeastern parts of the county. Precambrian rocks generally make up the mountainous (structurally complex) areas, Paleozoic and Mesozoic rocks adjoin the older formations, and Tertiary and Quaternary rocks underlie most of the county east of the Laramie Range (U.S. Geological Survey [USGS] 1960). The Laramide Orogeny was active in the county approximately 70 million years ago marking the beginning the Hartville uplift and Laramie Range. In the Cenozoic, streams eroded the eastern side of the range depositing silts, sands, and gravels of the Brule and Arikaree Formations that underlie the Wheatland area and subsequently Basin Electric LRS.

Precipitation landing on the eastern flank of the Laramie Range supplies surface water to perennial and ephemeral streams that flow east towards the basin. Most surface water west of Wheatland eventually joins with the Laramie River continuing east before discharging into the Platte River near Fort Laramie. Groundwater near Wheatland is recharged primarily through infiltration on the eastern flank of the Laramie Range, and through re-infiltration of irrigation water during the spring, summer, and fall months. Some groundwater in the saturated zones eventually returns to the land surface through seeps and springs, or is discharged by wells and evapotranspiration; however, the majority flows into surface streams. Alluvial drainages bounding the eastern (Wheatland Creek) and western portions (Chugwater Creek) of the facility transport surface water generally northward, discharging to the Laramie River (USGS 1960). Some groundwater within these regions percolates into the Arikaree Formation which holds the uppermost aquifer beneath the facility.

The LRS facility is underlain by a 5- to 30-ft section of Quaternary sediments that overlies the Arikaree Formation. The Arikaree Formation is comprised primarily of loosely- to moderately-cemented very fine- to fine-grained sandstone containing interbeds of silts and clays. A lower unit consists of lenses of loosely to well-cemented red to gray coarse sandstone interbedded with lenses of well-cemented conglomerate. A basal conglomerate lies unconformably upon the underlying Brule Formation in many places throughout Platte County (USGS 1960). The 2016 AECOM drilling investigation did not penetrate to depths great enough to expose the lower portions of the Arikaree; however, a review of the geologic logs generated during the drilling of the on-Site water supply well (Forell-Baumgardner No. 2) suggests the Brule Formation is approximately 820 ft below ground surface (bgs) in the western portions of the Site. Based on this information, the local thickness of the Arikaree Formation on-Site is approximately 790 ft thick.

The lithologic characteristics of the Arikaree Formation beneath the LRS are generally consistent, although there are slight differences in the degree of cementation and induration, and minor variations in grain size. Few fractures were noted in borehole soil cores obtained during monitoring well network installation described in **Section 4**. Interbeds with higher silt and clay content, coupled with greater cementation generate thin discontinuous perched groundwater horizons that are interpreted to hold only seasonal groundwater. The perched groundwater would tend to percolate downward to what is interpreted as the uppermost aquifer based on data obtained during monitoring well installation and aquifer testing. The uppermost aquifer is present at a depth of approximately 95 ft bgs in the southeastern portion of the LRS facility, and slopes generally north towards the Laramie River. The hydraulic gradient for the uppermost aquifer beneath the Site appears to be controlled dominantly through topographic features and enhanced infiltration zones in permeable shallow alluvium.

4. Monitoring Well System Selection and Installation

Monitoring well systems have been established at LRS to comply with the requirements of the EPA CCR Rule published in the Federal Register on April 17, 2015. The systems are comprised of several existing monitoring wells that predate the CCR Rule, as well as a number of additional monitoring wells that were installed to complete the systems and fully comply with requirements of the Rule.

4.1 Monitoring Well Installation in 2016

Sixteen monitoring wells were installed at LRS during the summer of 2016 to target the uppermost aquifer in the vicinity of the LRS CCR units, including nine monitoring wells (MW-32B through MW-40B) around the landfill and ash ponds west of the main plant area, and seven monitoring wells (MW-41B through MW-47B) surrounding the emergency holding ponds generally north of the main plant area (**Figure 4-1**). The monitoring well locations were selected to evaluate the direction of groundwater flow in the vicinity of the LRS CCR units.

Monitoring well installation involved drilling, well construction, development, and aquifer testing is described below.

4.2 Drilling and Well Construction

Subsurface utilities in the vicinity of each planned monitoring well installation location were identified by utility representatives following the One-Call of Wyoming notification system. The uppermost 5 ft of each boring location was excavated using hydro-excavation or hand-auger as an additional precaution against utility strikes.

Monitoring well drilling and construction occurred between July 13 and August 12, 2016. The monitoring wells were installed using sonic drilling methods. Soil cores recovered during drilling operations were photographed and logged by AECOM geologists. Boring logs are included in **Appendix A**. Each boring was drilled 10 to 15 ft below the elevation at which groundwater was encountered. Moist to wet horizons were encountered at multiple depths during drilling. A submersible pump was used to test these zones for water production. If water was not recharging within the test interval, the borehole was advanced deeper until a distinct groundwater horizon was reached, or pump testing yielded significant quantities of groundwater.

Each new monitoring well was constructed of 2-inch-diameter, schedule 40 polyvinyl chloride (PVC) riser pipe and slotted screen. The screen interval was constructed using 20 ft of 0.010-inch factory-slotted PVC screen straddling the water table or groundwater production zone. The annular space within the borehole around the screen was filled with clean 10/20 silica sand filter pack to a minimum of 2 ft above top of screen. Bentonite chips were placed above the filter pack and hydrated with potable water to seal the filter pack from surface influence. The remaining annular space above the bentonite seal was filled with Portland Type I/II grout and allowed to set for a minimum of 24 hours before well development activities were completed. Above-grade stainless steel monuments, lockable J-Plugs, and monument locks were installed to protect and secure the wellhead. Surface monuments were labeled with the well identification number and set within a 2-foot square concrete pad. Steel bollards painted yellow were installed around wells located near traffic areas to enhance visibility and protect the wells. Well construction diagrams are included in **Appendix A**, and construction details are summarized in **Table 4-1**.

4.3 Well Development

The newly installed monitoring wells were developed between August 15, 2016, and August 17, 2016. Well development activities included measuring the water level and total depth of the well, surging the well with a PVC surge block, bailing the well with a weighted bailer to remove initial influx of sediment into the well, and finally using a submersible pump to purge the well. After well measurements were taken, a surge block was used to surge water into and out of the screened portion of the well for a minimum of 10 to 15 minutes. Bailers were then used to remove water and sediment from the well prior to pumping using a submersible electric pump. A minimum of ten well volumes of water were removed from each monitoring well during well development. Field parameters (pH, temperature, specific conductance and turbidity) were measured and recorded at regular intervals during pumping. Each well was purged until visibly clear and a minimum ten well casing volumes were removed. Purge water generated during well development was spread on the adjacent ground surface.

4.4 Aquifer Testing

A combination of aquifer pumping and slug tests were performed on the 16 new monitoring wells around the emergency ponds and ash ponds to evaluate the hydraulic conductivity of the geologic formation at each well location. The aquifer tests were performed between August 19, 2016 and August 23, 2016, after all wells had been developed.

Slug tests were performed on eight wells (MW-33B, MW-35B, MW-37B, MW-38B, MW-39B, MW-42B, MW-45B, and MW-47B). Wells MW-42B, MW-45B, and MW-47B are located directly adjacent to the Emergency Holding Ponds (Figure 4-1). Wells MW-33B, MW-35B, MW-37B, MW-38B, and MW-39B are located around the Ash Ponds to the southwest of the Emergency Holding Ponds. Slug tests were performed by first taking water level and total depth measurements on the well. A transducer was then lowered into each well and set at a depth of approximately 1 to 2 ft off the bottom of the well. The well was then allowed to recover back to the static water level recorded before the transducer was placed in the well. The transducer then began recording data and a slug was lowered into the well. The slug was placed at a depth to be fully submerged in the well above the height of the transducer, making sure that the transducer did not move during placement. The slugs used for the tests at the Basin Electric Laramie River station consisted of a 1-inch by 6-ft long section of PVC capped at both ends and filled with sand. One end of the slug contained an eye hook to allow a nylon rope to be attached to it for placement and retrieval. After slugs were placed in the well, the water level was allowed to return to within 95 percent of the static water level. Once 95 percent of static was achieved, the slug was removed and the water level was once again allowed to return to within 95 percent of static. The transducer in the well recorded the depth of water above the transducer, temperature, and hydraulic head pressure measurements within the well for the entirety of the test. Once a 95 percent static water level was achieved after removing the slug, the test was stopped and equipment was removed and decontaminated before using in the next well to be tested. Manual water level measurements were recorded on field slug test forms (Appendix B) and electronic data was removed from the transducer to be used for data evaluation.

Aquifer pumping tests were performed at eight of the newly installed monitoring wells at the Site (MW-32B, MW-34B, MW-36B, MW-40B, MW-41B, MW-43B, MW-44B, and MW-46B). Wells MW-41B, MW-44B, MW-46B, and MW-43B are located around the Emergency Holding Ponds (Figure 4-1). MW-32B, MW-34B, MW-36B, and MW-40B are located around the Ash Ponds. The pumping tests were performed in a similar manner to the slug testing (reference AECOM Standard Operating Procedure 18.0, Appendix B). Prior to starting each aquifer test, water level and total well depth measurements were taken. After well measurements were taken, a submersible electric pump was lowered into the well and placed at a level approximately 2 to 3 ft off the bottom. A transducer was attached to the pump tubing approximately 1 ft above the top of the pump. The water level was then allowed to stabilize before the test was started. After water level stabilization, the transducer started recording data and the pump was turned on to a flow rate ranging from 0.5 to 1.5 gallons per minute. The pumping rate was held constant during the test and drawdown in the well was recorded using the transducer and periodic manual water level measurements using an electronic water level meter. The pump remained on until drawdown neared the elevation the transducer was placed, or the water level in the well stabilized. The pump was then shut off and recovery of the water level was measured until 95 percent of the static initial water level was reached, at which time the test was stopped and equipment removed from the well and decontaminated prior to testing of the next well. Manual measurements were recorded on field aquifer testing forms (Appendix B) and electronic data was removed from the transducer and used for data evaluation.

4.5 Slug Test Analysis

Data from the slug tests performed at the Site were processed and analyzed using the software AQTESOLV (Duffield 2007), which provides type curve solutions from published methods corresponding to a range of conceptual models for various well completions and aquifer types (e.g., fully penetrating well in an unconfined aquifer), and simplifying hydrologic assumptions (e.g., infinite aquifer extent). After initial processing and analysis, the most appropriate conceptual model was determined to be the Bouwer and Rice Slug Test Solution for Unconfined Aquifers (Bouwer & Rice 1976). This method uses a straight line fit to the observed water-level displacement and is applicable to wells screened below and across the water table. Graphs of the slug test results are included in **Appendix B**. For wells screened across the water table (all except MW-39B and MW-42B), a double straight line can be observed in the data sets when plotted on a log-normal axes, with a line of a steeper slope in early time representing groundwater entering or exiting the well from the filter pack. The second segment of data with a shallower slope represents the behavior of the geologic formation located immediately outside of the filter pack. In these cases, an effective casing

radius correction factor is applied and the straight line solution is fit to the second slope in the data. Some basic assumptions of the Bouwer-Rice solution include:

- Aquifer has infinite areal extent;
- Aquifer is isotropic and has uniform thickness;
- Aquifer potentiometric surface is initially horizontal;
- Control well is fully or partially penetrating;
- A volume of water, V (the slug), is injected or discharged instantaneously from the control well;
- Flow is steady; and
- Aquifer is confined or unconfined.

Results from the slug test analyses are summarized in **Table 4-2**. AQTESOLV analyses of the slug test data for each well are presented in **Appendix B**. The average estimated hydraulic conductivity of the completed slug tests was 2.16 ft per day (ft/d) with a geometric mean of 1.65 ft/d. The minimum hydraulic conductivity of 0.45 ft/d was estimated at MW-39B. The maximum of 6.28 ft/d was estimated at MW-37B. Although the software calculates a value for aquifer storage from the slug test data, these values are assumed to represent rough approximations, as both slug tests and single-well pumping tests are considered relatively poor methods to determine aquifer storage.

4.6 Pumping Test Analysis

Pumping test data also were analyzed using the software package AQTESOLV. Type curve solutions for pumping tests available in AQTESOLV typically require observation well data. In cases where observations from only the pumping well are available, aquifer storage calculations are not usable; however, hydraulic conductivity calculations are still valid. Data were analyzed as single well pumping tests using the Moench solution for a pumping test in an unconfined aquifer (Moench 1997). The Moench solution is the only available option in AQTESOLV for unconfined aquifers which accounts for wellbore storage. Due to the low flow rate of the pump test, wellbore storage has a clear effect on the data in early time. Although the data most indicative of aquifer behavior is in late time, it is helpful to account for the wellbore storage through derivative plot analysis, ensuring that the conceptual model for the solution is valid. For the pumping test analysis graphs shown in **Appendix B**, the observed data are plotted with black squares, and the Moench solution is plotted with a blue line. The derivate data are plotted with gray crosses and the derivate solution is plotted in a red line.

The Moench solution utilizes the following assumptions:

- Aquifer has infinite areal extent;
- Aquifer is homogeneous, isotropic and of uniform thickness;
- Aquifer is unconfined; and
- Flow is unsteady.

Results from the pumping test analyses are summarized in **Table 4-2**. AQTESOLV analyses of the pump test data for each well are presented in **Appendix B**. The average estimated hydraulic conductivity of the eight pumping tests was 1.40 ft/d with a geometric mean of 1.19 ft/d. The minimum hydraulic conductivity of 0.65 ft/d was estimated at MW-34B. The maximum of 3.12 ft/d was estimated at MW-41B. During the pumping test on MW-44B, the well began to recharge prior to the completion of the test. No significant fluctuations were observed in the flow rate at this time. It is possible that these results were affected by the drawdown cone reaching a recharge boundary, or insufficient well development. Hydraulic conductivity results for well MW-44B yielded similar results to the other wells tested.

4.7 Addition of Existing Monitoring Wells in November 2016

The first CCR baseline groundwater monitoring event at LRS was conducted in September 2016. A review of the resulting data concluded that the assessment of baseline groundwater conditions associated with the ash impoundments and landfill could be improved by modifying the list of monitoring wells included in the CCR monitoring system. Therefore, the monitoring system was modified in November 2016, as described below for subsequent baseline monitoring events:

- Existing monitoring wells MW-14BR, MW-20B, and MW-21B were added to the groundwater monitoring
 program (Figure 4-1). These wells are located downgradient of the ash impoundments and eastern portion of
 the ash landfill, and supplement the downgradient data provided by MW-36B, MW-37B, MW-38B, and MW-48B.
- Monitoring wells MW-33B, MW-34B, and MW-35B were removed from the groundwater sampling program because they were found to be cross-gradient from the ash impoundments, although groundwater elevations continued to be measured in these wells to support interpretation of Site-wide groundwater flow.

4.8 Addition of New and Existing Monitoring Wells in 2017

The LRS CCR groundwater monitoring network was modified in July 2017 based on an evaluation of interim baseline data acquired in 2016 through the spring of 2017. The rationale for expanding the network was to provide greater resolution of baseline groundwater quality and flow in the vicinity of the three ash ponds, and support an evaluation of upgradient and downgradient conditions for Ash Pond 1 and a multi-unit consisting of Ash Pond 2, Ash Pond 3, and the Ash Landfill. The monitoring wells added to the network in July 2017 included two existing wells along the northern edge of the ash ponds: MW-22B and MW-23B. In addition to these wells, six new monitoring wells were installed along the northern edge of the ash ponds and between the ash ponds: MW-48B, MW-49B, MW-50B, MW-51B, MW-52B and MW-53B.

The six new monitoring wells were installed using sonic drilling methods consistent with the methods described above to install the monitoring wells in 2016. Drilling and well construction was performed by O'Keefe Drilling of Butte, Montana. Soil cores recovered during drilling were logged by an AECOM geologist. **Appendix A** contains the borings log for each monitoring well. Construction details for the six monitoring wells installed in July 2017 are presented in **Table 4-1**. Each well was constructed of 2-inch-diameter, schedule 40 PVC riser pipe and slotted screen. The well screen was constructed using 20 ft of 0.010-inch factory-slotted PVC screen positioned to straddle the water table. The annular space between the borehole and screen was filled with clean 10/20 silica sand filter pack to a minimum of 2 ft above top of screen. Bentonite chips were placed above the filter pack and hydrated with potable water to seal the filter pack from potential infiltration of surface water. The annular space above the bentonite seal was filled with Portland Type I/II grout and allowed to set for a minimum of 24 hours. All wells except for MW-48B were completed with flush-mount construction to provide protection from vehicular traffic. MW-48B was constructed with an above-grade outer steel casing. The surface monuments were labeled with the well identification number and anchored in a 2-ft square concrete pad. Construction diagrams for the six monitoring wells are included in **Appendix A**.

The six newly installed monitoring wells were developed by surging with a PVC surge block and pumping using a submersible pump. Each well was purged until visibly clear and after a minimum ten well casing volumes were removed. Purge water generated during development was spread onto the adjacent ground surface. Aquifer testing was not performed on the six new monitoring wells because sufficient data was obtained during testing of the 16 wells installed in 2016 to adequately characterize the hydrogeological characteristics of the uppermost aquifer in the vicinity of the LRS CCR units.

4.9 Addition of New Monitoring Wells in 2019

Because of the detection of Appendix IV constituents at statistically significant levels above groundwater protection standards in monitoring well MW-38B, LRS installed a series of four monitoring wells to fulfill the requirements of 40 CFR 257.95(g)(1)(i) to characterize the nature and extent of the release. Three of the wells (MW-54B, MW-55B, and MW-56B) help delineate the horizontal extent, including the characterization at the downgradient property boundary (**Figure 4-1**). The fourth new monitoring well, MW-38C, was installed near MW-38B to evaluate the vertical extent of potential release of CCR constituents from Bottom Ash Pond 1.

The four new monitoring wells were installed using hollow system auger methods. Drilling and well construction was performed by Inberg Miller located in Riverton, Wyoming. Soil cores recovered during drilling were logged by an AECOM geologist. **Appendix A** contains the borings log for each monitoring well. Construction details for the four monitoring wells installed in 2019 are presented in **Table 4-1**. MW-54B was constructed with a 2-inch internal diameter (ID), Schedule 40, PVC riser. MW-55B, MW-56B, and MW-38C were constructed with 2-inch ID, Schedule 80, PVC risers. The well screen was constructed using 20 ft of 0.010-inch factory-slotted PVC screen positioned to straddle the water table for wells MW-54B, MW-55B, and MW-56B. MW-38C was constructed with an 11-ft long, 2-inch ID, 0.010-inch slotted PVC screen with prepacked sand. A prepacked screen was utilized due to flowing sands entering the borehole due to increased hydrostatic pressure causing the surrounding formation to enter

the borehole before a screen and sand pack could be installed. The annular space between each PVC well casing and boring wall was filled with 10/20 silica sand pack from the bottom of the boring to 2 ft above the top of each screen. The filter pack material was emplaced in a manner to prevent bridging between the borehole and the casing. A 2-foot thick layer of 3/8-inch bentonite chips was installed above the sand pack and hydrated. A hydrated bentonite grout mixture was placed above the interval of hydrated bentonite chips to the surface. Wells were constructed with an above-grade outer steel casing. The surface monuments were labeled with the well identification number and anchored in a 2-ft square concrete pad.

4.10 Replacement of Existing Monitoring Wells in 2024

LRS replaced monitoring well MW-53B with monitoring well MW-53BR in April 2024 due to damage to the screen interval of well MW-53B, which was located on the top edge of a steep berm that supports an access road encircling Pond 3. The old well was drilled to a depth of 120 ft bgs between May 20, 2017 and May 28, 2017, as part of the original CCR network. MW-53B was determined to have a damaged 20-ft screen (100 ft bgs to 120 ft bgs), which allowed silt to infiltrate into the well and completely fill and block the screened interval with silt. The well was abandoned using hydrated bentonite grout in April 2024. Monitoring well MW-53BR was drilled to a depth of 120 ft bgs to match the depth of the original MW-53B. **Appendix A** contains the boring log for MW-53BR. Materials and well completion consisted of the following:

- 100 ft of 2-inch ID, schedule 40 PVC casing;
- 20 ft of 2-inch ID, schedule 40 PVC, 0.010 slotted screen (100 120 ft bgs);
- Casing centralizers placed approximately every 25 vertical ft;
- 10/20 silica sand filter pack from 120 to 98 ft bgs;
- Bentonite seal from 98 to 93 ft bgs;
- Bentonite grout from 93 to 10 ft bgs;
- Bentonite seal from 10 to 4 ft bgs; and
- Concrete from 4 ft bgs to surface.

Monitoring wells MW-50B, MW-51B, and MW-52B, which are located on the same berm as MW-53B/BR, required their surface completions be raised between six and 30 inches to be flush with the access road, because the elevation of the access road was modified during Pond 3 retrofit activities. In April 2024, Cascade Drilling removed the original surface completions of the three wells and attached a sufficient length of 2-inch ID schedule 40 PVC casing to the existing well riser to bring them flush with the access road. New traffic rated flush-mount surface completions were added for security and protection. Stockpile soil at the Site was used to fill in the depressions left by the removal of the original surface completions and then a new 2-ft by 2-ft concrete pad was constructed into which a new metal vault was secured for each well location. Construction details for the four monitoring wells installed or modified in April 2024 are presented in **Table 4-1**.

4.11 Recovery Well Installation

There are two wells at the facility that are not currently part of the compliance network. The two recovery wells, RW-1 and RW-2, were installed between November 3, 2021 and November 6, 2021 at Basin LRS. A subcontracted drilling company (Cascade Drilling/Technical Services) mobilized a sonic drilling rig and crew to LRS for the well installation with observation and oversight by an AECOM geologist. Both wells are constructed of 6-inch ID schedule 40 PVC. Each well was temporarily completed with a stick up above ground surface with four safety bollards surrounding. Outer protective casings were not installed at the wells because the surface completions were redone during the final completion phase of the extraction well system completed in 2024. RW-1 and RW-2 were drilled and completed to 80 and 85 ft bgs), respectively. Each well has a screen interval of 20 ft at the bottom of the well with a 0.020-inch PVC slotted screen. The borehole annulus around the screen was filled with 10/20 Colorado silica sand pack that was placed from the bottom of the screen to 2 ft above the top of screen. A minimum of 2 ft of bentonite chips was placed above the sand pack as a seal and allowed to hydrate before the well was grouted. The annulus of the borehole was grouted using a high solids bentonite grout from the top of the seal to approximately 7 to 8 ft bgs. The remaining space to ground surface was initially filled with 10/20 sand, which was later removed during installation of pitless adapters during extraction system installation and startup. Sampling ports were installed on extraction system piping

at each well head to allow for potential compliance monitoring of the system in the future. The recovery wells are not part of the compliance network that is required to be certified in this report.

5. System Evaluation

The Final CCR Rule establishes the following general performance standard for CCR groundwater monitoring systems:

- All groundwater monitoring systems must consist of a sufficient number of appropriately located wells (at least one upgradient and three downgradient wells) in order to yield groundwater samples from the uppermost aquifer that represent the quality of background groundwater and the quality of groundwater passing the CCR waste boundary.
- The objective of a groundwater monitoring system is to intercept groundwater to determine whether the
 groundwater has been contaminated by the CCR disposal unit. The number, spacing, and depths of the
 monitoring wells must be determined based on a thorough characterization of the Site, including a number of
 specifically identified factors relating to the hydrogeology of the Site.

The "uppermost aquifer" and "aquifer" are defined in the Final CCR Rule in § 257.53, as follows:

"Uppermost aquifer" means the geologic formation nearest the natural ground surface that is an aquifer, as well as lower aquifers that are hydraulically interconnected with this aquifer within the facility's property boundary. Upper limit is measured at a point nearest to the natural ground surface to which the aquifer rises during the wet season.

"Aquifer" means a geologic formation, group of formations, or portion of a formation capable of yielding usable quantities of groundwater to wells or springs.

As described in the drilling and well construction discussion in **Section 4**, drilling equipment and procedures were employed to identify the uppermost aquifer and ensure each new monitoring well was installed with appropriate total depth and placement of the well screen to: (1) facilitate collection of representative samples of the uppermost aquifer; and (2) accurately measure water table elevations to support evaluation of groundwater gradient and flow direction.

Also as described in **Section 4**, selection and construction of the CCR monitoring system for LRS evolved and adapted based on the results obtained from baseline groundwater monitoring in 2016 and 2017. The final monitoring system consists of 22 monitoring wells that will be sampled as part of the detection monitoring program. The list of wells selected for sampling background and downgradient groundwater quality for each CCR unit or multi-unit is summarized below:

CCR Unit/Multi-unit	Background Wells	Downgradient Wells
Ash Pond 1	MW-52B, MW-53BR	MW-49B, MW-21B, MW-38B, MW-38C*
Ash Pond 2, Ash Pond 3, Ash Landfill	MW-39B, MW-32B	MW-36B, MW-37B, MW-20B, MW-14BR, MW-40B, MW-52B, MW-53BR
Emergency Holding Ponds	MW-41B, MW-42B, MW-43B	MW-44B, MW-45B, MW-46B, MW-47B

*MW-38C, installed in April 2019 was added to the assessment monitoring program as a downgradient compliance well for Bottom Ash Pond 1.

The following nine monitoring wells are included in the monitoring system only for the purpose of measuring groundwater elevations and evaluating groundwater flow direction and velocity in the vicinity of the ash ponds and landfill:

- MW-33B, MW-34B, MW-35B, MW-48B, MW-50B, MW-51B, MW-545B, MW-55B, and MW-56B.
- Monitoring wells MW-22B, and MW-23B were removed from the monitoring well network due to damage sustained to the wells during retrofit of Ponds 1 and 2. MW-22B and MW-23B had been used for measuring groundwater elevations only.

Potentiometric surface maps have been constructed using the depth to groundwater measurements obtained during recent groundwater monitoring and monitoring well locations and elevations measured by a licensed professional land surveyor. Maps of the potentiometric surface for June 2024 and September 2024 are presented as **Figures 5-1**

and **5-2**, respectively. The associated depth to groundwater measurements and calculated groundwater elevations are presented in **Table 5-1** and **Table 5-2**. Groundwater elevations were calculated at each well by subtracting the measured depth to groundwater from the surveyed top of casing elevation. Groundwater elevations for each monitoring well are posted on the figures, with inferred isoelevation contours of the groundwater potentiometric surface. The direction of groundwater flow is generally to the northeast, perpendicular to the potentiometric contour lines. The groundwater flow maps reflect a relatively consistent pattern of groundwater flow beneath the LRS CCR units, and support the selection of the wells listed above to represent background groundwater quality and the quality of groundwater downgradient of the CCR units.

- The monitoring well system has been established at the CCR Landfill at the Basin Electric Power Cooperative Laramie River Station to comply with the requirements of the EPA CCR Rule. The following factors support the spacing and depth of the monitoring wells of the network. Groundwater flow at the Site is generally to the north-northeast. The majority of the recharge to this aquifer is from irrigation on agricultural lands to the south of the facility, resulting in relatively shallow, predominantly horizontal groundwater flow.
- The wells were screened in the uppermost aquifer, which beneath the LRS facility is the upper unit of the Arikaree Formation. As detailed in **Section 3**, this is a very fine-grained soft- to moderately-hard, generally massive sand and silt. Well screens are 20-ft long for most of the monitoring wells at the facility and the screens range in depth from 55 to 110 ft bgs.
- The Arikaree Formation, which is about 790-ft thick beneath the Site, is the principal water source for agricultural irrigation in the Wheatland Flats. Because of the relatively low permeability of the Arikaree Formation, large yields of water cannot be obtained from its shallower depths. Wells with depths of 500 ft or more are typically required in order to obtain sufficient yields for irrigation (Morris and Babcock 1960). Thus, the CCR program wells are screened a significant vertical distance above the primary water source for the region.
- In 2019, monitoring well MW-38C was installed with a screened interval from 125 to 114 ft bgs to assess for the
 potential of a downward contaminant migration component from the uppermost aquifer well MW-38B (screened
 from 75 to 55 ft bgs). Water quality sampling and groundwater level measurements from these two wells have
 provided the following results:
 - The static water level measured at MW-38C has been higher than at MW-38B, indicating an ambient upward gradient in groundwater flow (i.e., an impediment to downward migration of contaminants) (AECOM 2023).
 - Contaminants detected in the shallow well (MW-38B) were not detected in the deep well (MW-38C) above cleanup criteria (AECOM 2019).

These provide further evidence that groundwater flow is relatively shallow and predominantly horizontal, and that the monitoring wells are screened in the uppermost aquifer and are hydraulically isolated from the regional principal aquifer.

- The Arikaree Formation is largely aeolian in origin, resulting in deposition of relatively homogeneous and isotropic silty fine sand deposits through which groundwater flow can be expected to be relatively consistent and predictable. To determine horizonal well spacing, flow paths were drawn across the units with downstream arrows terminating at monitoring wells downgradient of each unit/multi-unit. This demonstrates that the horizontal spacing of the downgradient wells ranges from approximately 600 ft between MW-14BR and MW-20B to approximately 1,000 ft between MW-36B and MW-37B.
- The unsaturated (vadose) zone between the base of the landfill and the monitored aquifer is approximately 60 to 160 ft thick. Hypothetical vertical contaminant transport from the landfill to the aquifer would be subject to significant lateral diffusion prior to encountering and mingling with groundwater, suggesting that any impacts to groundwater are unlikely to be confined to a discrete flow path as might be expected in a more heterogeneous aquifer and/or vadose zone.

Based on these factors, the spacing of downgradient wells in this aquifer is sufficient to accurately represent the quality of groundwater passing the waste boundary of each CCR unit/multi-unit and potential contaminant pathways are being monitored.

6. Statistical Methodology

6.1 Regulatory Guidance

Regulatory guidance provided in 40 CFR §257.90 specifies that a CCR groundwater monitoring program include selection of the statistical procedures to be used for evaluating groundwater quality data as required by 40 CFR §257.93. Groundwater quality monitoring data will be collected under the detection or assessment monitoring programs outlined in this plan and will include collection and analysis of samples from each background and downgradient compliance well, for each CCR unit or multi-unit, as required by 40 CFR §257.94(b) or 40 CFR §257(b). The groundwater samples will be analyzed for the constituents listed in 40 CFR §257 Appendices III and IV.

After the eight sets of groundwater samples were collected and analyzed, these data were statistically evaluated to determine if there are any statistically significant increases (SSIs) over background concentrations for the Appendix III and IV constituents. In determining whether an SSI has occurred, the constituent concentrations at the downgradient wells and the background wells for each unit/multi-unit were compared using one or more of the statistical methods discussed below.

40 CFR §257.93(f) outlines the statistical methods available to evaluate groundwater monitoring data. The statistical test(s) chosen will be conducted separately for each constituent in each monitoring well and will be appropriate for the constituent data and their distribution. The available statistical methods include the following:

- A parametric analysis of variance (ANOVA) followed by multiple comparison procedures to identify statistically significant evidence of contamination. The method must include estimation and testing of the contrasts between each compliance well's mean and the background mean levels for each constituent;
- An ANOVA based on ranks followed by multiple comparison procedures to identify statistically significant evidence of impacts. The method must include estimation and testing of the contrasts between each compliance well's median and the background median levels for each constituent;
- A tolerance or prediction interval procedure, in which an interval for each constituent is established from the distribution of the background data and the level of each constituent in each compliance well is compared to the upper tolerance or prediction limit;
- A control chart approach that gives control limits for each constituent; or
- Another statistical test method that meets the performance standards of 40 CFR §257.94(g) outlined in the paragraph below.

The chosen statistical method will comply with the following performance standards, as appropriate, based on the statistical test method used. The performance standards include the following:

- The statistical method used to evaluate groundwater monitoring data will be appropriate for the constituent distribution (i.e., parametric or nonparametric).
- If an individual well comparison procedure is used to compare an individual compliance well constituent concentration with background constituent concentrations or a groundwater protection standard, the test shall be done at a Type I error level no less than 0.01 or 0.05, depending on the method chosen. This performance standard does not apply to tolerance intervals, prediction intervals, or control charts.
- If a control chart approach is used to evaluate groundwater monitoring data, the specific type of control chart
 and its associated parameter values shall be such that this approach is at least as effective as any of the other
 statistical analysis approaches specified above.
- If a tolerance interval or a prediction interval is used to evaluate groundwater monitoring data, the levels of confidence and, for tolerance intervals, the percentage of the population that the interval must contain, shall be such that this approach is at least as effective as any of the other statistical analysis approaches specified above.
- The statistical method must account for data below the limit of detection with one or more statistical procedures that shall be at least as effective as any of the other statistical analysis approaches specified above.

• If necessary, the statistical method must include procedures to control or correct for seasonal and spatial variability as well as temporal correlation in the data.

Per 40 CFR §257.93(h)(2), statistical analysis of the first eight rounds of data must be completed within 90 days after completing the initial groundwater sampling and analysis to determine whether there has been an SSI over background for any constituent. The first eight rounds of groundwater sampling and analysis must be completed no later than October 17, 2017. In accordance with 40 CFR §257, LRS must obtain a certification from a qualified professional engineer stating that the selected statistical method is appropriate for evaluating the groundwater monitoring data for the CCR management area. The certification must include a narrative description of the statistical method selected to evaluate the groundwater monitoring data.

Assessment monitoring is required per 40 CFR §257.95 whenever a statistically significant increase over background levels has been detected for one or more of the indicator parameters listed in 40 CFR §257 Appendix III. An assessment monitoring program also includes annual groundwater sampling and analysis for the constituents listed in 40 CFR §257 Appendix IV. The purpose of assessment monitoring is to determine if releases of CCR constituents have occurred.

The facility can return to detection monitoring once assessment monitoring results are at or below background values for two consecutive assessment monitoring events. If the assessment monitoring identifies an Appendix IV constituent at a statistically significant level above the groundwater protection standard established per 40 CFR §257.95(h), groundwater assessment of corrective measures must be initiated.

6.2 Statistical Analysis Approach

There is no single method of statistical analysis that is appropriate for each groundwater constituent dataset. It is most prudent to use a suite of statistical methods that are dependent on the data and their distributions. The statistical analyses will be based on an interwell and/or an intrawell approach for the purpose of determining if an LRS CCR unit/multi-unit has impacted groundwater quality. The statistical algorithms used for the interwell and intrawell approaches will be chosen based on the groundwater constituent data and their distributions as well as consideration of natural seasonally- or spatially-varying groundwater constituent concentrations.

Eight rounds of baseline groundwater monitoring data were collected and analyzed for the 40 CFR 257 Appendices III and IV constituents. These data will be used to represent background groundwater quality for the LRS CCR units. The detection monitoring data collected at the downgradient wells will be used to determine if any of the CCR units/multi-unit have impacted groundwater quality. The initial eight rounds of detection monitoring sampling and analysis were completed prior to the October 17, 2017 deadline established in the CCR Rule (40 CFR §257.94).

A preliminary, exploratory statistical analysis was conducted after the eight rounds of baseline data were obtained to initially assess the constituent data and determine the most appropriate statistical approach(es) for the data. The data were examined for outliers and the percentage of non-detect values to verify that the data collected are suitable for statistical analysis. The data were also examined using goodness-of-fit tests to determine the most appropriate statistical distribution and time series plots and areal maps were used to determine if seasonal or spatial variations in constituent concentrations were present. Based on this preliminary evaluation of the data, an interwell statistical approach was selected as appropriate for evaluating groundwater at LRS, as described below.

Per 40 CFR §257.93(h)(2), statistical analysis of all eight rounds of data must be completed within 90 days after completing groundwater sampling and analysis to determine whether there has been a statistically significant increase over background for any Appendix III constituent.

6.3 Interwell Statistical Approach

Interwell tests compare the statistical differences between (upgradient) background and downgradient compliance wells. An interwell statistical approach will be used during detection monitoring for the following reasons:

• Sufficient data are available in the upgradient background well to ensure adequate degrees of statistical power to detect real exceedances above background levels, and also reasonable control over the site-wide false positive rate so that spurious exceedances have little chance of being identified.

Although there is evident spatial variation among most, if not all, of the Appendix III constituents, it is unclear to
what extent the similarly evident variation among the downgradient wells is due strictly to natural differences in
groundwater quality and/or other factors unrelated to management of the CCR ash. Because of this uncertainty,
an interwell comparison strategy appears to be initially more appropriate for LRS.

As a caveat to this approach, for constituents that occur naturally and vary substantially in concentration across LRS due to natural hydrogeologic or geochemical factors — thus, exhibiting significant spatial variability — an interwell testing scheme will not always be helpful. Using an interwell approach, constituent concentrations greater than background might be attributed to anthropogenic contamination, when in fact the differences are actually natural and due to locally varying distributions of groundwater constituents. In such cases, an intrawell approach may be warranted.

Furthermore, there is no requirement either in RCRA or the CCR Rule to use exactly the same statistical method or approach for every constituent. Depending on characteristics of LRS and data that are collected, a mix of interwell and intrawell tests may be warranted. At this Site, the initial statistical screening suggests that interwell comparisons are most appropriate despite evident spatial variability. However, that conclusion could change as additional data are collected during future detection monitoring. If new information indicates that constituent concentrations remain relatively stable and that the existing spatial variation is unrelated to the CCR units, a modification of the statistical approach to intrawell testing may be recommended for one or more constituents.

Under an interwell statistical approach in detection monitoring, the actual statistical method(s) chosen will be determined based on the constituent data distribution (as outlined below), which in turn is influenced both by the percentage and pattern of non-detect measurements as well as the temporal stability of the concentration levels.

When (1) the percentage of non-detects is low to moderate (i.e., less than 50-60 percent); (2) the background data can be normalized (perhaps via a standard transformation); and (3) the results are stationary (i.e., stable over time), the following statistical methods are highly recommended by USEPA (2009):

- Interwell control charts with retesting; or
- Parametric interwell prediction limit methods with retesting.

When the background data cannot be normalized (perhaps due to a large percentage of non-detects), but the data are stationary (i.e., stable over time), the following statistical method is recommended by USEPA (2009):

• Non-parametric interwell prediction limits with retesting.

Note that the specific retesting method in each of these options will be chosen to account for the size of the well network, the amount of background data available, the number of constituents being monitored, the site-specific mix of intrawell and interwell tests, and the impact of these factors on the statistical power and accuracy of the test. At this Site, the upgradient background wells relative to the number of downgradient wells to be tested on a semi-annual basis will enable use of a 1-of-2 retesting plan. This necessitates collection of a single independent resample at any location in which the initial routine measurement exceeds its respective statistical limit. A confirmed statistical exceedance will not be recorded unless both the initial measurement and resample value both exceed the statistical limit.

If the upgradient background data are non-stationary and thus exhibit a clear trend, it will suggest that factors unrelated to the CCR unit are impacting upgradient groundwater quality. Three general scenarios will be considered:

- Older background data may no longer be representative of current Site conditions and may need to be excluded from statistical calculations. In this case, the interwell statistical limits will be updated to include only the most representative background data.
- The compliance wells will be examined to see if similar trends are occurring downgradient. If so, a common trend component will be estimated across the Site and removed from every well. The residual data will then be used to construct revised statistical limits and tested, as described above.
- If the trend in upgradient background wells is not reflected in downgradient wells, further investigation may be needed to determine if the upgradient data still serve as a reasonable background with which to compare

downgradient compliance measurements. If not, the statistical approach will be modified to an appropriate intrawell strategy.

Because of the decision matrix needed to establish the correct statistical approach, the background data for each constituent will be periodically screened prior to construction of new or revised statistical limits. This screening will examine the proportion and pattern of outliers and potential data anomalies (perhaps due to laboratory or field sampling factors), the presence or absence of statistically significant trends over time, the presence or absence of statistically significant outliers, and the identification of an appropriate statistical distribution. In particular, any confirmed background outliers will be excluded from statistical calculations, so as not to unduly bias the statistical limits.

6.4 Proposed Statistical Methods for Appendix III Analytes

Table 6-1 provides a summary of the proposed statistical method by well for Appendix III analytes. This table is based on a screening of the background and downgradient well data collected to date. The proposed statistical method is subject to review and potential change in the future, as necessary, to address significant changes in conditions.

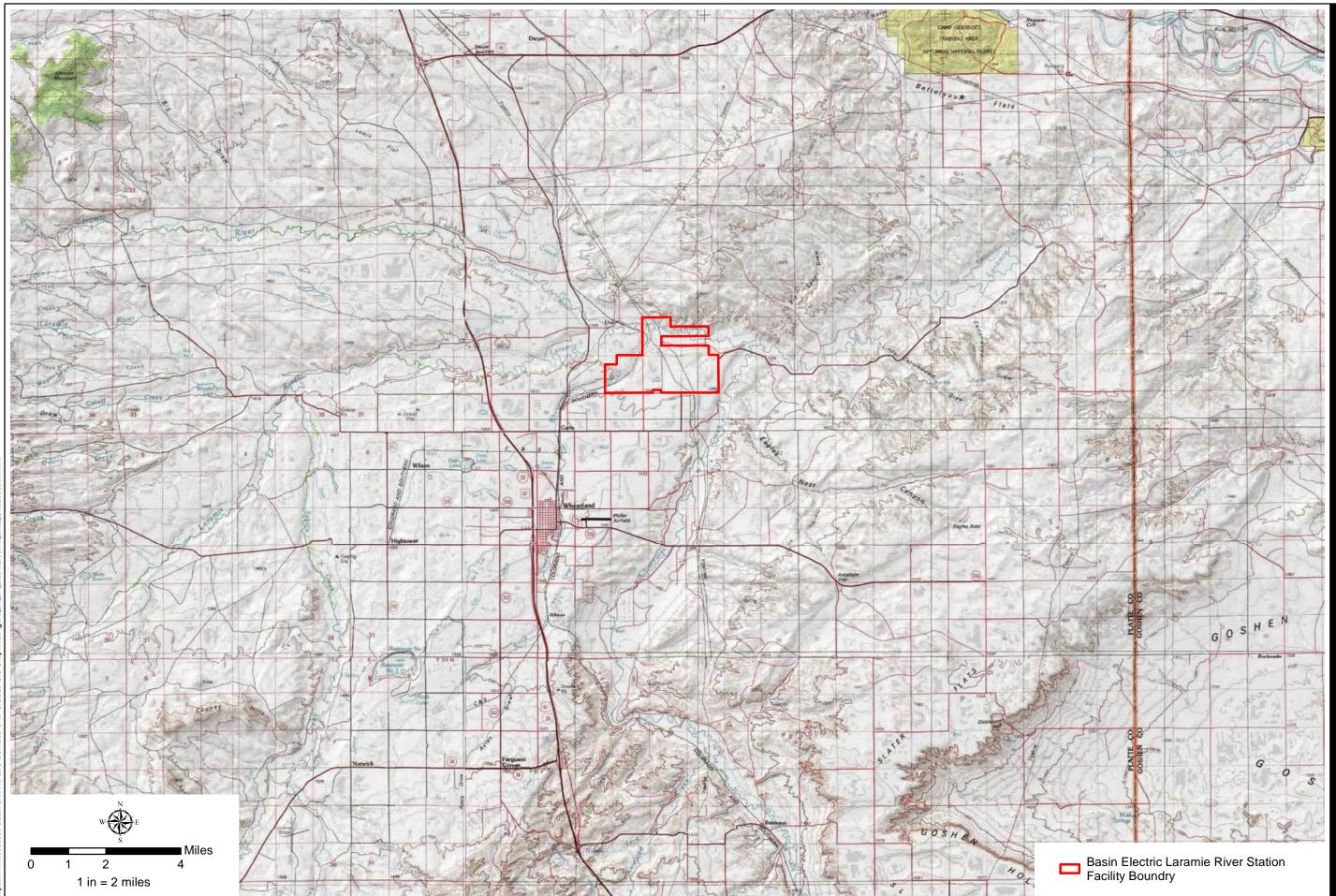
7. Limitations

The signature of AECOM's (Consultant's) authorized representative on this document represents that, to the best of Consultant's knowledge, information, and belief in the exercise of its professional judgment, it is Consultant's professional opinion that the aforementioned information is accurate as of the date of such signature. Any opinion or decisions by Consultant are made on the basis of Consultant's experience, qualifications, and professional judgment and are not to be construed as warranties or guaranties. In addition, opinions relating to environmental, geologic, and geotechnical conditions or other estimates are based on available data, and actual conditions may vary from those encountered at the times and locations where data are obtained, despite the use of due care.

8. References

- AECOM Technical Services, Inc. (AECOM). 2023. 2023 Annual Groundwater Monitoring and Corrective Action Report, Laramie River Station, Wheatland, Wyoming, January 31, 2024.
- AECOM. 2019. Groundwater Characterization Report, Laramie River Station, Wheatland, Wyoming, July 10, 2019.
- Bouwer, H. and R. Rice. 1976. A slug test method for determining hydraulic conductivity of unconfined aquifers with completely or partially penetrating wells. Water Resources Research, 12(3), pp. 423-428.
- Duffield, G. M. 2007. AQTESOLV Version 4.50, s.l.: HydroSOLVE, Inc.
- Moench, A. 1997. Flow to a well of finite diameter in a homogeneous, anisotropic water-table aquifer. Water Resources Research, 33(6), pp. 1397-1407.
- Morris, Donald A. and Babcock, Horace M. 1960. Geology and Groundwater Resources of Platte County, Wyoming. Geological Survey Water-Supply Paper 1490. Washington, D.C.: United States Government Printing Office.
- U.S. Environmental Protection Agency (EPA). 2009. Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities. Unified Guidance. EPA 530-R-09-007. March. 884 pp.
- U.S. Geological Survey (USGS). 1960. United States Department of the Interior, Geology and Ground Water Resources of Platte County, Wyoming, Geological Survey Water-Supply Paper 1490, 1960.

Figures

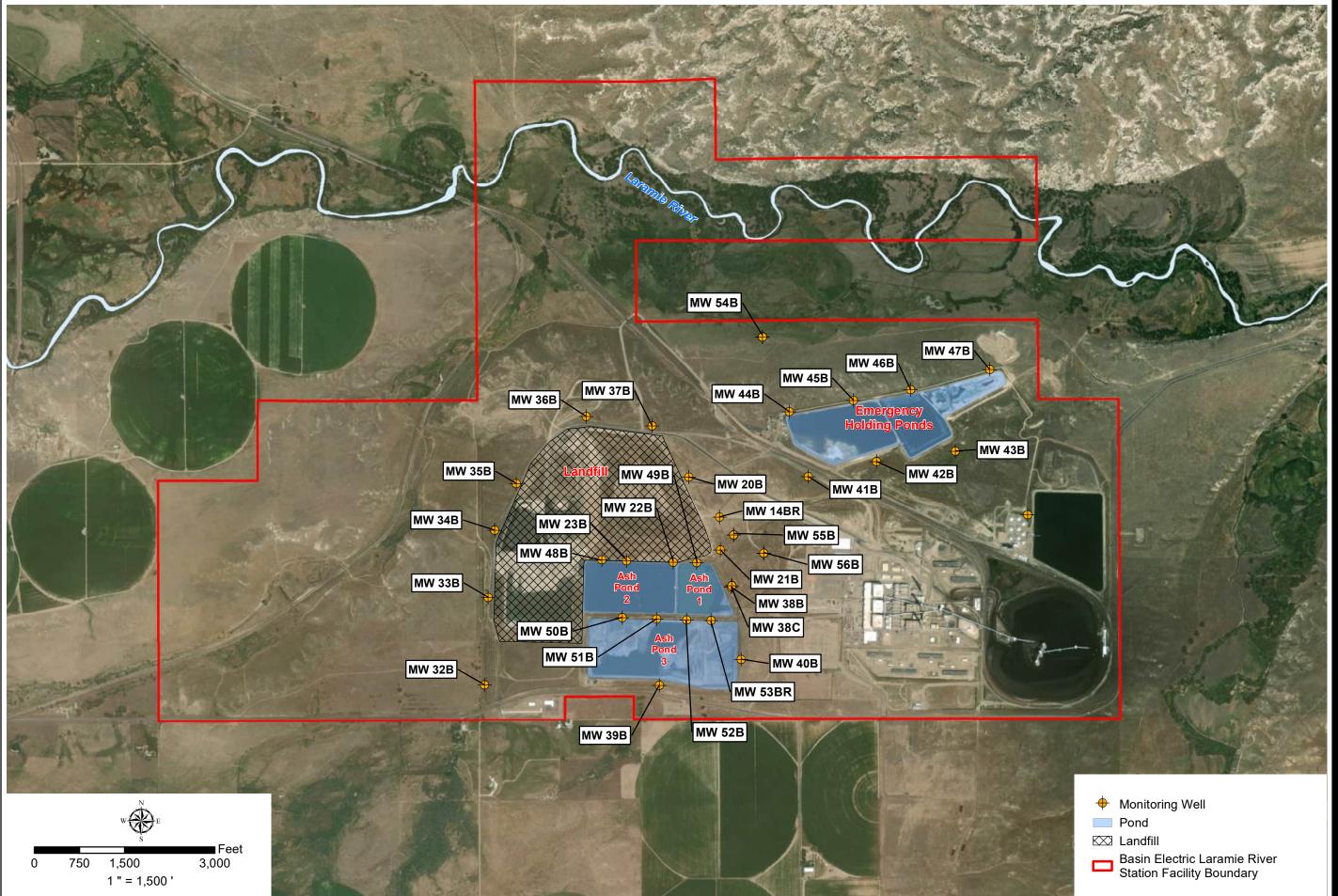


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ATCOM Figure: 1-1

Site Location Map

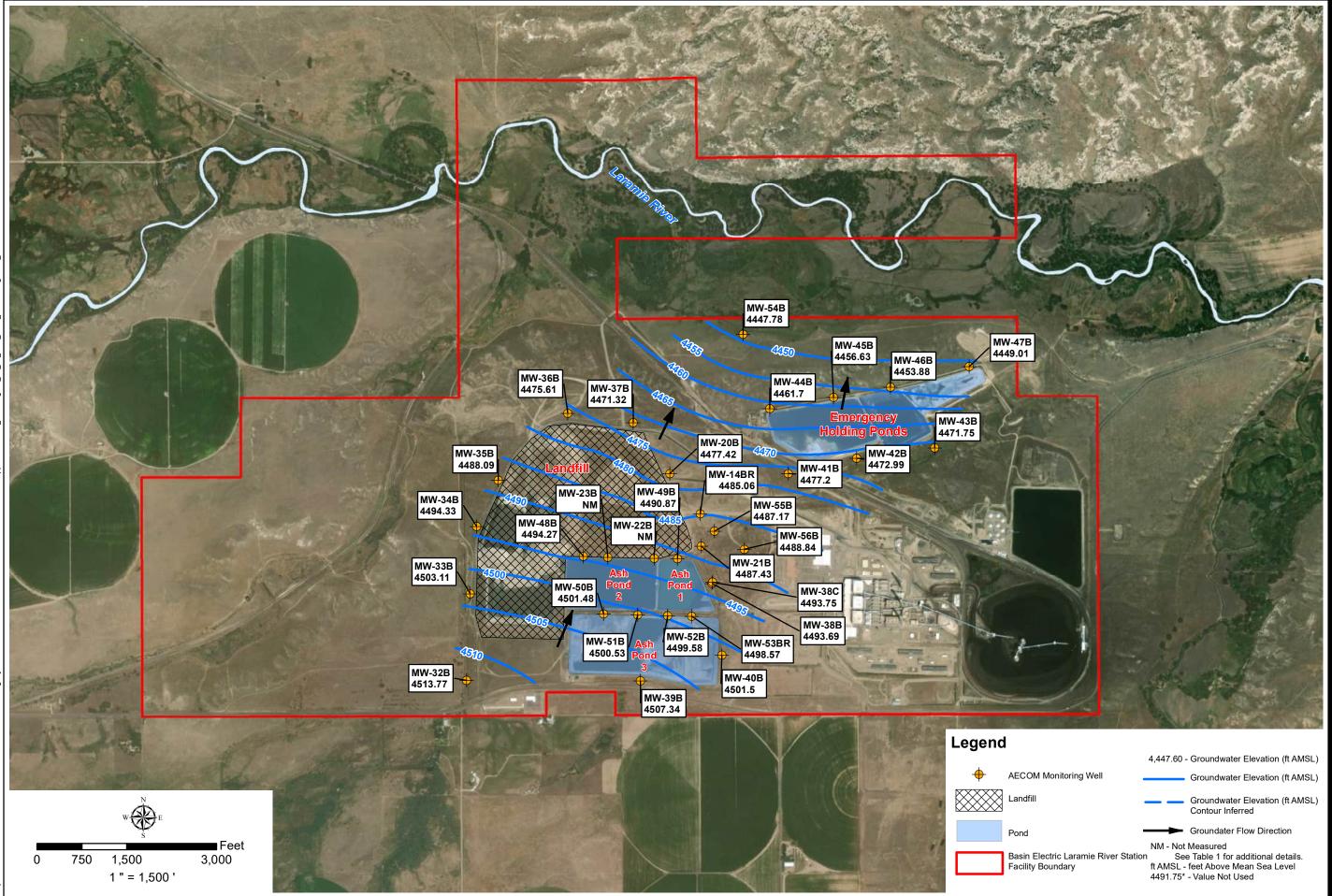
Basin Electric Laramie River Station Platte County, Wyoming Project No.: 60506860 Date: 09/28/2016



AECOM Figure: 4-1

LRS CCR Monitoring Well Network

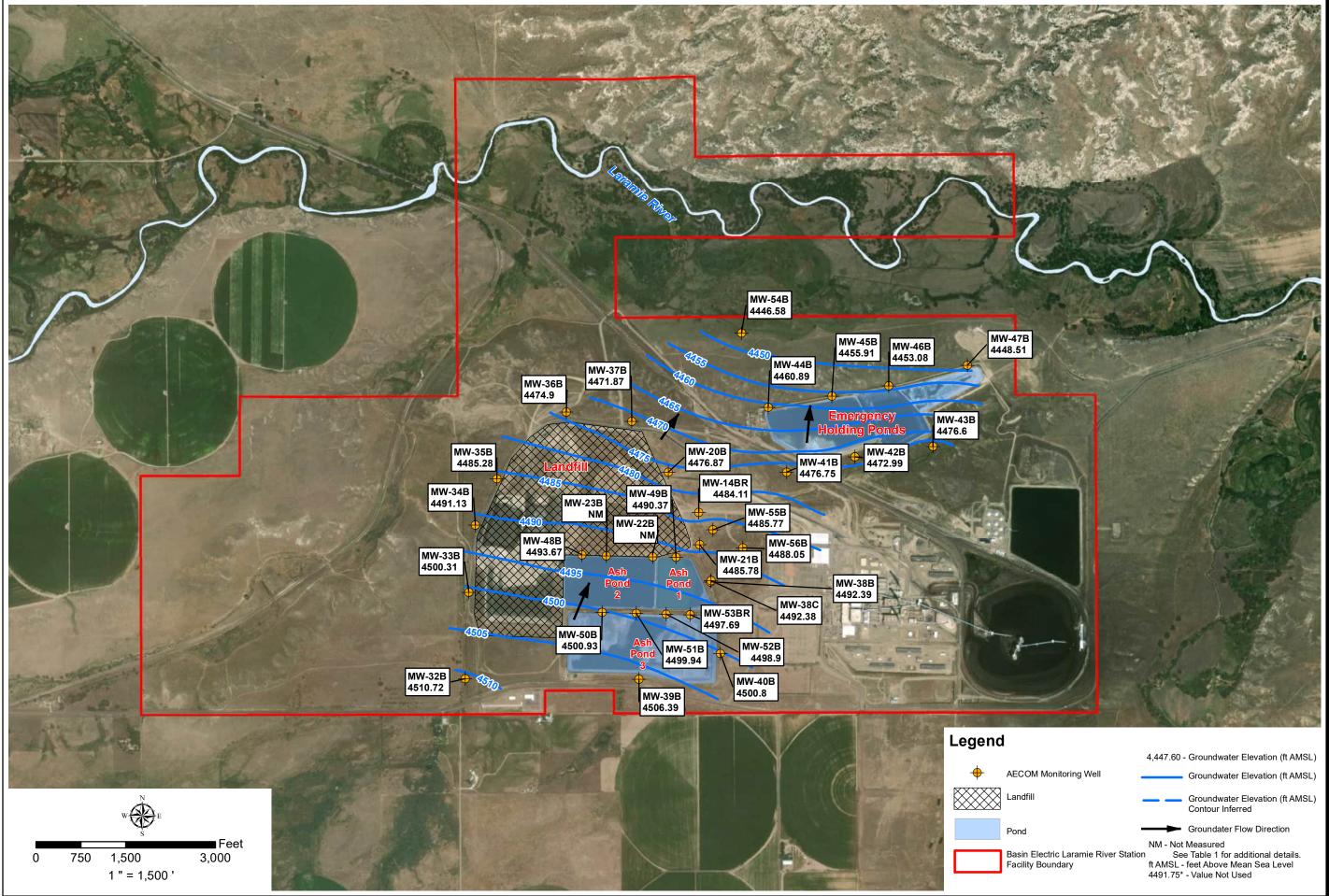
Basin Electric Laramie River Station Platte County, Wyoming Project No.: 60732883 Date: 12/30/2024



AECOM Figure: 5-1

Potentiometric Surface Map June 4-5, 2024

Basin Electric Laramie River Station Platte County, Wyoming Project No.: 60732883 Date: 12/30/2024



AECOM Figure: 5-2

Potentiometric Surface Map September 26-27, 2024

Basin Electric Laramie River Station Platte County, Wyoming Project No.: 60732883 Date: 12/30/2024

Tables

 Table 4-1:
 Monitoring Well Construction Details

Well	Year of	State Plane V Coord		Top of Casing Elevation ¹	Ground Surface Elevation	Total Depth	Well Screen Interval	Well Screen	Coordinate System
Name	Construction	Northing	Easting	(ft amsl)	(ft amsl)	(ft bgs)	(ft bgs)	Lithology	(Elevation)
Ash Ponds/	Landfill								
MW-14BR	2001	587906.6	730355.4	4540.51	4537.14	72.0	62-72	Silty Sandstone	NAVD88
MW-20B	1982	587906.6	729846.4	4535.47	4534.41	73.0	43.3-73	Silty Sandstone	NGVD29
MW-21B	1982	587358.2	730371.0	4539.58	4538.54	80.0	39.3-79	Sandstone	NGVD29
MW-22B**	1982	587159.1	729587.2	4569.21	4565.87	96.0	66.3-96	Sandstone	NGVD29
MW-23B**	1982	587179.9	728811.2	4569.11	4565.07	90.0	60.2-90	Silty Sandstone	NGVD29
MW-32B	2016	585117.1	726449.5	4569.72	4567.54	75.0	55-75	Sand with Silt	NAVD88
MW-33B	2016	586570.9	726509.5	4569.22	4581.97	89.0	65-85	Sand with Silt	NAVD88
MW-34B	2016	587689.7	726621.3	4557.33	4566.91	89.0	66-86	Sand with Silt	NAVD88
MW-35B	2016	588465.4	726979.2	4551.28	4554.76	90.0	66-86	Silty Sand	NAVD88
MW-36B	2016	589573.8	728145.6	4535.05	4548.80	79.0	58-78	Sand with Silt	NAVD88
MW-37B	2016	589424.0	729236.4	4532.98	4532.87	79.0	57.5-77.5	Sand with Silt	NAVD88
MW-38B	2016	586742.1	730549.7	4550.09	4530.69	88.0	55-75	Sand with Silt	NAVD88
MW-38C	2019	586762.0	730563.0	4549.45	4547.31	130.0	114-125	Sand with Silt	NAVD88
MW-39B	2016	585111.6	729357.3	4584.06	4546.54	109.3	89.3-109.3	Sand with Silt	NAVD88
MW-40B	2016	585540.5	730716.8	4592.20	4590.01	108.0	87.9-107.9	Sand with Silt	NAVD88
MW-48B	2017	587197.1	728402.3	4571.27	4569.39	100.0	80-100	Sand with Silt	NAVD88
MW-49B	2017	587153.4	729978.4	4566.97	NM	100.0	80-100	Sand and Sandstone	NAVD88
MW-50B*	2017/2024	586231.9	728742.5	4593.48	4593.84	120.0	100-120	Sand with Silt	NAVD88
MW-51B*	2017/2024	586215.1	729312.9	4593.14	4393.51	120.0	100-120	Sand with Silt	NAVD88
MW-52B*	2017/2024	586198.0	729809.7	4593.30	4393.65	125.0	104.6-124.6	Sand with Silt	NAVD88
MW-53BR	2024	586183.8	730230.5	4592.89	4593.523	120.0	100-120	Sandstone	NAVD88
MW-54B	2019	590900.5	731071.9	4454.80	4452.16	25.0	12.5-24.5	Sand with Silt	NAVD88
MW-55B	2019	587609.3	730593.4	4532.37	4529.77	70.0	50-70	Sand with Silt	NAVD88
MW-56B	2019	587310.4	731092.3	4541.95	4539.57	79.0	59-79	Sand with Silt	NAVD88
Emergency	Holding Ponds								
MW-41B	2016	588577.1	731829.2	4532.25	4529.99	79.0	53-73	Sand with Silt	NAVD88
MW-42B	2016	588829.6	732965.3	4518.44	4515.91	69.0	48.5-68.5	Sand with Silt	NAVD88
MW-43B	2016	589002.8	734274.6	4504.05	4500.61	79.0	58.5-78.5	Sand with Silt	NAVD88
MW-44B	2016	589659.8	731518.4	4532.00	4529.93	99.0	72.1-92.1	Sand with Silt	NAVD88
MW-45B	2016	589851.3	732581.2	4533.53	4531.27	89.0	69-89	Silty Sand	NAVD88
MW-46B	2016	590022.2	733532.2	4530.33	4527.94	94.0	73-93	Sand with Silt	NAVD88
MW-47B	2016	590358.4	734848.3	4525.21	4523.04	89.0	69-89	Silty Sand	NAVD88

Notes:

ft amsl = feet above mean sea level

ft bgs = feet below ground surface

NM = not measured

WY = Wyoming

* = new surface completions installed in 2024. wells were resurveyed. Ground surface elevation equals top of concrete pad elevation. ** = removed from well network, well damaged during Pond retrofit activities.

1 = Ground surface elevations re-evaluated and updated in 2024.

Well ID	Test Type	Aquifer Thickness (ft)	Hydraulic Conductivity (ft/day)	Transmissivity (ft ² /d)
MW-33B	Slug In	16.36	3.80	62.17
10100-330	Slug Out	10.50	2.27	37.14
MW-35B	Slug In	19.96	3.50	69.86
WW-35D	Slug Out	19.90	1.72	34.33
MW-37B	Slug In	16.14	6.04	97.49
WW-37B	Slug Out	10.14	6.28	101.36
MW-38B	Slug In	15.83	0.99	15.67
WW-30D	Slug Out	15.65	1.09	17.25
MW-39B	Slug In	25.17	0.45	11.33
10100-390	Slug Out	23.17	0.55	13.84
MW-42B	Slug In	21.62	1.23	26.59
10100-420	Slug Out		1.28	27.67
MW-45B	Slug In	12.80	1.27	16.26
10100-430	Slug Out	12.00	2.35	30.08
MW-47B	Slug In	12.53	1.11	13.91
10100-47D	Slug Out	12.00	1.36	17.04
MW-32B	Pumping	12.59	1.29	16.21
MW-34B	Pumping	16.62	0.65	10.88
MW-36B	Pumping	14.97	2.42	36.22
MW-40B	Pumping	11.39	0.79	8.99
MW-41B	Pumping	15.03	3.12	46.84
MW-43B	Pumping	20.00	0.75	14.90
MW-44B	Pumping	19.69	1.40	27.59
MW-46B	Pumping	17.10	0.76	13.00

Table 4-2: Aquifer Test Results

Notes:

ft = feet ft/day = feet per day ft^2/d = squared feet per day

ID = Identification

Location ID	Date Gauged	TOC Elevation (ft amsl)	Depth to Water (TOC ft)	Water Level Elevation (ft amsl)
MW-14BR	6/5/2024	4540.51	55.45	4,485.06
MW-20B	6/5/2024	4535.47	58.05	4,477.42
MW-21B	6/5/2024	4539.58	52.15	4,487.430
MW-32B	6/5/2024	4569.72	55.95	4,513.77
MW-33B	6/5/2024	4569.22	66.11	4,503.11
MW-34B	6/5/2024	4557.33	63.00	4,494.33
MW-35B	6/5/2024	4551.28	63.19	4,488.09
MW-36B	6/5/2024	4535.05	59.44	4,475.61
MW-37B	6/5/2024	4532.98	61.66	4,471.32
MW-38B	6/5/2024	4550.09	56.40	4,493.69
MW-38C	6/5/2024	4549.45	55.70	4,493.75
MW-39B	6/5/2024	4584.06	76.72	4,507.34
MW-40B	6/5/2024	4592.20	90.70	4,501.50
MW-41B	6/4/2024	4532.25	55.05	4,477.20
MW-42B	6/4/2024	4518.44	45.45	4,472.99
MW-43B	6/4/2024	4504.05	32.30	4,471.75
MW-44B	6/4/2024	4532.00	70.30	4,461.70
MW-45B	6/4/2024	4533.53	76.90	4,456.63
MW-46B	6/5/2024	4530.33	76.45	4,453.88
MW-47B	6/4/2024	4525.21	76.20	4,449.01
MW-48B	6/5/2024	4571.27	77.00	4,494.27
MW-49B	6/5/2024	4566.97	76.10	4,490.87
MW-50B	6/5/2024	4593.48	92.00	4,501.48
MW-51B	6/5/2024	4593.14	92.61	4,500.53
MW-52B	6/5/2024	4593.30	93.72	4,499.58
MW-53BR [*]	6/5/2024	4592.89	94.32	4,498.57
MW-54B	6/5/2024	4454.80	7.02	4,447.78
MW-55B	6/5/2024	4532.37	45.20	4,487.17
MW-56B	6/5/2024	4541.95	53.11	4,488.84

Table 5-1:	Groundwater	Elevations	- June	4-5. 2024
	•••••••••••••••••••••••••••••••••••••••			,

Notes:

TOC = top of casing

ft amsl = feet above mean sea level

ID = Identification

NM¹ = Not Measured - Well Not Found

NM = Not Measured

* = MW-53B replaced with MW-53BR in April 2024

Vertical Datum: North American Vertical Datum of 1988 (NAVD 88)

Location ID	Date Gauged	TOC Elevation (ft amsl)	Depth to Water (TOC ft)	Water Level Elevation (ft amsl)
MW-14BR	9/26/24 - 9/27/24	4540.51	56.40	4,484.11
MW-20B	9/26/24 - 9/27/24	4535.47	58.60	4,476.87
MW-21B	9/26/24 - 9/27/24	4539.58	53.80	4,485.780
MW-32B	9/26/24 - 9/27/24	4569.72	59.00	4,510.72
MW-33B	9/26/24 - 9/27/24	4569.22	68.91	4,500.31
MW-34B	9/26/24 - 9/27/24	4557.33	66.20	4,491.13
MW-35B	9/26/24 - 9/27/24	4551.28	66.00	4,485.28
MW-36B	9/26/24 - 9/27/24	4535.05	60.15	4,474.90
MW-37B	9/26/24 - 9/27/24	4532.98	61.11	4,471.87
MW-38B	9/26/24 - 9/27/24	4550.09	57.70	4,492.39
MW-38C	9/26/24 - 9/27/24	4549.45	57.07	4,492.38
MW-39B	9/26/24 - 9/27/24	4584.06	77.67	4,506.39
MW-40B	9/26/24 - 9/27/24	4592.20	91.40	4,500.80
MW-41B	9/26/24 - 9/27/24	4532.25	55.50	4,476.75
MW-42B	9/26/24 - 9/27/24	4518.44	45.45	4,472.99
MW-43B	9/26/24 - 9/27/24	4504.05	27.45	4,476.60
MW-44B	9/26/24 - 9/27/24	4532.00	71.11	4,460.89
MW-45B	9/26/24 - 9/27/24	4533.53	77.62	4,455.91
MW-46B	9/26/24 - 9/27/24	4530.33	77.25	4,453.08
MW-47B	9/26/24 - 9/27/24	4525.21	76.70	4,448.51
MW-48B	9/26/24 - 9/27/24	4571.27	77.60	4,493.67
MW-49B	9/26/24 - 9/27/24	4566.97	76.60	4,490.37
MW-50B	9/26/24 - 9/27/24	4593.48	92.55	4,500.93
MW-51B	9/26/24 - 9/27/24	4593.14	93.20	4,499.94
MW-52B	9/26/24 - 9/27/24	4593.30	94.40	4,498.90
MW-53BR [*]	9/26/24 - 9/27/24	4592.89	95.20	4,497.69
MW-54B	9/26/24 - 9/27/24	4454.80	8.22	4,446.58
MW-55B	9/26/24 - 9/27/24	4532.37	46.60	4,485.77
MW-56B	9/26/24 - 9/27/24	4541.95	53.90	4,488.05

Table 5-2: Groundwater Elevations - September 26-27, 2024

Notes:

TOC = top of casing

ft amsl = feet above mean sea level

ID = Identification

 NM^1 = Not Measured - Well Not Found

NM = Not Measured

* = MW-53B replaced with MW-53BR in April 2024

Vertical Datum: North American Vertical Datum of 1988 (NAVD 88)

Table 6-1: Proposed Statistical Methods for Appendix III Constituents in Background Wells

CCR Unit/Multi-unit	Background Wells	Statistical Method	Constituents
Ash Pond 1	MW-52B, MW-53B	Parametric Prediction Interval	Boron, Calcium, Fluoride, pH
	MW-52B, MW-53B	Nonparametric Prediction Interval	Chloride, Sulfate, Total Dissolved Solids
Ash Pond 2, Ash Pond 3, Ash Landfill Multi-unit	MW-32B, MW-39B	Parametric Prediction Interval	Boron, Calcium, Fluoride, pH
	MW-32B, MW-39B	Nonparametric Prediction Interval	Chloride, Sulfate, Total Dissolved Solids
Emergency Holding Ponds Multi-unit	MW-41B, MW-42B, MW-43B	Parametric Prediction Interval	Calcium
	MW-41B, MW-42B, MW-43B	Nonparametric Prediction Interval	Boron, Chloride, Fluoride, pH, Sulfate, Total Dissolved Solids

Note:

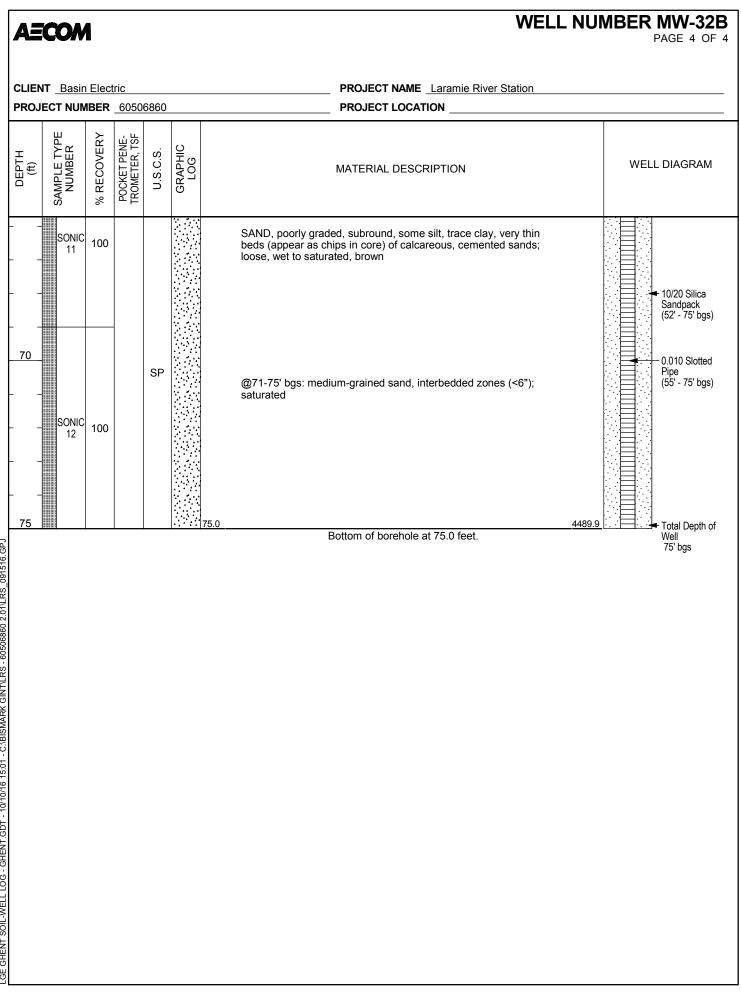
CCR = Coal Combustion Residuals

Appendix A Boring Logs and Well Construction Diagrams

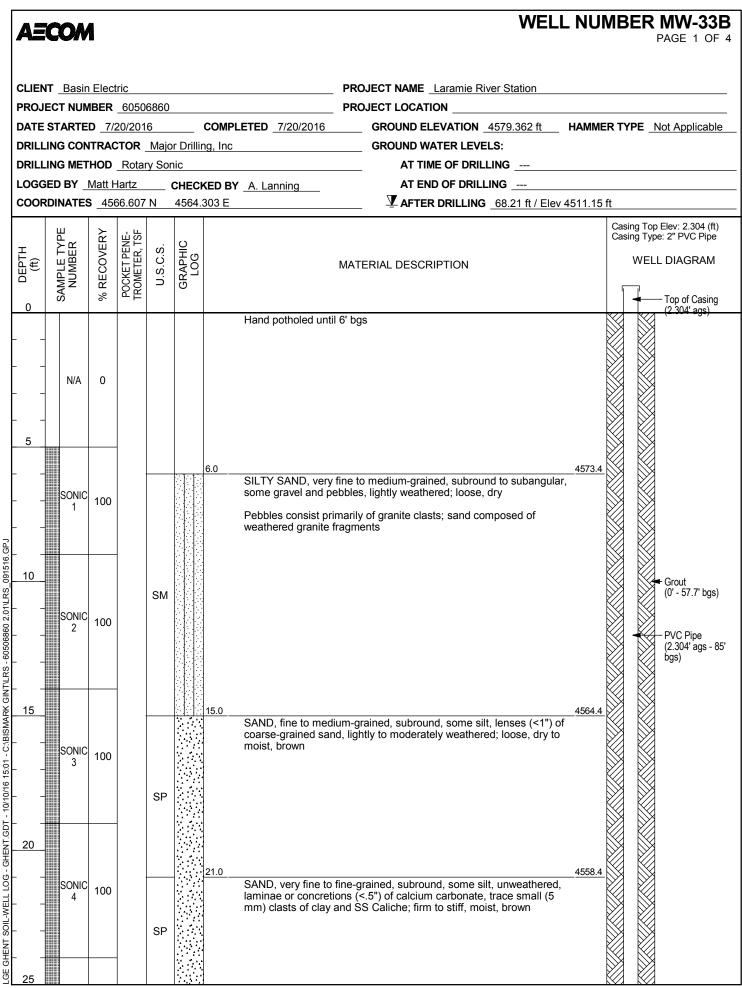
DATE STARTED _7/19, DRILLING CONTRACTO DRILLING METHOD _F LOGGED BY _Matt Han COORDINATES _4567.	50506860 /2016 COMPLETED _7/20, OR _Major Drilling, Inc Rotary Sonic rtz CHECKED BY _A. Lanning .106 N 4564.93 E	PROJECT NAME Laramie River Station PROJECT LOCATION /2016 GROUND ELEVATION 4564.93 ft GROUND WATER LEVELS: AT TIME OF DRILLING AT END OF DRILLING	HAMMER TYPE Not Applicable
DEPTH (ff) (ff) (ff) (ff) (ff) (ff) (ff) (ff	U.S.C.S. U.S.C.S. GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM Top of Casing (2.176' ags)
0 0 0 0 0 0 0 0 0 0 0 0 0 0	6.0	led until 6' bgs (fine to coarse-grained, some silt, some gravel and ty weathered to weathered; loose, dry, light gray-brown consists primarily of granite clasts (qtz, k-spar, lithies) st	

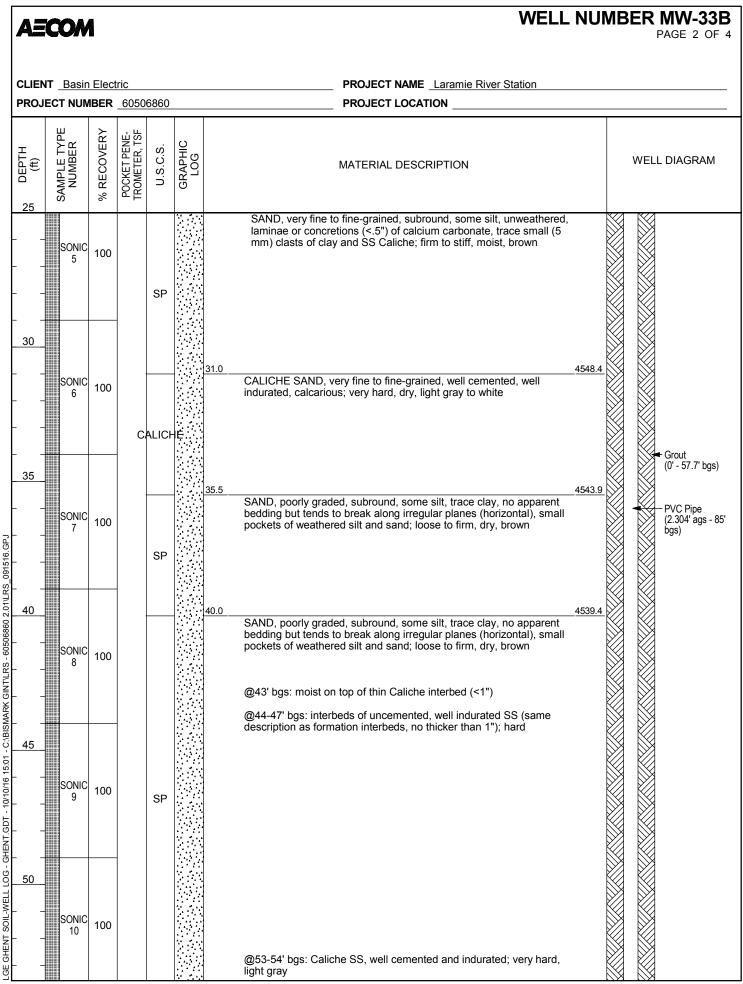
	CON						JMBER MW-32B PAGE 2 OF 4					
	T <u>Basir</u> ECT NUN											
(ff) 50	SAMPLE TYPE NUMBER	% RECOVERY		NUMBER % RECOVERY		U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM			
_20				SW		20.5 <u>4544</u>	.4					
 <u>25</u> 	SONIC 3	100		SP		SAND, very fine to fine-grained, subround, some silt, massive/no apparent bedding; firm to stiff, predominantly dry with moist intervals (typically on top of SS Caliche horizons), light brown @28-29' bgs: light gray SS Caliche, hard, well cemented						
30	SONIC 4	100				@30-33' bgs: intervals of very stiff (nearly SS), intervals contained calcium carbonate stringers and nodules (cemented sands) 33.0 4531	9					
						SANDSTONE, very fine to fine-grained, some silt, well indurated, laminae of Caliche silts and clays, interbeds of dense unconsolidated sands (same as 30-33' bgs interval); hard, moist,						
35	SONIC 5	100	SAN	IDST	ONE	light gray-brown	9					
						SAND, very fine to fine-grained, subround, some silt, trace clay, very thin beds (appear as chips in core) of calcareous, cemented						
<u>40</u>	SONIC 6	100		SP		sands; loose, dry to moist, light gray-brown @39-42' bgs: color change from light gray-brown to brown						

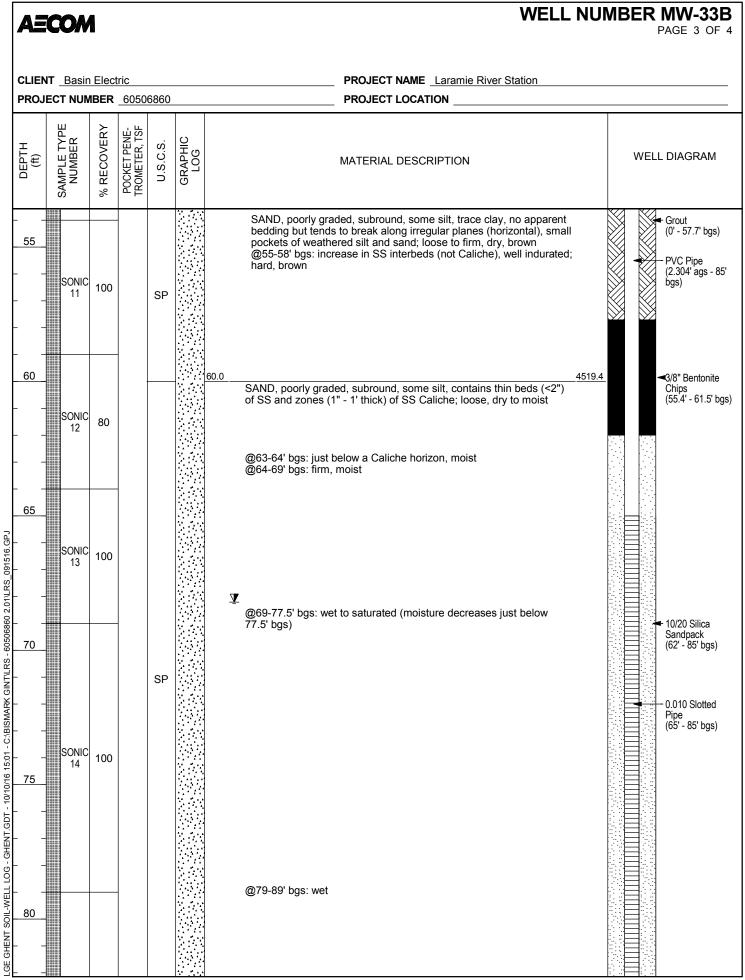
AECOM		WELL	NUMBER MW-32B PAGE 3 OF 4
CLIENT Basin Electric PROJECT NUMBER 6050		PROJECT NAME Laramie River Station PROJECT LOCATION	
DEPTH (ff) (ff) (ff) SAMPLE TYPE NUMBER % RECOVERY POCKET PENE- TROMETER, TSF	U.S.C.S. GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM
45 - - - - - - - - - - - - - - - - - - -	SP SP	SAND, poorly graded, subround, some silt, trace clay, very thin beds (appear as chips in core) of calcareous, cemented sands; loose, dry to moist, brown @43-49' bgs: dry @49-56' bgs: moist	 Grout (0' - 47' bgs) PVC Pipe (2.176' ags - 75 bgs) Solution <
55 - SONIC - SONIC 60 - SONIC - SONIC 100 - 100		 @56.5-57.5' bgs: 1' of SS Caliche, well cemented; very dense, moist on top and bottom of interval ✓ @59-60' bgs: wet to saturated 	 10/20 Silica Sandpack (52' - 75' bgs) 0.010 Slotted Pipe (55' - 75' bgs)
65			



LGE GHENT SOIL-WELL LOG - GHENT.GDT - 10/10/16 15:01 - C.\BISMARK GINTLRS - 60506860 2.01\LRS_091516.GPJ



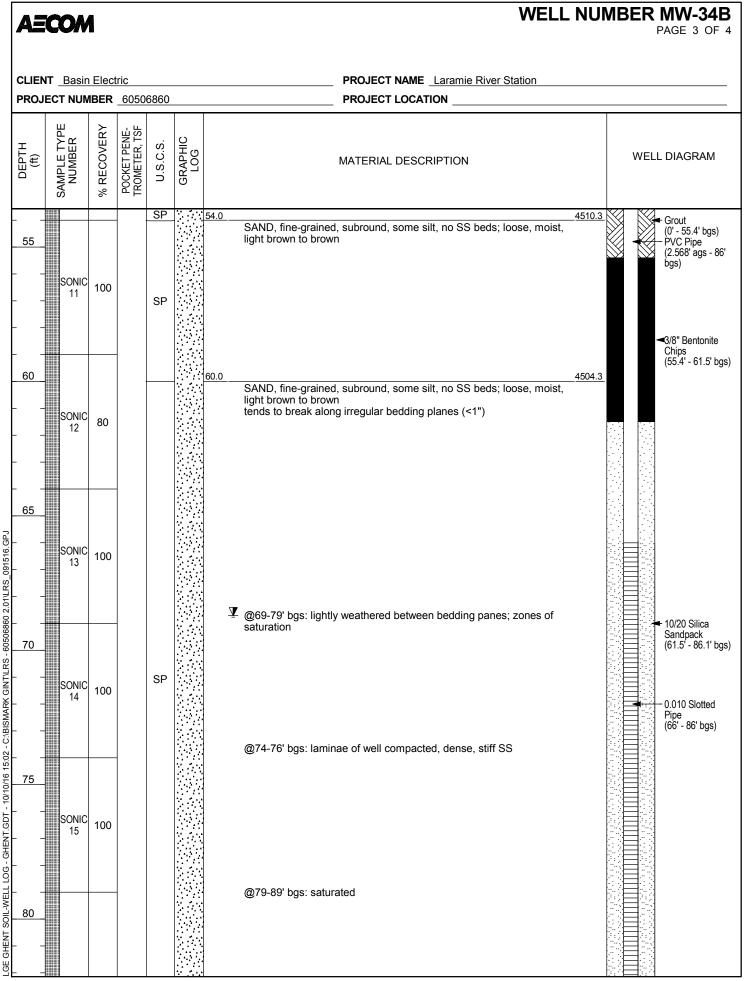




A	CON					WELL	NUMBER MW-33B PAGE 4 OF 4
	NT <u>Basin</u> ECT NUN			6860		PROJECT NAME Laramie River Station PROJECT LOCATION	
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM
 <u>85</u> 	- SONIC 15	100		SP		SAND, poorly graded, subround, some silt, contains thin beds (<2") of SS and zones (1" - 1' thick) of SS Caliche; loose, wet 89.0 Bottom of borehole at 89.0 feet.	4490.4

LIENT Basin Electric	WELL NUMBER MW-34 PAGE 1 OF PAGE 1 OF Laramie River Station
	016 GROUND ELEVATION 4564.303 ft HAMMER TYPE Not Applicable
RILLING CONTRACTOR _Major Drilling, Inc	
RILLING METHOD _ Rotary Sonic	
DGGED BY Matt Hartz CHECKED BY A. Lanning	
OORDINATES 4554.72 N 4552.152 E	
Ш ≻ . ц	Casing Top Elev: 2.568 (ft)
(ft) SAMPLE TYPE NUMBER % RECOVERY POCKET PENE- TROMETER, TSF U.S.C.S. GRAPHIC LOG	Casing Type: 2" PVC Pipe MATERIAL DESCRIPTION
0 Hand pothole	Top of Casing
- N/A 0 5	4558.3 ine to fine-grained, subround, some silt, trace gravel loose, dry, light brown
SONIC 100 SP SONIC 100 FILL, sand, gr loose, dry, light	4555.3 ravel, pebbles, and cobbles, angular to subangular; ht gray
- SONIC 100 - FILL	(0' - 55.4' bgs) PVC Pipe (2.568' ags - 8 bgs)
I5 SONIC 3 100	4548.3 ine to fine-grained, subround, some silt, no apparent rbedded SS (<1"), unweathered; loose to firm and firm
to stiff at dept	th, dry, light brown silt interbed, light gray
SONIC 4 100	
25 @24-25.5' bg (perched, not	is: slight moisture increase just above SS interbed holding significant water year-round)

	IT <u>Bas</u> ECT NI				6860			PROJECT NAME Laramie River Station PROJECT LOCATION					
(1) (1) 25	SAMPLE TYPE NUMBER	% RECOVERY		POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM					
	- SONI 5	C 10	0				SAND, very fine to fine-grained, subround, some silt, no apparent bedding, interbedded SS (<1"), unweathered; loose to firm and firm to stiff at depth, dry, light brown						
- 30 - -	SON 6	C 10	0				@30-36' bgs: SS beds increase in frequency; firm to stiff						
35	SON 7	C 10	0				@39-44' bgs: moisture content increases above stiff sand interval	- Grout (0' - 55.4' bgs - PVC Pipe (2.568' ags - 4 bgs)					
<u>40</u> - -	SON 8	C 10	0		SP								
-	SON 9	C 10	0				@46-50' bgs: thin SS interbed above zone of saturation; wet to saturated						
<u>50</u> - -	SON 10	C 10	0				@53-54' bgs: SS intebed, cemented, possibly Caliche; hard, dry,						



A	CON					WELL N	WELL NUMBER MW-34B PAGE 4 OF 4					
	NT <u>Basin</u>			6860		PROJECT NAME Laramie River Station PROJECT LOCATION						
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM					
 85 	. SONIC 16	100		SP		SAND, fine-grained, subround, some silt, no SS beds; loose, saturated, light brown to brown tends to break along irregular bedding planes (<1") 89.0 447 Bottom of borehole at 89.0 feet.	 10/20 Silica Sandpack (61.5' - 86.1' bgs) 0.010 Slotted Pipe (66' - 86' bgs) Total Depth of Well 86' bgs Native Clay Below Well - Natural Collapse 					

420	CON					WELL	NUI	MBER N	(W-351 AGE 1 OF
	Basin					PROJECT NAME Laramie River Station			
						PROJECT LOCATION			
						COMPLETED <u>8/1/2016</u> GROUND ELEVATION <u>4552.152 ft</u> I	HAMME	ER TYPE No	t Applicable
RILLI	NG CON	TRAC	CTOR _	Majo	r Drillin	g, Inc GROUND WATER LEVELS:			
	NG MET					AT TIME OF DRILLING			
OGGE	D BY	Chris	Ahrend	<u>it</u> (CHECK	ED BY _A. Lanning AT END OF DRILLING			
COORD	DINATES	3 _454	48.665	N	4546.1	85 E ¥ AFTER DRILLING _63.70 ft / Elev 4	488.45	ft	
UEPIN (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION		 	
0						Hand potholed until 6' bgs			2.480' ags)
	N/A SONIC 2 SONIC 3	75		SP- GP SP- SP- SP-		5.0 SAND, poorly graded, with subangular gravel, trace fines, no odor or staining; very loose, moist, brown (10YR 5/5) 9.0 S.A.A., round gravel 11.0 SAND, poorly graded, with subangular gravel and pebbles, little fines, no odor or staining; very loose, moist, light brownish gray (10YR 6/2)	<u>4546.2</u> <u>4543.2</u> <u>4541.2</u>	F	Grout 0' - 59.5' bgs) 2VC Pipe 2.480' ags - 80 gs)
			-	GP	∘ ()) ⊘	14.4	4537.8		5-7
15			[SP		SAND, very fine to fine-grained, traces fines and gravel; very loose, dry, light gray (10YR 7/1)	4537.1		
	SONIC 4	76		SW		SAND, very fine to coarse-grained, little subangular gravel, trace fines; very loose, moist, light gray (10YR 7/1) @16.9-17.8' bgs: increasing fines, little silt	4533.2		
20				SM		SILTY SAND, very fine to fine-grained, noncohesive, no odor or staining; very loose, moist, grayish brown (10YR 5/2) 20.4	4531.8		
	SONIC 5	100		ML		SILT, with dark gray, fractured, blocky claystone (1"), trace sand, no odor or staining; soft, moist (sticky when wet), light gray			
				ML		24.0 SILT, noncohesive, nonplastic, no oxidation or staining or odor; very soft, moist, pale brown (10YR 6/3)	4528.2		

	T <u>Basin</u> ECT NUM					PROJECT NAME Laramie River Station PROJECT LOCATION						
н (II) 25	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION		WELL DIAGRAM				
	SONIC	100		ML		SILT, noncohesive, nonplastic, no oxidation or staining or odor; very soft, moist, pale brown (10YR 6/3)	4505.0					
-				ML		27.0 SILT, trace very fine sand, noncohesive, nonplastic, no oxidation or staining or odor, blocky sturctures (1") that break in hand; very stiff, moist, dark gray (10YR 4/1)	4525.2					
<u>30</u> - - -	SONIC 7	100		SM		30.0 SILTY SAND, very fine to fine-grained, slow dilatancy, no odor or staining; soft, wet, dark yellowish brown @30-35' bgs: perched aquifer	4522.2	Grout				
35			SAN		DNE	34.7 35.0 SANDSTONE, no odor or staining; hard, dry SANDY SILT, with gravel, slow dilatancy, noncohesive, nonplastic,	4517.5 4517.2	(0' - 59.5' bg				
-	SONIC 8	100		ML ML		<u>36.2</u> no odor or staining; very soft, wet, gray (10YR 5/1) SILT, with gravel (15% subangular standstone), noncohesive, nonplastic; loose, dry, white (10YR 6/1)	4516.0	PVC Pipe (2.480' ags - bgs)				
_				SP		38.3 39.0 SAND, poorly graded, little fines, with thinnly bedded sandstone lenses (.5"-1"), no odor or staining; loose, dry, pale brown (10YR	4513.9 4513.2					
40				SM		6/3) SILTY SAND, very fine to fine-grained, slow dilatancy, no odor or staining; medium dense, wet (perched aquifer), brown (10YR 5/3)						
_	SONIC 9	100		SM		41.5 S.A.A., 10% of 3/4"-1" blocky sandstone; moist, light gray (10YR 7/2)	4510.7					
45				SP- SM		44.0 SAND, very fine to fine-grained, with silt, with 3/4"-1" hard sandstone, no odor or staining; loose, moist, brown	4508.2					
_	SONIC	100		SM		46.1 46.4 SILTY SAND, very fine to fine-grained; very hard, moist, dark gray SAND, very fine to fine-grained, with silt, with 3/4"-1" hard	4506.1 4505.8					
-						@49-50.1' bgs: 1" hard fragments of sandstone; wet (due to drilling waters)						
50	SONIC	94		SP								

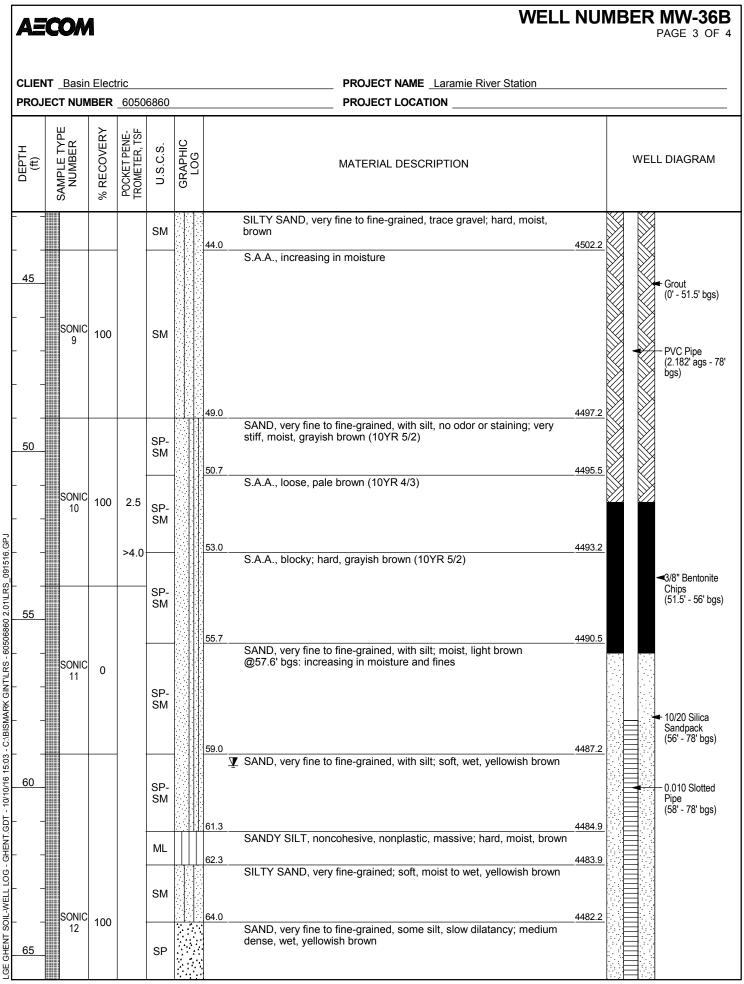
	NT Bas	in <u>E</u> lec	tric			PROJECT NAME Laramie River Station						
				6860		PROJECT NAME Laramie River Station PROJECT LOCATION						
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION			WEL	L DIAGRAM		
_				SP	54.		4498.2		\mathbb{K}			
55				SP		SAND, very fine to fine-grained, with silt, no odor or staining; medium dense, wet, yellowish brown (10YR 5/4), mottled dark gray (10YR 4/1) 7	4496.5			 Grout (0' - 59.5' bgs) 		
-	SONI 12	^C 100		SP	57.	SAND, very fine to fine-grained; medium dense, moist, light brownish gray (10YR 6/1)	4495.1			— PVC Pipe		
-			_	ML	58.	SANDY SILT, slow dilatancy, noncohesive, nonplastic; soft, wet,	4494.2			(2.480' ags - 8 bgs)		
- 60				SM		brown (10YR 5/2)						
					<u>60.</u>	5 S.A.A., soft, decreasing in moisture	4491.7					
-	. SONI 13	70		SM						◄3/8" Bentonite Chips (59.5' - 64' bgs		
- 65			-	SP-	64.	$\frac{\Psi}{SAND}$, very fine to fine-grained, with silt; soft, wet, brown (10YR 5/3)	4488.2					
-			4.0	SM TSTC	65.	7 SILTSTONE, massive, blocky; hard, moist, brown (10YR 4/3)	4486.5					
-	SONI 14	100	SIL		NIEX * X X X67. X X X X X X	0 S.A.A., brown (10YR 5/3)	4485.2					
-			0	тотс						- 40/00 0'l's -		
70				TSTC	× × × × × × × × × × × × × × × × × × ×					 10/20 Silica Sandpack (64' - 86' bgs) 		
-	SONI 15	^C 100	2.0	SM	× × 71.	SILTY SAND; stiff, moist, brown	4481.2					
-	10		SAN	IDST	DNE 72.		<u>4480.2</u> 4479.7			- 0.010 Slotted		
_				SM	74.	SILTY SAND, very fine to fine-grained; medium dense, moist, brown	4478.2			Pipe (66' - 86' bgs)		
- 75]	SM		S.A.A., moist						
<u>, , , , , , , , , , , , , , , , , , , </u>				SM	<u>75.</u>	SILTY SAND, very fine to fine-grained, slow dilatancy; soft, wet, brown	4477.2					
_	SONI 16	^C 100			77.	0 S.A.A., medium dense, moist	4475.2					
-			>4.0	SM	78.		4473.9					
-				SM	79.	S.A.A., stiff SANDY SILT, noncohesive, nonplastic, massive, slow dilatancy;	4473.2					
80						very soft, wet, grayish brown		E	=			

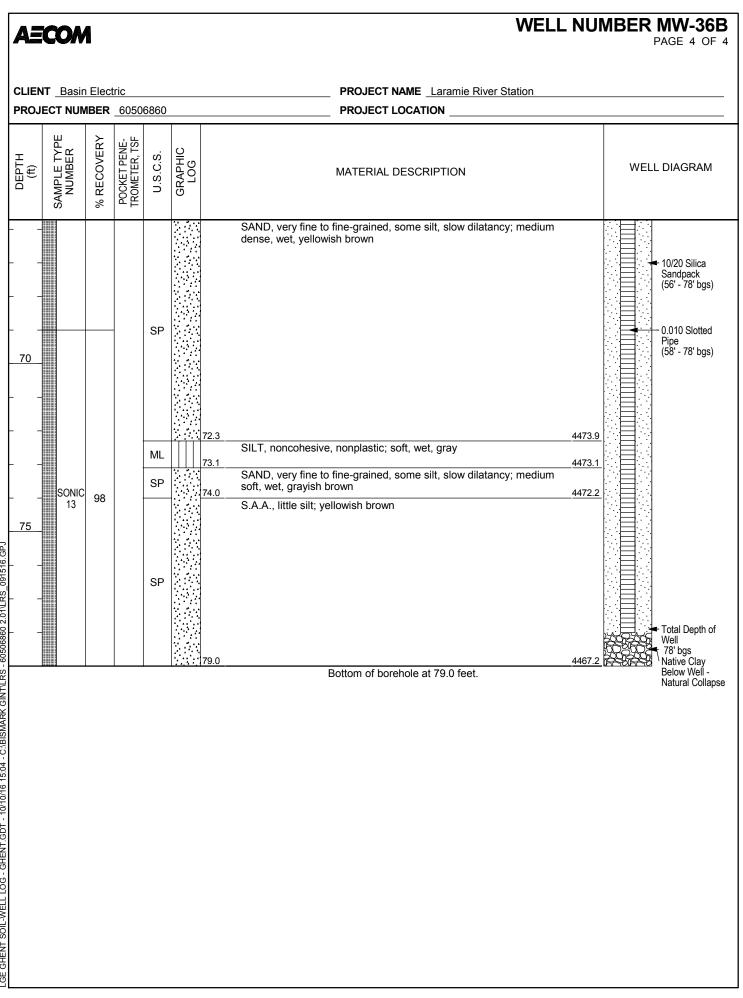
A	CON					WELL NU	MBER MW-35B PAGE 4 OF 4
	NT <u>Basin</u> ECT NUN			6860		PROJECT NAME Laramie River Station PROJECT LOCATION	
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF U.S.C.S. GRAPHIC LOG			MATERIAL DESCRIPTION	WELL DIAGRAM
 <u>85</u> 90		75	-	ML		SANDY SILT, noncohesive, nonplastic, massive, slow dilatancy; very soft, wet, grayish brown <u>85.5</u> SILTY SAND, very fine to fine-grained, slow dilatancy, no odor or staining; loose, wet, brown Note: drillers drilled to 89', but all of it fell back into the hole because outer casing was not used. <u>90.0</u> <u>4462</u> .	Total Depth of Well 86' bgs Solution Solut

Bottom of borehole at 90.0 feet.

A	:CON	1								WEL	L NUI	MBER	MW-36B PAGE 1 OF 4
CLIEN	NT Basin	Elect	tric					_ PROJ	ECT NAME _Laramie F	River Station			
DRILI		ITRAC	TOR	Majo	or Drilling, Ir	IC			GROUND ELEVATION GROUND WATER LEV $\overline{\Sigma}$ AT TIME OF DRI	ELS:			
							anning		AT END OF DRI				
COOF	RDINATES	3 _453	32.438	Ν	4530.256 I	Ξ				G _59.35 ft / Elev	v 4486.84	ft	
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG			MATEI	RIAL DESCRIPTION			Casing Typ	DELEV: 2.182 (ft) DE: 2" PVC Pipe LL DIAGRAM
0	Ś	%	<u></u> ч н			<u> </u>							 Top of Casing (2.182' ags)
01/LRS_091516.GPJ	- N/A	0		SW- GP	5.0	SANE	potholed un), fine to coa , moist brow	nse-grain	ed, with gravel, no odo	r or staining;	<u>4541.2</u> 4537.2		- Court
LGE GHENT SOIL-WELL LOG - GHENT.GDT - 10/10/16 15:03 - C.\BISIMARK GINTLRS - 60506860 2.01LRS_ 01 01 02 02	- SONIC 2	100		SW- GP					ontent, subround; too h		4532.2		 Grout (0' - 51.5' bgs) PVC Pipe (2.182' ags - 78' bgs)
HENT SOIL-WELL LOG - GHENT.GDT - 10/10/1	- SONIC 3	96		SM GP	°°° °°° °°°	GRAN odor o	, brown (10Y /EL, poorly g or staining; k	graded, s pose, mo	ubround (1-2"), little cla ist to wet, brown	y, little sand, no	4528.4 4527.2		
0 10 20				SM					in (10YR 5/3)	,			

	15	CON					WELL NU	MBER MW-36B PAGE 2 OF 4
		T <u>Basin</u> ECT NUN			6860		PROJECT NAME Laramie River Station PROJECT LOCATION	
	(1) (1)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM
-	-	SONIC 4	94		SM		SILTY SAND, very fine to fine-grained, trace gravel, no odor or staining; loose, moist, brown (10YR 5/3)	
_	- 25 - -	SONIC 5	98		SM	29.0	4522. S.A.A., little gravel 4517.:	
RS - 60506860 2.01\LRS_091516.GPJ	30	SONIC 6	100	SAN	ML IDST(32.0 DNE 32.8	SANDY SILT, noncohesive, nonplastic, blocky, no staining; very soft, moist, dark grayish brown (10YR 4/2) 4514.3 SANDSTONE, broken; dry to moist, white and tan 4513.4 SILTY SAND, very fine to fine-grained, little gravel, no odor or staining; medium dense, moist, pale brown (10YR 6/3)	PVC Pipe (2.182' ags - 78' bgs)
15:03 - C:\BISMARK GINT\L	- <u>35</u>	SONIC 7	100	0.75	ML SM	34.0 35.2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	4512.: SANDY SILT, noncohesive, nonplastic, massive; medium dense, moist, brown (10YR 4/3) SILTY SAND, very fine to fine-grained, no odor or staining; loose, moist, brown	
LGE GHENT SOIL-WELL LOG - GHENT.GDT - 10/10/16 15:03 - C.\BISMARK GINTLRS - 60506860 2.01\LRS_091516.GPJ	- - <u>40</u>	SONIC 8	69	SAN	ML ML	DNE 39.0	SILT, with sand, trace gravel, noncohesive, nonplastic, no odor or staining; soft, wet, brown SANDSTONE, with wet silt in between bedding, noncohesive, nonplastic; hard, moist to wet, brownish gray 4507.3 SILT, noncohesive, nonplastic, blocky, no oxidation; hard, moist, brown 4506.3 SILTY SAND, very fine to fine-grained, trace gravel; hard, moist, brown	





LGE GHENT SOIL-WELL LOG - GHENT.GDT - 10/10/16 15:04 - C.\BISMARK GINTLRS - 60506860 2.01\LRS_091516.GPJ

AECOM		WEL	L NUMBER MW-37B PAGE 1 OF 4
	506860	PROJECT NAME Laramie River Station PROJECT LOCATION /3/2016 GROUND ELEVATION _4530.256 ft	
DRILLING CONTRACTOR DRILLING METHOD _Rot	<u>Major Drilling, Inc</u> ary Sonic	GROUND WATER LEVELS:	
LOGGED BY <u>Chris Ahre</u> COORDINATES <u>4530.36</u>	ndt CHECKED BY <u>A. Lann</u> 7 N 4528.075 E	ing AT END OF DRILLING AFTER DRILLING AFTER DRILLING	
DEPTH (ft) SAMPLE TYPE NUMBER % RECOVERY POCKET PENE-	U.S.C.S. GRAPHIC LOG	MATERIAL DESCRIPTION	Casing Top Elev: 2.292 (ft) Casing Type: 2" PVC Pipe WELL DIAGRAM Top of Casing (2.292' ags)
0 0 0 0 0 0 0 0 0 0 0 0 0 0	SP- GP GP SW- GP SW- CO SW- CO SAND, v Ioose, mu SAND, v Staining; SW- CO SAND, v Staining; SAND, v Staining; SAND, v Staining; SAND, v Staining; SAND, v Staining; SAND, v Staining; SAND, v Staining; SILTY SJ	tholed until 5.5' bgs ery fine to medium-grained, with gravel, no odor or stainin oist, brown ery fine to coarse-grained, with subround gravel, no odor or loose, moist, brown, poorly graded, subangular, with fine to coarse-grained tee fines, no odor or staining; loose, moist, tan SILT, noncohesive, nonplastic, no odor or staining; loose, the brown AND, very fine-grained, trace fine grained sand, no odor or loose, moist, light brown	4524.8 g; 4521.3 or 4521.3 or 4518.5 4517.7 4516.3
	SM 20.0		4510.3

A	E	cow					WELL N	UMBE	PAGE 2 OF 4			
CL	JENT	Basin	l Elect	tric			PROJECT NAME Laramie River Station					
					6860		PROJECT LOCATION					
DEPTH		SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	١	WELL DIAGRAM			
							SILTY SAND, very fine-grained; loose, moist, brown					
-		SONIC 4	100		SM		21.745 SILT, little sand, noncohesive, nonplastic, blocky, no odor or staining; hard, moist, pale brown	08.6				
_	_				ML		24.0 45	06.3				
_2	5	SONIC	100		ML		SANDY SILT, noncohesive, nonplastic, no odor or staining, blocky; soft, moist, brown					
-		5			ML		S.A.A., slow dilatancy; wet (perched) 29.0 45	01.3				
1\LRS_091516.GPJ	<u>0</u>	SONIC			SM		SILTY SAND, very fine to fine-grained, no odor or staining; loose, wet (due to drilling waters), brown S.A.A., moist	<u>20.3</u>	Grout (0' - 50' bgs)			
S - 60506860 2.01\L		6	94		SM		32.944 S.A.A., blocky; hard	97.4	(2.292' ags - 77.5' bgs)			
SMARK GINT\LR	5				ML		SILT, with sand, noncohesive, nonplastic, slow dilatancy; soft, wet, grayish brown (10YR 5/2), mottled gray (10YR 5/1)	96.3				
/16 15:04 - C:\Bl		SONIC 7	100		SM		SILTY SAND, very fine to fine-grained, no odor or staining; soft, wet, brown	93.0				
ENT.GDT - 10/10					SM		S.A.A., decreasing moisture	91.3				
LGE GHENT SOIL-WELL LOG - GHENT.GDT - 10/10/16 15:04 - C:\BISMARK GINT\LRS - 60506860 2.0 	0	SONIC 8	100		SM		SILTY SAND, very fine to fine-grained, no staining; loose, moist, brown					
LGE								\square	\bowtie			

	IT <u>Basir</u> ECT NUM			6860		PROJECT NAME Laramie River Station PROJECT LOCATION		
UEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION		WELL DIAGRAM
				SM		SILTY SAND, very fine to fine-grained, no staining; loose, moist, brown		
45				SW	45.0	SAND, very fine to coarse-grained, with round gravel, no odor or	<u>4485.3</u> 4484.9	Grout (0' - 50' bgs)
_	SONIC 9	100		SM	49.0	staining; loose, wet (due to drilling waters), brown/ SILTY SAND, very fine to fine-grained, no odor or staining; loose, moist, brown	4481.3	PVC Pipe (2.292' ags - 77.5' bgs)
	SONIC			SP	51.0	SAND, very fine to fine-grained, little silt; stiff, moist, brown S.A.A., loose	4479.3	
_	10	100	1.25	SP				◄3/8" Bentonite Chips (50' - 54' bgs)
55				SP- SM	56.0	SAND, very fine to fine-grained, with silt, no odor or staining; loose, moist, brown	<u>4476.3</u>	- 10/20 Silica
	SONIC	88		SM	56.9	SILTY SAND, very fine to fine-grained; soft, wet, yellowish brown	4473.4	Sandpack (54' - 77.5' bgs)
-				SM		S.A.A., moist		0.010 Slotted
<u>60</u>				SP- SM	59.6	SAND, very fine to fine-grained, with silt; loose, moist, yellowish brown	4470.7	Pipe (57.5' - 77.5' bg
_	SONIC 12	48		SP- SM	62.0	S.A.A., slow dilatancy; wet	4468.3	

A	CON					WELL	NUM	MBER MW-37B PAGE 4 OF 4
	NT <u>Basin</u> ECT NUN			6860		PROJECT NAME Laramie River Station PROJECT LOCATION		
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION		WELL DIAGRAM
			2.0	SM		66.2 SILTY SAND, very fine to fine-grained; stiff, moist, yellowish brown	4464.1	+ 10/20 Silica Sandpack (54' - 77.5' bgs)
 				SP- SM		68.0 SAND, very fine to fine-grained, with silt, slow dilatancy; loose, wet, yellowish brown 69.0 No recovery	<u>4462.3</u> <u>4461.3</u>	0.010 Slotted Pipe (57.5' - 77.5' bgs)
 	SONIC 13	43				76.3	<u>4454.0</u>	
 	-			SP- SM ML SM		77.9 78.4 SANDY SILT, noncohesive, nonplastic, no odor or staining; soft, wet, gray	<u>4452.4</u> <u>4451.9</u>	Total Depth of Well 77.5' bgs Correction Below Well - Natural Collapse
			1	1	1.08/14.3	<u>79.0</u> <u>SILTY SAND, very fine-grained, no odor or staining; soft, wet, gray</u> Bottom of borehole at 79.0 feet.	<u>4451.3</u>	Natural Collapse

LGE GHENT SOIL-WELL LOG - GHENT.GDT - 10/10/16 15:04 - C:\BISMARK GINT\LRS - 60506860 2:01\LR\

LIENT							PROJECT NAME _Laramie River Station PROJECT LOCATION		
RILLIN	g con G Met	itra('Hod	TOR _	<u>Majo</u> y Sor	or Drillin nic				
OORDI	NATES	3 _454	47.479	N	4544.6	695 E	AFTER DRILLING 56.90 ft / Elev	/ 4471.18 ft	
	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION		Casing Top Elev: 2.784 (ft; Casing Type: 2" PVC Pipe WELL DIAGRAM
- - 5 - - -	N/A SONIC	0		SP		loose, moist, light b Thin beds of mediu lithies) loosely ceme 9.0	fine-grained, subround, some silt, trace clay; rown m to coarse-grained sand (k-spar, qtz, minor ented grained sand, with gravel and cobbles	<u>4523.1</u> 4519.1	- Grout (0' - 62' bgs)
- - - 15 - -	SONIC 2	100		FILL		17.0 SILTY SAND, very light brown	fine to fine-grained, round; loose, moist to dry,	4511.1	PVC Pipe (2.874' ags - 7 bgs)
 20 	SONIC	100	-	SM SM		20.0 SILTY SAND, very light brown	fine to fine-grained, round; loose, moist to dry, ly bedded sandstone with sand matrix, loosely	4508.1	

	Basir	1 Elect		6860			NUMBER MW-38 PAGE 2 OF
(ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM
25 - - - 30 - - - - - - - - - - - - -	SONIC 4			SM		SILTY SAND, very fine to fine-grained, round; loose, moist to dry, light brown	Grout (0'-49.6'bgs) PVC Pipe (2.874'ags - 7 bgs)
	SONIC	100					 ◄3/8" Bentonite Chips (49.6' - 52.9' b ₩ ₩ 10/20 Silica Sandpack

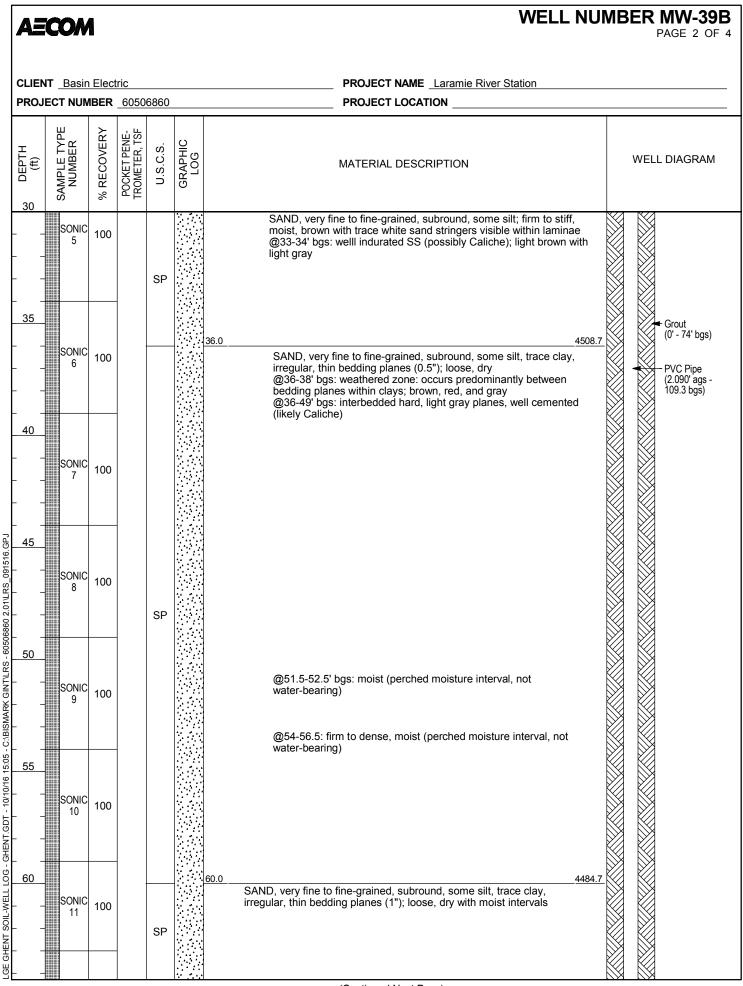
WELL NUMBER MW-38B

	NT <u>Basir</u>			<u></u>		
					PROJECT LOCATION	
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	MATERIAL DESCRIPTION	WELL DIAGRAM
LGE GHENT SOIL-WELL LOG - GHENT GDT - 10/10/18 15:05 - C.\BISIMARK GINT/LRS - 60506860 2.01/LRS - 091516.GPJ - 009		100	PC TR	SM	SILTY SAND, very fine to fine-grained, round; loose, moist to dry, light brown @56' bgs: increase in moisture content @057-57.5': well-graded, competent SS, cemented (likely calcareous); dense, white 80.0 4468: SILTY SAND, very fine to fine-grained, round, slightly more com- petent zones/beds of moderately cemented SS; loose, moist, light brown to brown intervals with increasing moisture	(52.9' - 76.4' bgs) 10/20 Silica Sandpack (52.9' - 76.4' bgs) 0.010 Slotted Pipe (55' - 75' bgs) Total Depth of Well 75' bgs
LGE GHENT SOIL-WELL LOG - GH			_			Bentonite Chip Fill Below Well

AECOM

A	CON					WELL NUMBER MW-38E PAGE 4 OF	
	NT <u>Basir</u> ECT NUN			6860		PROJECT NAME Laramie River Station PROJECT LOCATION	_
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION WELL DIAGRAM	
 <u>85</u> 	SONIC 9	100		SM		SILTY SAND, very fine to fine-grained, round, slightly more com- petent zones/beds of moderately cemented SS; loose, moist, light brown to brown intervals with increasing moisture 88.0 4440.1	
		1		1	1.1.4.	Bottom of borehole at 88.0 feet.	

PROJECT NUMBER 605068	WELL NUMBER MW-39B PAGE 1 OF 4 PROJECT NAME Laramie River Station 9ROJECT LOCATION 9ROJECT LOCATION 60 9ROJECT LOCATION 60 9ROJECT LOCATION 60 9ROJECT LOCATION 4544.695 ft 9ROJECT Drilling, Inc 60 9ROJECT LOCATION 4544.695 ft 9ROJECT Drilling, Inc
DRILLING METHOD Rotary : LOGGED BY Matt Hartz	Sonic AT TIME OF DRILLING CHECKED BY _A. Lanning AT END OF DRILLING
	4579.362 E V AFTER DRILLING _78.30 ft / Elev 4466.40 ft
DEPTH (ft) (ft) (ft) SAMPLE TYPE NUMBER % RECOVERY POCKET PENE- TROMETER, TSF	No. No. No. No. Casing Top Elev: 2.09 (ft) Casing Type: 2" PVC Pipe MATERIAL DESCRIPTION WELL DIAGRAM Top of Casing
	Hand potholed until 6' bgs
SONIC 100 SONIC 100 	6.0 4538.7 SAND (possibly fill), fine to medium-grained, some silt, trace gravel and cobbles; dry, light brown 4535.7 9.0 4535.7 SILTY SAND, very fine to fine-grained, subround, trace clay, no apparent bedding; loose, dry, light brown to brown with light gray
R599 SONIC 100	Grout (0' - 74' bgs) SM
88 82 100 24	@16-17' bgs: thin beds (<2") of well indurated and cemented SS, possibly Caliche; light gray
20000000000000000000000000000000000000	SP
	30.0 4514.7 (Continued Next Page)



			tric				NUMBER MW-39B PAGE 3 OF 4
PROJ	ECT NUN	IBER	6050	6860		PROJECT LOCATION	
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM
65 	SONIC	100				SAND, very fine to fine-grained, subround, some silt, trace clay, irregular, thin bedding planes (1"); loose, dry with moist intervals	- Grout (0' - 74' bgs)
	SONIC 13	100				@75-78' bgs: moist (perched moisture interval, not water-bearing)	PVC Pipe (2.090' ags - 109.3 bgs)
	SONIC	100				¥	- ⊲ 3/8" Bentonite Chips (74' - 79' bgs)
 85 	SONIC	100		SP		@82-88' bgs: moist (due to drilling waters)	- 10/20 Silica Sandpack (79' - 109.3' bgs
90						@91-99' bgs: moist to wet	
 	SONIC 16	100				@96-98' bgs: increase in well consolidated SS interbedding (2"), siliceous cementation; hard	0.010 Slotted Pipe (89.3' - 109.3' bgs)

A	CON	1				WELL NU	MBER MW-39B PAGE 4 OF 4			
	NT <u>Basin</u> ECT NUN			6860			PROJECT NAME Laramie River Station PROJECT LOCATION			
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM			
 100 105 				SP		SAND, very fine to fine-grained, subround, some silt, trace clay, irregular, thin bedding planes (1"); loose, dry with moist intervals @101-107' bgs: moist to wet intervals within medium-grained sand interbeds (<2") within native formation, potentially water-bearing 109.3 4435. Bottom of borehole at 109.3 feet.	4 Total Depth of Well 10/20 Silica Sandpack (79' - 109.3' bgs) 0.010 Slotted Pipe (89.3' - 109.3' bgs) Total Depth of Well 109.3' bgs			
LGE GHENI SOIL-WELL LOG - GHENI GUI - 10/10/16 15:06 - C.:BISMARK GINI ILKS - 60506860 2.01 LKS_091516.GFJ										

	Basin	Elect					PROJECT NAME Laramie River Station		
DATE ST DRILLIN DRILLIN LOGGEI	TARTEI IG CON IG METI D BY _N	D <u>7/*</u> TRAC HOD	15/201 TOR Rotar	6 Majo ry Sor	<u>CON</u> r Drilling, Ir hic CHECKED	MPLETED <u>7/16/2016</u>	GROUND ELEVATION 4587.399 ft GROUND WATER LEVELS: AT TIME OF DRILLING AT END OF DRILLING	HAMMER	TYPE _Not Applicable
0 (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION	C: C:	asing Top Elev: 2.194 (ft) asing Type: 2" PVC Pipe WELL DIAGRAM
	N/A	0			5.0	Hand potholed until FILL, sand and silt n Varying lithologies	5' bgs natrix with gravel to large cobbles	4582.4	(2.194 ⁻ aga)
	SONIC 1 SONIC 2	100		FILL	12.0	SILTY SAND, fine to	o medium-grained, trace clay, unconsolidated,	4575.4	Grout (0' - 81' bgs)
- 	SONIC 3	100				irregular thin beds, s moist, light brown	ilts and clays within bedding planes; soft to firm	ι, 	PVC Pipe (2.194' ags - 107.9 bgs)
20 - - - - - - - - - - - - - - - - - - -	SONIC	100		SM		23' bgs: firm, moist t	o wet		
						28' bgs: firm, moist t 28-30' bgs: moderat	o wet ely consolidated, firm to stiff, moist (on the		

WELL NUMBER MW-40B AECOM PAGE 2 OF 4 CLIENT Basin Electric PROJECT NAME Laramie River Station PROJECT NUMBER 60506860 PROJECT LOCATION SAMPLE TYPE NUMBER POCKET PENE-TROMETER, TSF % RECOVERY GRAPHIC LOG DEPTH (ft) U.S.C.S. WELL DIAGRAM MATERIAL DESCRIPTION 30 SILTY SAND, fine to medium-grained, trace clay, unconsolidated, irregular thin beds, silts and clays within bedding planes; soft to firm, moist, light brown 30-35' bgs: moderately consolidated, firm to stiff, moist (on the verge of SS) SONIC 100 5 SM 35-36' bgs: interbeds (<6") of consolidated SS (primarily qtz), very fine to fine-grained, siliceous cementation; dry, white 35 Grout (0' - 81' bgs) PVC Pipe (2.194' ags -107.9 bgs) <u>38.0</u> 4549.4 SAND, very fine to fine-grained, subround, some silt, trace clay, minimal weathering, interbeds (<6") of sandstones; firm, moist, light brown to brown 40 SONIC 100 6 _GE GHENT SOIL-WELL LOG - GHENT GDT - 10/10/16 15:06 - C:\BISMARK GINT\LRS - 60506860 2.01\LRS 091516.GP. 45 50 SP SONIC 100 7 55 @57-60' bgs: wet (water-bearing) @60-64' bgs: wet (water-bearing) 60 SONIC 100 8

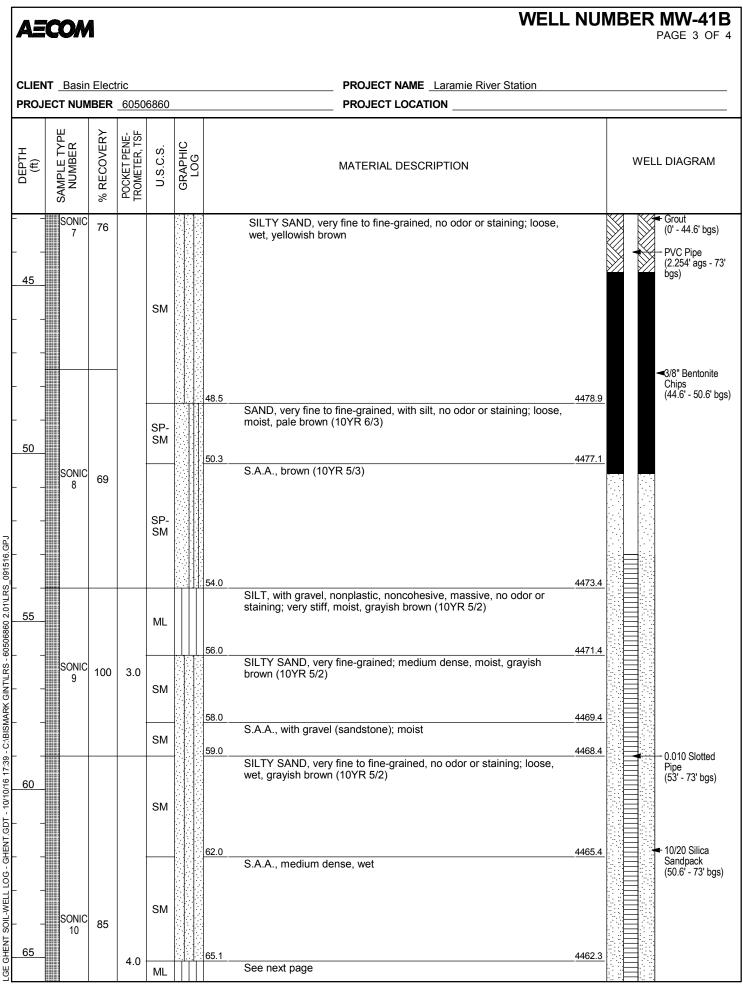
EA	CON	1				WELL	NUM	BER MV	V-40B E 3 OF 4
	NT <u>Basir</u> IECT NUN			6860		PROJECT NAME Laramie River Station PROJECT LOCATION			
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION		WELL DIA	GRAM
65 70 75 75 	- SONIC 9 - SONIC 9 - SONIC 9	100	-			SAND, very fine to fine-grained, subround, some silt, trace clay, minimal weathering, interbeds (<6") of sandstones; firm, moist, light brown to brown @68-78' bgs: moist	V K K K K K K K K K	PVC	ıt 81' bgs) 94' ags - 9 bgs)
BISMARK GINTLRS 60506860 2.011.LRS 091516.GPJ CS 0	- SONIC - SONIC - 10	100		SP		 @80-82' bgs: increase in moisture above SS unit @82' bgs: SS horizon (2"), well indurated and silified; hard, dry, gray with white clay at top and bottom @83-85' bgs: increase in moisture below SS unit 		-3/8" Chip (81'-	Bentonite s - 85.1' bgs)
LGE GHENT SOIL-WELL LOG - GHENT.GDT - 10/10/16 15:06 - C:\BISMARK GINTLRS - 60506860 2.01/LRS_091516.GPJ		100	SAN	DST	96.1 DNE	SANDSTONE, fine to medium-grained, subround to subangular, well indurated, well-cemented (siliceous); moist, brown	4491.4	Sanc (85.1	0 Silica dpack I' - 18' bgs) 0 Slotted 9' - 107.9'
				SP	98.0	See next page	4489.4		

CLIENT Basin Electric PROJECT NAME Laramie River Station PROJECT NUMBER 60506860 PROJECT LOCATION	WELL DIAGRAM
SAND, very fine to medium-grained, subround to subangula silt, slight zones of weathered, dark brown, oxidized beds; fin	WELL DIAGRAM
100 silt, slight zones of weathered, dark brown, oxidized beds; fill	
SP SONIC 105 105 105 105 105 105 105 105	some n to 10/20 Silica Sandpack (85.1' - 18' bgs) 0.010 Slotted Pipe (87.9' - 107.9' bgs) 4479.4 Total Depth of Well 107.9' bgs

LGE GHENT SOIL-WELL LOG - GHENT.GDT - 10/10/16 15:06 - C./BISMARK GINTLRS - 60506860 2.01/LRS_091516.GPJ

AECOM		WEL	L NUMBER MW-41B PAGE 1 OF 4			
PROJECT NUMBER 60506 DATE STARTED 8/3/2016	8860	PROJECT NAME _Laramie River Station PROJECT LOCATION GROUND ELEVATION _4527.383 ft HAMMER TYPE Not Applicable CROUND WATER LEVEL 0;				
DRILLING METHOD _ Rotary						
 DEPTH (ft) SAMPLE TYPE SAMPLE TYPE NUMBER NUMBER RECOVERY POCKET PENE- TROMETER, TSF 	U.S.C.S. GRAPHIC LOG	MATERIAL DESCRIPTION	Casing Top Elev: 2.254 (ft) Casing Type: 2" PVC Pipe WELL DIAGRAM			
10 10 10 10 5000 2 46 15 10 10 10 10 10 10 10 10 10 10	SP- GP SM SM SM SP- GP SM SN SW- SM SW- SM SW- SM SW- SM SW- SM SW- SM SW- SM SAND, very fine to light gray SAND, very fine to odor or staining; lo	fine-grained, with gravel, some cobbles, little si r fine to fine-grained; loose, moist, brown medium-grained, with round gravel; loose, moist coarse-grained, with silt, little angular gravel, no ose, moist, brown	<u>4518.4</u> <u>4517.4</u> st, <u>4517.4</u> <u>4517.4</u> PVC Pipe (2.254' ags - 73' bgs)			

	IT <u>Basir</u> ECT NUN			3860		PROJECT NAME Laramie River Station PROJECT LOCATION				
(ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM			
20				ML		SANDY SILT, nonplastic, noncohesive, no odor or staining; very soft, wet, grayish brown				
-			-	SM	21.0	SILTY SAND; soft, moist, gray	4506.4			
_				ML	21.8	SILT, nonplastic, noncohesive; medium, moist, gray and mottled brown and gray	4505.6			
_				IVIL	23.0	SILTY SAND, very fine to fine-grained, no staining; loose, moist,	4504.4			
- 25 -	SONIC 4	77		SM		brown, no mottling	4500.4			
_			_	ML	29.0	SILT, with sand, nonplastic, noncohesive, no odor or staining; very soft, wet, brownish gray	4300.4	Grout		
30				SM		SILTY SAND, very fine to fine-grained; loose, wet, grayish brown wetness due to drilling waters		(0' - 44.6' bgs)		
_	SONIC	80		SM	30.4	SILTY SAND, very fine to fine-grained, little subangular gravel; loose, moist, grayish brown (10YR 5/2)	<u>4497.0</u> 4495.9	PVC Pipe (2.254' ags - 7		
_	5			SP	32.2	SAND, very fine to fine-grained, trace silt, no odor or staining; loose, moist, light brownish gray	, <u>4495.2</u>	(2.254' ags - 7 bgs)		
_				SP		S.A.A., light gray (10YR 7/1)				
+				SP	34.0 34.5	SAND, very fine to medium-grained, little silt; medium dense, moist,	<u>4493.4</u> 4492.9			
<u>35</u>	SONIC	100		SP		dark grayish brown (10YR 4/2) S.A.A., little subangular gravel; light brownish gray (10YR 6/2)	,			
_	6				SP	37.0	S.A.A., no gravel; light gray (10YR 7/1)	4490.4		
40 -				SM	★ ★ 39.0 ↓ ↓ ↓ <td>SILTY SAND, very fine to fine-grained, no odor or staining; loose, wet, yellowish brown</td> <td>4488.4</td> <td></td>	SILTY SAND, very fine to fine-grained, no odor or staining; loose, wet, yellowish brown	4488.4			



⁽Continued Next Page)

EA	CON	8				WELL NU	MBER MW-41B PAGE 4 OF 4
	T <u>Basin</u> ECT NUN			6860		PROJECT NAME _Laramie River Station PROJECT LOCATION	
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM
				ML		SILT, nonplastic, noncohesive, blocky, no odor or staining; stiff, moist, light brownish gray (10YR 6/2), no mottling 69.0 ∇ 4458.4	0.010 Slotted Pipe (53' - 73' bgs)
				SP- GW ML	° ()	SAND, poorly graded, with gravel; very loose, wet, grayish brown	
 <u>75</u>	SONIC 11	78		SM		74.04453. S.A.A., decreasing in moisture	Total Depth of Well 73' bgs
				SM		79.04448.	A Native Clay Below Well - Natural Collapse
						Bottom of borehole at 79.0 feet.	

LGE GHENT SOIL-WELL LOG - GHENT.GDT - 10/10/16 17:39 - C:\BISMARK GINT\LRS - 60506860 2.01\LRS_091516.GPJ

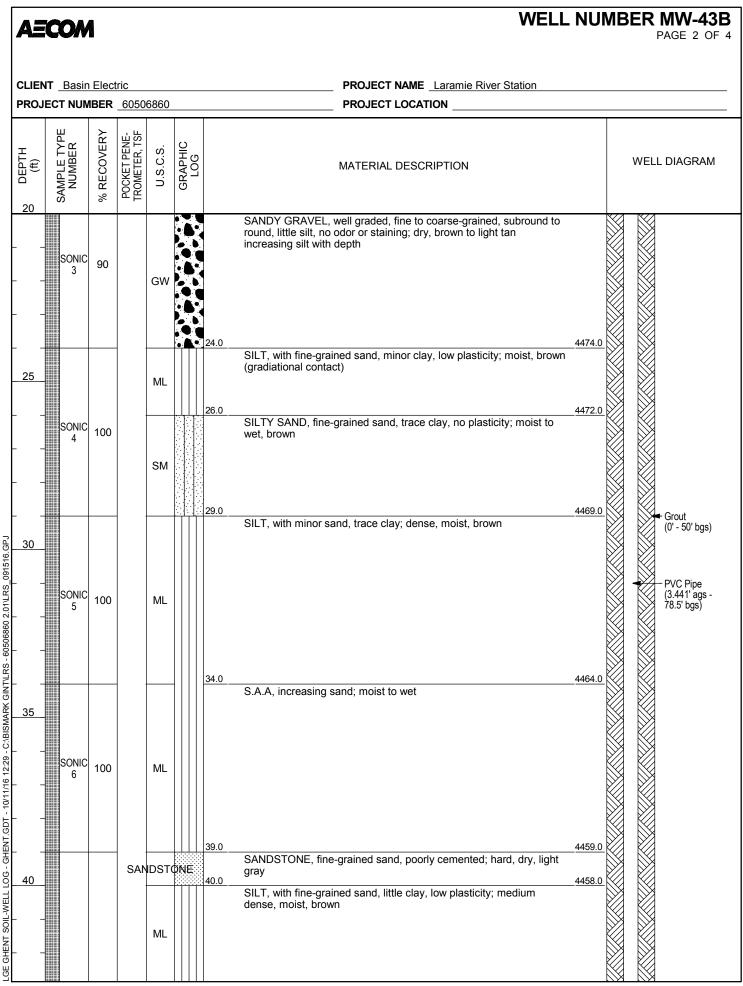
AECOM	Electric			PROJECT NAME Laramie River Station			
DATE STARTEI DRILLING CON DRILLING METI LOGGED BY	D <u>8/9/20</u> TRACTO HOD <u>R</u> o leremy H	016 DR <u>Majo</u> otary Sor lurshman	r Drilling, Inc	HAMMER TYPE Not Applicable			
O DEPTH (ft) SAMPLE TYPE NUMBER	% RECOVERY POCKET PENE-	TROMETER, TSF U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	Casing Top Elev: 2.534 (ft) Casing Type: 2" PVC Pipe WELL DIAGRAM		
- N/A - N/A 5	0	SM	9.0	ntil 5.5' bgs e-grained; loose, dry, medium brown to tan e-grained, poorly cemented sandstone lenses, no	4507.8		
10 SONIC	100	SM	odor or staining; i	moist to wet, light tan	(0 - 42 bgs) PVC Pipe (2.534' ags - 68.5' bgs)		
15	100	SM	SILTY SAND; dry 14-16' bgs: poorly drilling	γ, light tan y cemented sandstone lenses, lenses broken by			

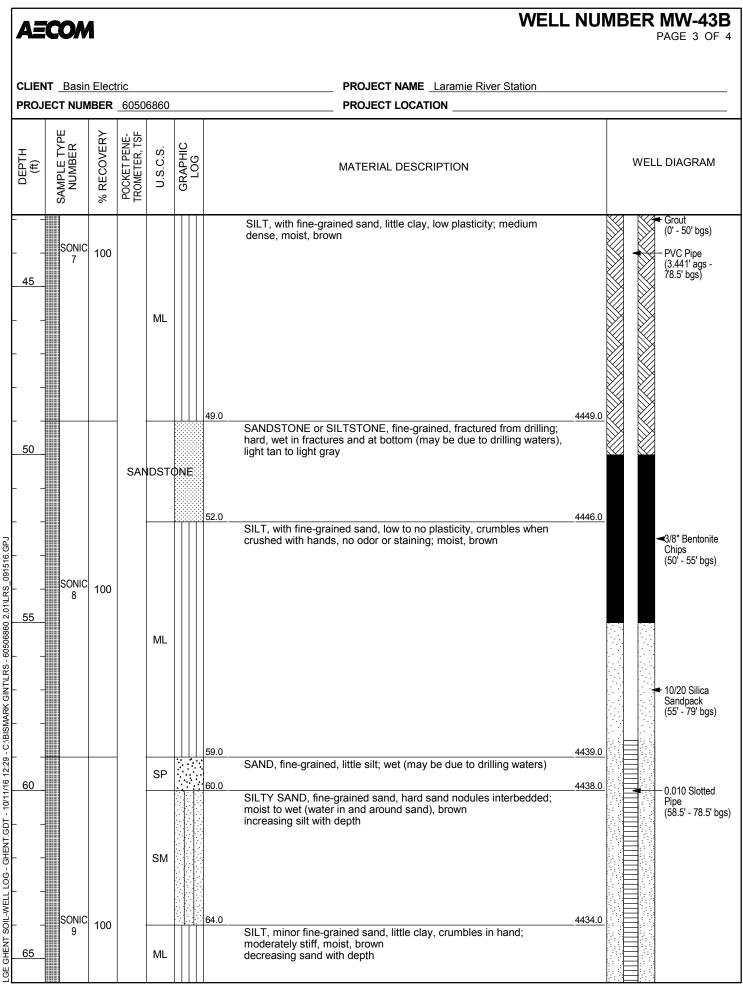
A	ECO/	R				WELL	NUN	ABER MW-42B PAGE 2 OF 4
	ENT <u>Bas</u> Dject Nu					PROJECT NAME Laramie River Station PROJECT LOCATION		
HLTH (#) 50	SAN	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION		WELL DIAGRAM
LGE GHENT SOIL-WELL LOG - GHENT.GDT - 10/10/16 15:07 - C:\BISIMARK GINTLRS - 60506860 2.01\LRS_091516.GPJ		² 100 ² 100 ² 100		SM SM SP SP SP		SILTY SAND; dry, light tan 22.0 SILTY SAND, fine-grained, subround, trace sandstone lenses; moist, light brown 27.0 SAND, fine-grained; loose with hard sections (breaks in hand), moist 29.0 SAND, fine-grained, few poorly cemented sandstone lenses, minor clay, no odor or staining, crumbles; dry to moist, brown 34.0 SILTY SAND, subround to round; loose, moist to wet (due to drilling waters), brown 38.0 SAND, fine-grained; dry, light tan @38' bgs: 1 inch of hard zone of visible thin channel, brown silt deposits SAND, fine-grained; dry, light tan @38' bgs: 1 inch of hard zone of visible thin channel, brown silt deposits	_4491.3 _4486.3 _4486.3 _4484.3 _4479.3 _4477.3	• Grout (0' - 42' bgs) • PVC Pipe (2.534' ags - 68.5' bgs)
LGE GHENT SOIL-WELL LOG -				SP				

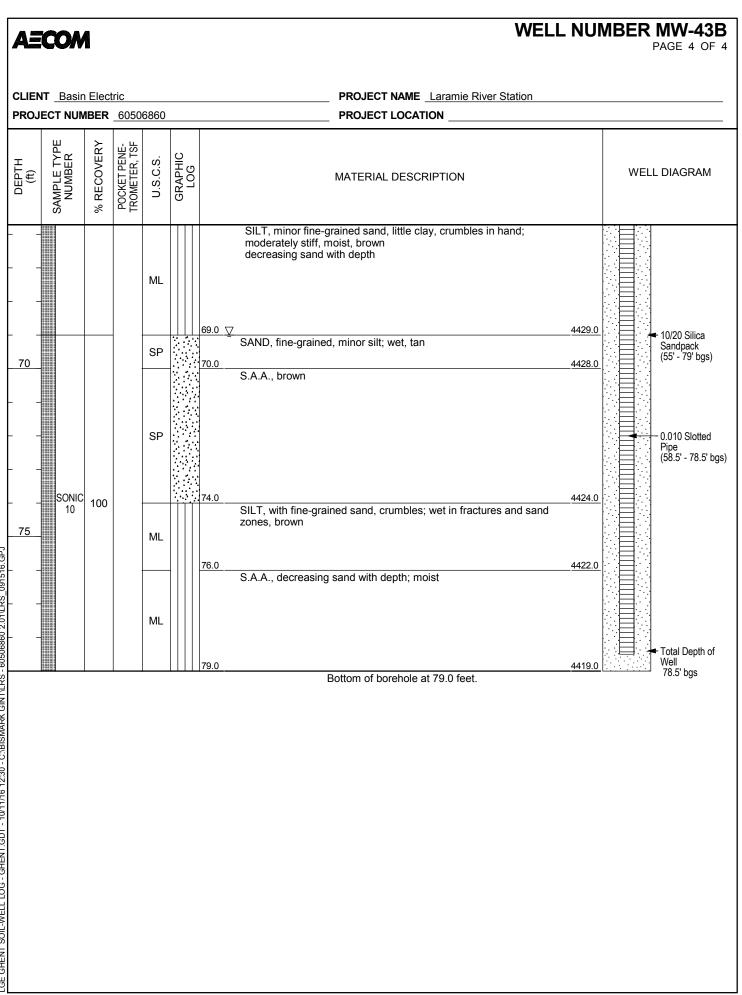
WELL NUMBER MW-42B AECOM PAGE 3 OF 4 CLIENT Basin Electric PROJECT NAME Laramie River Station PROJECT NUMBER 60506860 **PROJECT LOCATION** SAMPLE TYPE NUMBER POCKET PENE-TROMETER, TSF % RECOVERY GRAPHIC LOG U.S.C.S. DEPTH (ft) WELL DIAGRAM MATERIAL DESCRIPTION SAND, fine-grained, round, little silt, tight and compact; stiff, wet SONIC <3/8" Bentonite 100 Chips (42' - 45.75' bgs) 8 SP 45 @45' bgs: gradational contact: increasing silt and decreasing sand 46.0 4467.3 SILT, with fine-grained sand, few interbedded clay nodules, low to medium plasticity; stiff, wet, brown ML 49.0 4464.3 SILTY SAND, fine-grained, no visible structures; medium dense, wet, brown 50 10/20 Silica Sandpack (45.75' - 68.5' bgs) 0.010 Slotted . Pipe (48.5' - 68.5' bgs) .GE GHENT SOIL-WELL LOG - GHENT.GDT - 10/10/16 15:08 - C:\BISMARK GINT\LRS - 60506860 2:01\LRS_091516.GPJ SONIC 100 SM 9 55 59.0 4454.3 SANDSTONE, fine-grained, fractured from drilling; hard, light gray SANDSTONE to tan 60 60.0 4453.3 SAND, fine-grained, little silt; medium dense, wet, brown SP SONIC 100 10 65

ΞA	CON	1				WELL N	UMBER MW-42B PAGE 4 OF 4				
	IT <u>Basir</u> ECT NUN			6860		PROJECT NAME Laramie River Station PROJECT LOCATION					
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM				
				SP		SAND, fine-grained, little silt; medium dense, wet, brown	10/20 Silica Sandpack (45.75' - 68.5' bgs) 0.010 Slotted Pipe (48.5' - 68.5' bgs) 				
		Į	I	<u></u>	134 5.4.4	Bottom of borehole at 69.0 feet.	4.3 - 00.3 bgs) Total Depth of Well 68.5' bgs Native Clay Below Well - Natural Collapse				

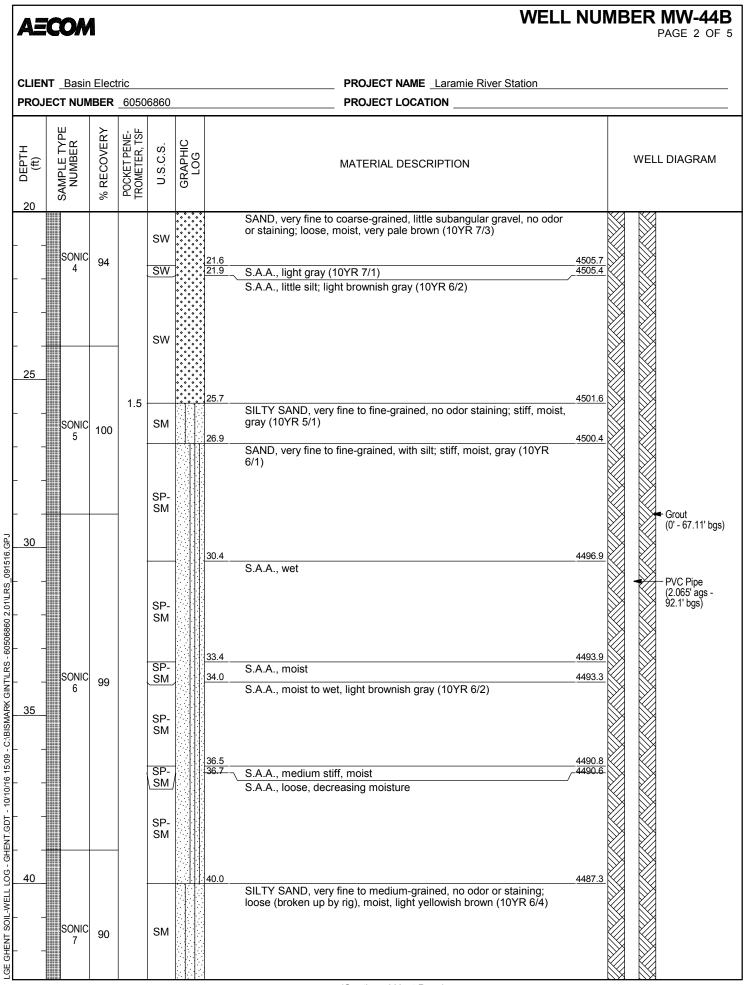
H H	MBER MW-43B PAGE 1 OF 4				
DRILLING CONTRACTOR Major Drilling, Inc GROUND WATER LEVELS: DRILLING METHOD Rotary Sonic AT TIME OF DRILLING _69.00 ft / Elev 4429 LOGGED BY Jeremy Hurshmatchecked BY A. Lanning AT END OF DRILLING	PROJECT LOCATION				
H H H H H H H 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 <t< th=""><th>9.00 ft</th></t<>	9.00 ft				
Hand potholed until 5' bgs N/A 0 	Casing Top Elev: 3.441 (ft) Casing Type: 2" PVC Pipe WELL DIAGRAM				
10 9.0 4489.0 10 SILTY SAND, fine-grained; moist to dry @9-10' bgs: minor round gravel (0.4-0.8" in size) little recovery due to bit being blocked 4489.0 - <td< td=""><td>Grout (0' - 50' bgs) PVC Pipe (3.441' ags - 78.5' bgs)</td></td<>	Grout (0' - 50' bgs) PVC Pipe (3.441' ags - 78.5' bgs)				

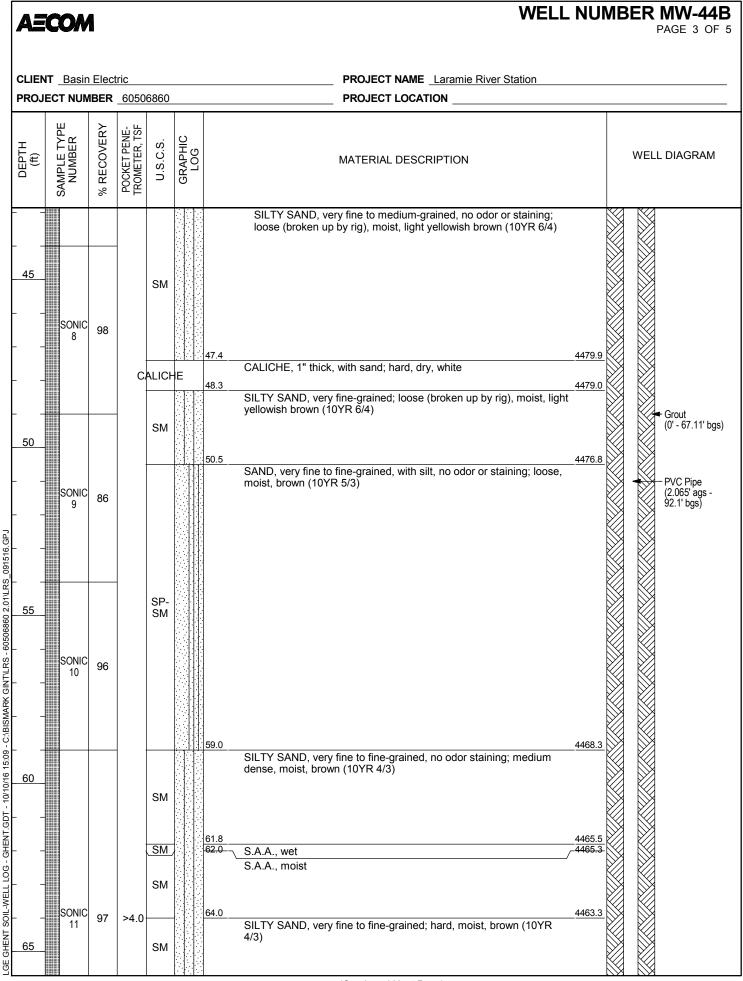






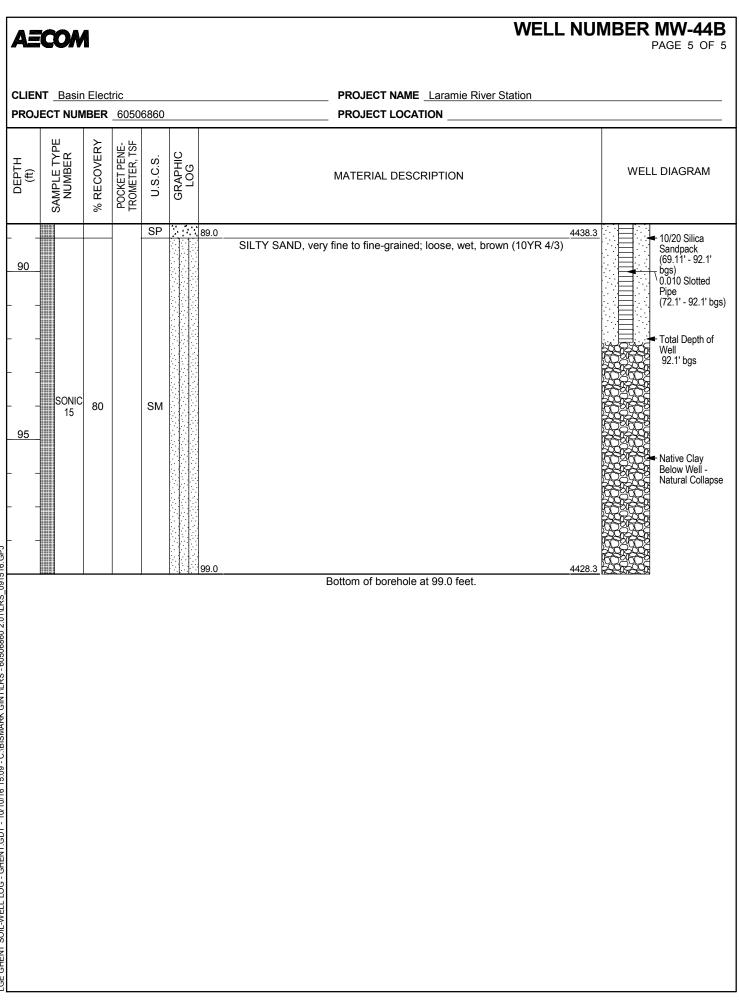
A ECO A	1			WELL	NUMBER MW-44
				PROJECT NAME Laramie River Station	
				PROJECT LOCATION	
ATE STARTE	D 8/4/20	016	COMPLETED _	8/5/2016 GROUND ELEVATION 4527.324 ft	HAMMER TYPE Not Applicable
				GROUND WATER LEVELS:	
RILLING MET				$\underline{\nabla}$ AT TIME OF DRILLING <u>74.00 ft / f</u>	
				ning AT END OF DRILLING	
	S <u>58965</u>	9.8 N	731518.4 E	AFTER DRILLING	
щ	≿	щ			Casing Top Elev: 2.065 (ft) Casing Type: 2" PVC Pipe
SAMPLE TYPE NUMBER	% RECOVERY POCKET PENE-	S. S.	GRAPHIC LOG		
THE	ET C	OMETER, T U.S.C.S.	LOO	MATERIAL DESCRIPTION	WELL DIAGRAM
, AMP	OCF R	U ROM	ц. Б.		Tur (Outin
ە 0	м <u>н</u>	<u>⊢</u>	SAND	(FILL), fine to coarse-grained, with round gravel, no odor or	Top of Casing (2.065' ags)
- - 5 5 - - - - - - - - - - - - - - - -		FILL	 Staining Sta	s: rock prevented 3.6' of recovery	Grout (0' - 67.11' bgs PVC Pipe (2.065' ags - 92.1' bgs)
	100	SW- GP SW	or stair	very fine to coarse-grained, with subangular gravel, no odor ing; loose, wet, grayish brown (10YR 5/2) no gravel; moist, very pale brown (10YR 7/3)	4513.3
		sw	18.0 S.A.A.	little subangular gravel	4509.3





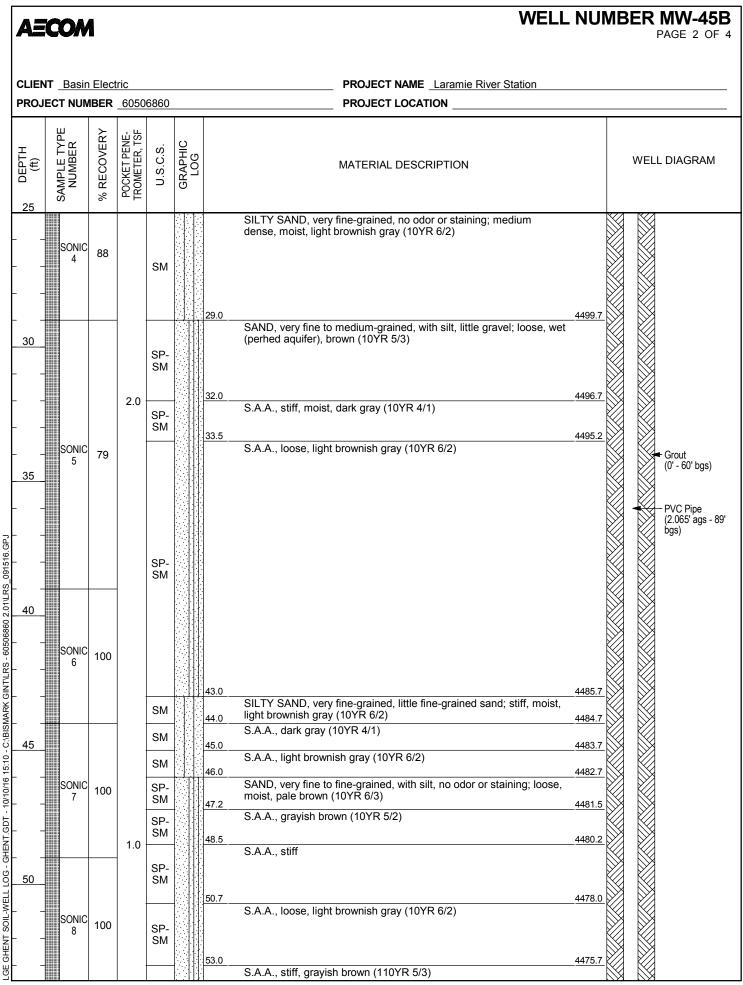
A	CON	1				WEL	L NU	MBER MW-44B PAGE 4 OF 5
	NT Basir					PROJECT NAME Laramie River Station		
PRO	JECT NUN	/IBER	6050	6860		PROJECT LOCATION		
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION		WELL DIAGRAM
				SM		SILTY SAND, very fine to fine-grained; hard, moist, brown (10YR 66.5 4/3)		
				SM		66.5 4/3) S.A.A., medium dense, moist	4460.8	Grout (0' - 67.11' bgs)
				SM		68.8 69.0- ¬S.A.A., soft, moist	<u>4458.5</u> 4458.3	
70						No recovery due to rock		Chips (67.11' - 69.11' bgs)
								+ 10/20 Silica Sandpack (69.11' - 92.1' bgs)
	SONIC	50				74.0 ▽	4453.3	0.010 Slotted Pipe (72.1' - 92.1' bgs)
75	12					SILTY SAND, very fine to fine-grained; loose, wet, grayish brown (10YR 5/2)		
				SM				
0 2 0 14						77.6 S.A.A., decreasing moisture	4449.7	
				SM		79.0	4448.3	
80						SILT, with sand, noncohesive, nonplastic, no odor or staining; medium dense, moist to wet, dark graish brown (10YR 4/2)		
	SONIC	60		ML				
۳ - ام - ام						84.0 SAND, very fine to fine-grained; loose, moist to wet, brown	4443.3	
85 - 9						Shire, very fine to fine-grained, 10056, 11015t to wet, brown		
	SONIC	98		SP				
						88.0	4439.3	
5				SP	1	S.A.A., fine to medium-grained		

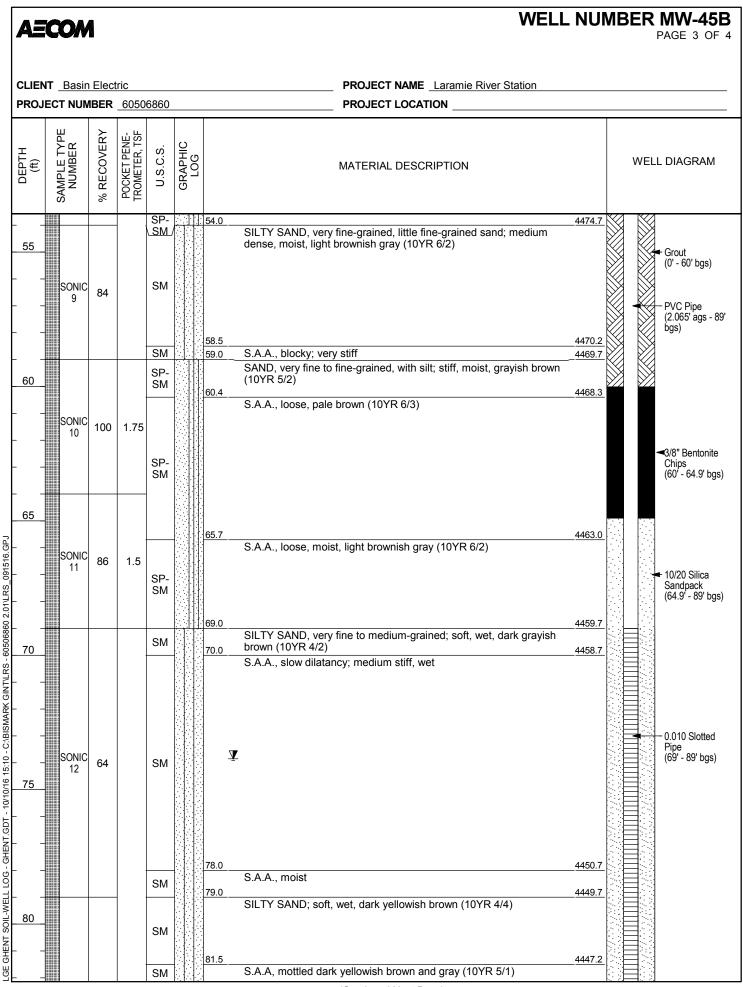
⁽Continued Next Page)



LGE GHENT SOIL-WELL LOG - GHENT.GDT - 10/10/16 15:09 - C.\BISMARK GINTLRS - 60506860 2.01\LRS_091516.GPJ

A	CON	1				WELI	- NUI	MBEF	MW-45B PAGE 1 OF 4	
PROJ DATE DRILL DRILL	ECT NUM STARTE ING COM ING MET GED BY	MBER 50 <u>8/0</u> NTRAC 6 THOD Chris	6050 6/2016 CTOR Rotai	Majc Majc ry Sor	or Drillin nic CHECK	PROJECT LOCATION	GROUND ELEVATION _4528.664 ft HAMMER TYPE _Not Appli GROUND WATER LEVELS: AT TIME OF DRILLING AT END OF DRILLING			
o DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION		Casing Ty	pp Elev: 2.257 (ft) pe: 2" PVC Pipe LL DIAGRAM — Top of Casing — (2.257' aos)	
 	SONIC 1	100		SW- GP SP- SM	○ 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	SAND, very fine to medium-grained, with gravel, little silt, little organic material; loose, moist, brown 3.0 S.A.A., with silt, less gravel	4522.7		(2.257' ags)	
 	SONIC 2	53	3	ML GP SW- GP SW- GP		SILT, with gravel and sand, noncohesive, nonplastic; soft, wet (due to drilling waters), grayish brown (10YR 5/2) 10.9 SAND, fine to coarse-grained, with subround gravel, little silt; loose, moist, brown S.A.A., some silt S.A.A., no silt	4517.8 4516.9 4516.1		- Grout (0' - 60' bgs) - PVC Pipe (2.065' ags - 89' bgs)	
 25	SONIC 3	100	>4.0	SM SM		19.0 SILTY SAND, very fine-grained, no odor or staining; soft, moist, ligh brownish gray (10YR 6/2) 23.0 S.A.A., hard 24.0 S.A.A., medium	4509.7 it 4505.7 4504.7			





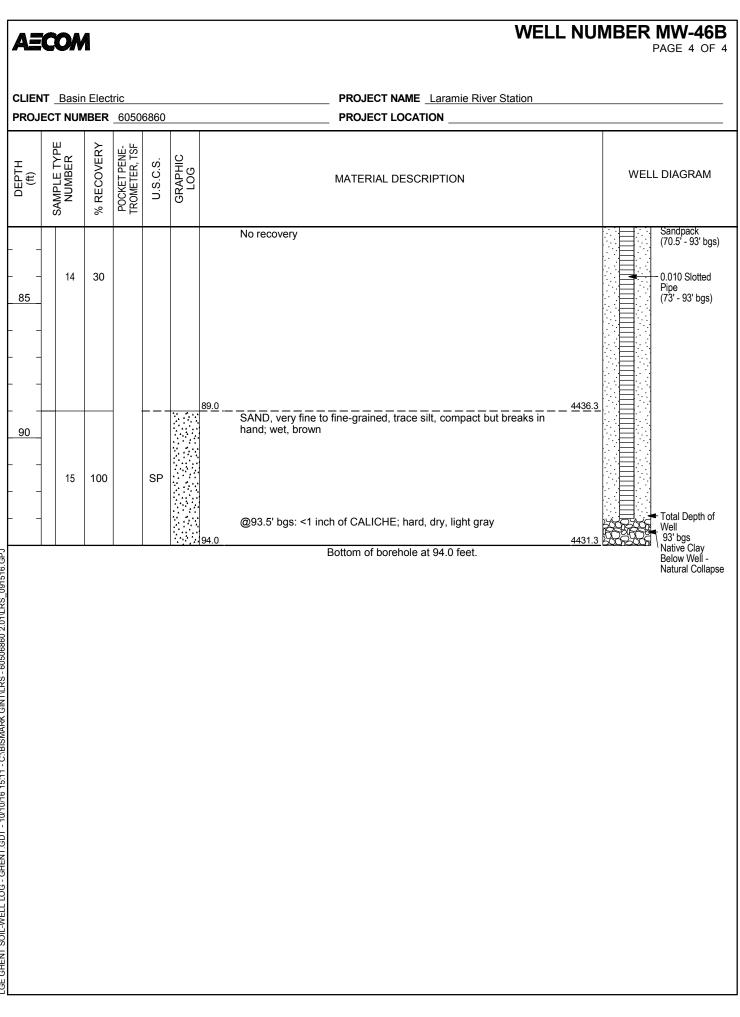
A	CON						WELL NUMBER MW-45B PAGE 4 OF 4			
CLIENT Basin Electric PROJECT NUMBER 60506860							PROJECT NAME _Laramie River Station PROJECT LOCATION			
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	Ν	IATERIAL DESCRIPTION	WELL DIAGRAM		
 <u>85</u> 	. SONIC 13	32				82.5 No recovery	4446.2	 10/20 Silica Sandpack (64.9' - 89' bgs) 0.010 Slotted Pipe (69' - 89' bgs) 		
		1	<u> </u>	<u>I</u>			ttom of borehole at 89.0 feet.	L ∵		



	n Elec				PROJECT NAME Laramie River Station PROJECT LOCATION			
DRILLING CON DRILLING MET OGGED BY	ITRA HOD Chris	CTOR _ _Rotary Ahrend	Majo / Son t (r Drilling, Inc ic CHECKED BY <u>A. Lanning</u>				
C UEPTIN (ft) SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	Casing Top Elev: 2.383 (ft) Casing Type: 2" PVC Pipe WELL DIAGRAM		
- - - - - - - - - - - - - - - - - - -	100		SP	staining; loose, d	to medium-grained, with gravel and silt, no odor or ry to moist, brown ic material (roots, etc)	(2.383' ags)		
2	100		SM	11.5 SILTY SAND, ve brown 14.0	ry fine to fine-grained; loose, moist, yellowish	4513.8 4511.3 4511.3		
<u>15</u> - 3	100		SW- GP	SAND, very fine f granite); loose, b		4507.9		
20			ML ML	19.0 SILT, some fine t	ve, nonplastic, blocky; stiff, moist, brown, mottled to medium-grained sand, little white qtz fragments, nplastic, blocky; medium stiff, moist, grayish)	4506.3		
4 4	100		ML	21.4 SILT, some fine t noncohesive, nor (10YR 5/2) 23.0 S.A.A., moist	o coarse-grained sand, little angular gravel, nplastic, blocky; medium stiff, wet, grayish brown	4503.9		

A	ECON	A				WELL	. NUI	MBER MW-46B PAGE 2 OF 4		
CLIE	NT Basi	n Elect	tric			PROJECT NAME Laramie River Station				
	JECT NUI			6860		PROJECT LOCATION				
(#) 25	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION		WELL DIAGRAM		
25 30 1 30 30 35		100 97 100	3.5 C	ML SM SM SM SM SM SM SM ALICF ML ML SP- SM ALICF		SILT, some fine to coarse-grained sand, little angular gravel, noncohesive, nonplastic, blocky; medium, moist, grayish brown (10YR 5/2) SILTY SAND, very fine to fine-grained; loose, moist, yellowish brown 29.0 SILTY SAND, very fine-grained; loose, moist, gray (10YR 5/1) 31.0 S.A.A., very loose 32.8 33.1 S.A.A., very stiff 34.0 S.A.A., very stiff 34.0 S.A.A., very loose SAND, poorly graded, with silt; loose, moist, grayish brown (10YR 5/2) 35.8 CALICHE, laminated; hard, dry, white SANDY SILT, noncohesive, nonplastic; medium stiff, moist, brown 41.0 S.A.A., increasing sand content 42.9 SAND, very fine to fine-grained, with silt, no odor or staining; loose, moist, light brownish gray (10YR 6/2) 47.5 A.1 47.5 CALICHE; hard, moist, white	4498.8 4496.3 4494.3 4492.2 4492.2 4491.3 4492.2 4491.3 4492.2 4491.3 4492.2 4482.4 4482.4 44477.8 44777.8 44777.2	Grout (0' - 66' bgs) PVC Pipe (2,383' ags - 93' bgs)		
LGE GHENT SOIL-WELL LOG - GHENT.	- 9 - 9	62	C	SP- GP SP- GP SP- SM SP	Ê	48.5 (48.7) SAND, poorly graded, with gravel 48.7 (49.0) CALICHE; hard, moist, white 50.4 SAND, poorly graded, with gravel; moist 50.4 SAND, poorly graded, with silt; loose, brown (10YR 5/3) SAND, very fine to medium-grained, with gravel and silt; loose, brown	4476.8 4476.6 4476.3/ 4474.9			

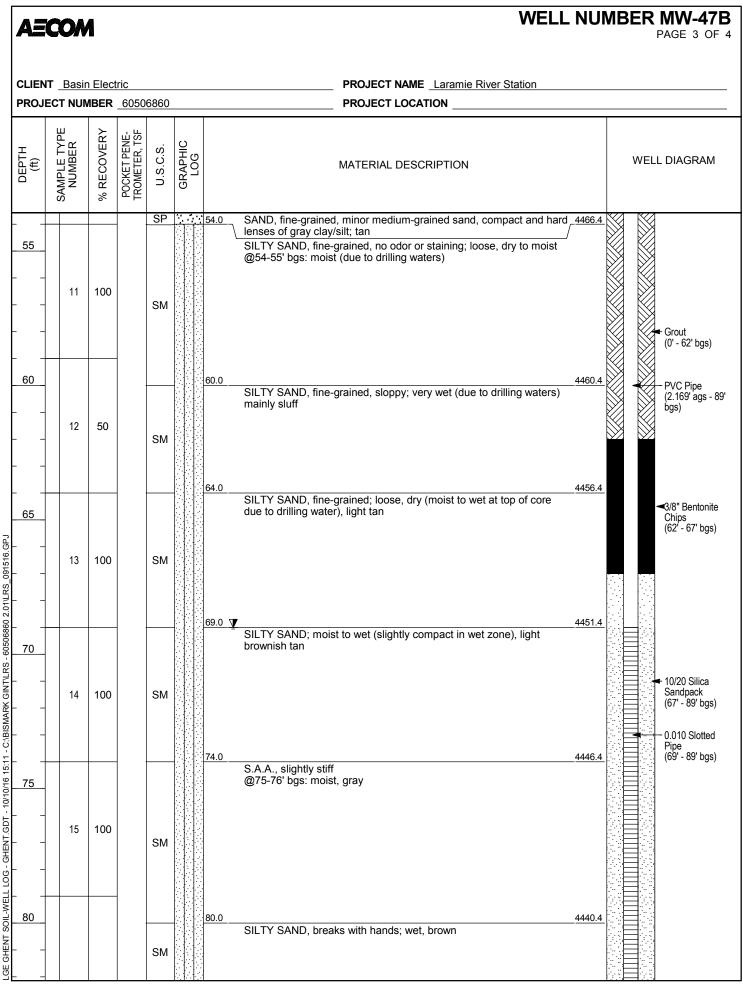
	T <u>Basir</u>			6960		BRO (FOT LOOATION				
UEPTH (ff)	SAMPLE TYPE SAMPLE TYPE NUMBER % RECOVERY % RECOVERY POCKET PENE- IROMETER, TSF LOG LOG LOG					MATERIAL DESCRIPTION	PROJECT LOCATION MATERIAL DESCRIPTION WELL DIAGRAM			
55				SP- SM- SP- SM	54	54.0 44 54.3 SAND, very fine to medium-grained, with silt, trace angular gravel; very dense, moist, grayish brown (10YR 5/2) S.A.A., loose	471.3 471.0			
_	10	96	>4.0	SP- SM SP- SM			468.3 467.8			
60 -	11	83		SP- SM		SAND, very fine to medium-grained, with silt, trace angular gravel; very dense, moist to wet, light brownish gray (10YR 6/2)	465.3	Grout (0' - 66' bgs) PVC Pipe (2.383' ags - 93 bgs)		
<u>65</u> _ _ _	12	100		SM SP- SM		SILTY SAND, very fine to fine-grained, no odor or staining; loose,	<u>460.3</u> 459.3			
<u>70</u>				SP- SM	<u>6</u>	59.0 44 S.A.A., wet	456.3	Chips (66' - 70.5' bgs		
- - 75	13	47	3.0	ML SP- SM SM		SILT, with fine sand, noncohesive, nonplastic; very soft, wet, dark gray (10YR 4/1)	452.8 452.5 451.3			
				SM SP-	71	S.A.A., dry 78.3 SAND, very fine to fine-orained, with gravel: stiff, dry, light grav	449.3	0.010 Slotted Pipe (73' - 93' bgs)		
80				GP SP- GP		S.A.A., medium stiff, moist	446.3			



LGE GHENT SOIL-WELL LOG - GHENT.GDT - 10/10/16 15:11 - C.\BISMARK GINT\LRS - 60506860 2.01\LRS_091516.GPJ

EA	CON	1				WELL NUMBER MW-47B PAGE 1 OF 4
PROJI DATE DRILL DRILL LOGG	ING COM ING MET ED BY _	MBER ED <u>8/8</u> NTRAC FHOD Jerem	_60506 3/2016 CTOR _ _Rotar	6860 Majo y Son	COMI r Drilling, Ind nic CHECKED E	PROJECT NAME Laramie River Station PROJECT LOCATION LETED 8/9/2016 GROUND ELEVATION 4520.426 ft GROUND WATER LEVELS: AT TIME OF DRILLING Y A. Lanning Y AFTER DRILLING _69.00 ft / Elev 4451.43 ft
o DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	Casing Top Elev: 2.169 (ft) Casing Type: 2" PVC Pipe WELL DIAGRAM
 	1	100		ML SP SW- GP	3.0 3.0 5.5 6.0 5.5 5.5 5.5 5.5 5.5 5.5 5.5 5.5 5.5 5	SILT, trace gravel, rootlets near surface, topsoil; dry, dark brown 4517.4 SILT, with very fine-grained sand; dry, tan 4514.4 SAND, well graded, with 30% round gravel and cobbles (max 3"), no odor or staining; dry, tan 4511.4
<u> 10 </u>	2	100		SP	16.0	SAND, poorly graded, with 25% subround gravel and 15% silt, few cobbles, no odor or staining; dry, light tan Material very hot from core barrel PVC Pipe (2.169' ags - 89' bgs) 4504.4 SAND, fine-grained; loose, slightly moist, light tan
	3	100		SP		@17' bgs: .5" thick cemented zone
 	4	100	C	ALICH	E 22.0	4500.4 CALICHE, sandy, breaks into discs, broken from drilling; hard, wet (due to drilling waters), white 4498.4 SILT, little sand, trace clay, low plasticity, crumbles; moist, light tan to white 4498.4 SILTY SAND, fine-grained, trace clay; dry, light tan to white

A	'CO/	N				WELL	NUI	MBER	MW-47B PAGE 2 OF 4
	IT Bas								
PROJ	ECT NU	MBER	6050	6860		PROJECT LOCATION			
(¥) 25	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION		WEL	L DIAGRAM
				SM		SILTY SAND, fine-grained, trace clay; dry, light tan to white @25.4-26.5' bgs: caliche modules	4400.0		
	5	100		SM		6.5 SILTY SAND, fine-grained, no odor or staining; dry to moist, medium tan	_4493.9		
30				SP		9.0 SAND, fine grained, few hard cemented sand nodules, trace fines; hard, wet (due to drilling waters)	4491.4		
		100		ML		SILT, minor fine-grained sand, few white caliche nodules; moist to dry	4490.1		
	6	100		ML		SILT, with very fine-grained sand, crumbles; medium tan to brown	4488.7		
35				SM		4.0 SILTY SAND, fine-grained; soft, wet (due to drilling waters) 5.0	<u>4486.4</u> 4485.4		- Grout (0' - 62' bgs)
				SP		SAND, fine-grained, with sandstone lenses, water in fractures, minor silt, no odor; stiff to very stiff	_4403.4_		- PVC Pipe
 	7	100		SM	3	6.7 SILTY SAND, fine-grained, no odor or staining, crumbles; light tan to brown could be siltstone	4483.7		(2.169'ags - 89' bgs)
			SAN	DST		9.0 SILTY SANDSTONE, very fine-grained; hard, wet in fractures, dry in macro core, light tan	4481.4		
	8	100		SP- SM	4	0.5 SAND, fine-grained, with silt; loost, moist to wet (water in borehole), medium to dark brown	4479.9		
			-		4	4.0	4476.4		
45				SP	4	5.0 wet, tan to light tan S.A.A., dry	4475.4		
	9	100							
5 50				SP		@49-50' bgs: wet (due to drilling waters) @50-53' bgs: moist, grayish brown			
	10	100							
					5	3.5	4466.9		



AE	CON	1				WELL NU	MBER MW-47B PAGE 4 OF 4		
CLIENT Basin Electric PROJECT NUMBER 60506860						PROJECT NAME Laramie River Station PROJECT LOCATION			
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM		
 <u>85</u> 	16	100		SM		SILTY SAND, breaks with hands; wet, brown 89.0 4431.4	10/20 Silica Sandpack (67' - 89' bgs) 0.010 Slotted Pipe (69' - 89' bgs) (69' - 89' bgs) Total Depth of		
					<u></u>	Bottom of borehole at 89.0 feet.	Well 89' bgs		

	COM	_						WELL NUMB	PAGE 1 OF 2	
	NT Basin						PROJECT NAME Laramie River Station			
							PROJECT LOCATION Wheatland, W GROUND ELEVATION 4565.87			
							GROUND ELEVATION 4303.07		FE	
						ang Eaboratorioo				
						ED BY				
						.2 E		ft / Elev 4488.27 ft		
	Ц	37	ᆄᄶ					Cas Cas	ing Top Elev: 0 (ft) ing Type: 4" PVC Pipe	
HL O	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	S.	GRAPHIC LOG				WELL DIAGRAM	
DEPTH (ft)		RECC	CKET	U.S.C.S.	LO		MATERIAL DESCRIPTION			
0	SAN	Ч %	Q TI D							
						SILTY SAND, with	gravel		(estimated 3' ags)	
			SAN	SM		36.0 SANDSTONE		4529.9	ASTM C-33 Concrete Fines (0' - 60' bgs) 4" Sch. 40 PVC Pipe (3' ags - 96' bgs)	

A	icow	1					WELL NUMBER MW-22B PAGE 2 OF 2			
CLIEN	IT Basin	Electri	ic				PROJECT NAME Laramie River Station			
PROJ	ECT NUM	BER _	60506	860			PROJECT LOCATION Wheatland, Wyoming			
05 DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION	WELL DIAGRAM		
50 		6		IDST		SANDSTONE ¥ 96.0	Bottom of borehole at 96.0 feet.	ASTM C-33 Concrete Fines (0' - 60' bgs) Sandpack (60' - 96' bgs) #12 Pipe (66.3' - 96' bgs) #12 Pipe (66.3' - 96' bgs) Total Depth of Well 96' bgs		
WELL LUG - יטטו - שבוווז וו.בס										

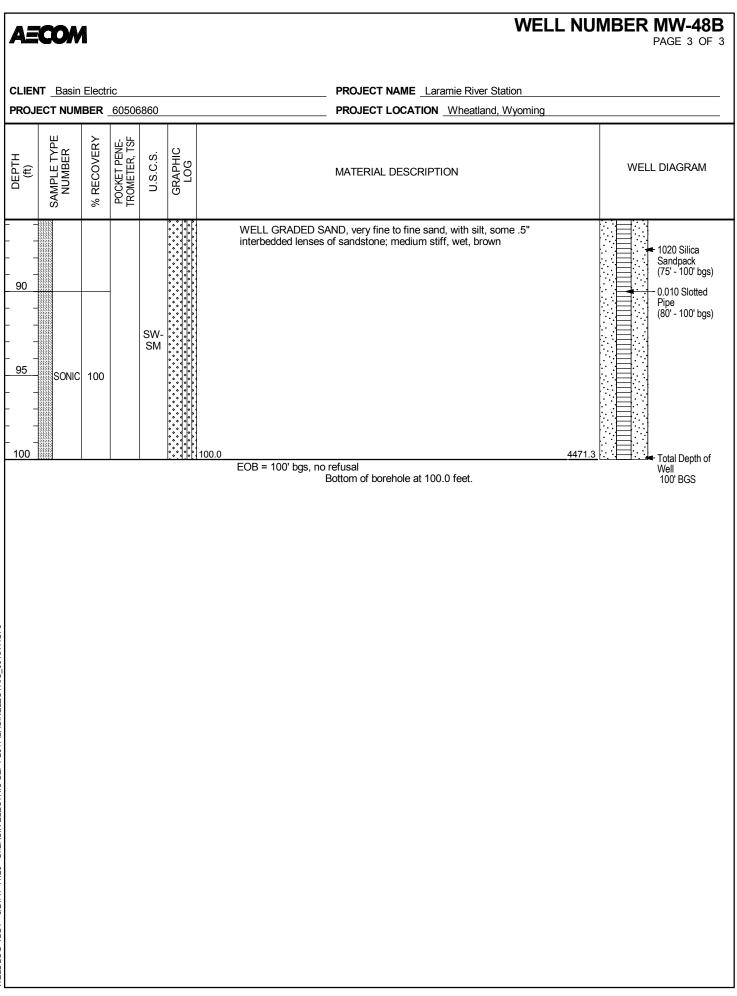
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DRILLING CONTRACTO DRILLING METHOD LOGGED BY	506860 1982 R <u>North</u> B-53 Ro	COMPLETED 8/21/1982 hern Testing Laboratories	AT END OF DRILLING		
DEPTH (ff) (ff) (ff) (ff) (ff) (ff) SAMPLE TYPE NUMBER % RECOVERY POCKET PENE-	TROMETER, TSF U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION		sing Top Elev: 0 (ft) sing Type: 4" PVC Pipe WELL DIAGRAM Top of Casing (estimated 4' ags)
	SANDST	34.0 SANDSTONE ¥	(Continued Next Page)	4531.1	ASTM C-33 Concrete Fines (0' - 55' bgs) 4" Sch. 40 PVC Pipe (4' ags - 90' bgs)

ΑΞ	ĊŎŴ						WE	ELL NUMBER MW-23B PAGE 2 OF 2			
CLIEN	IT Basin	Electr	ric				PROJECT NAME Laramie River Station				
PROJI	ECT NUM	IBER	60506	6860			PROJECT LOCATION Wheatland, Wyoming				
G DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION	WELL DIAGRAM			
 			SAN	NDST(DINE	SANDSTONE		ASTM C-33 Concrete Fines (0' - 55' bgs) Sandpack (55' - 90' bgs) #12 Pipe (60.2' - 90' bgs)			
							Bottom of borehole at 90.0 feet.	4475.1 Total Depth of Well 90' bgs			
90											

AECOM	WELI	L NUMBER MW-48B PAGE 1 OF 3
PROJECT NUMBER 60506860	PROJECT LOCATION _Wheatland, Wyoming	
DRILLING CONTRACTOR O'Ke DRILLING METHOD Sonic Track LOGGED BY C. Ahrendt	COMPLETED 7/8/2017 GROUND ELEVATION 4571.27 ft fe Drilling GROUND WATER LEVELS: GROUND WATER LEVELS: Rig AT TIME OF DRILLING CHECKED BY A. Lanning AT END OF DRILLING '28402.3 E Y AFTER DRILLING 80.21 ft / Elev 4	
DEPTH (ft) (ft) SAMPLE TYPE NUMBER % RECOVERY POCKET PENE- TROMETER, TSF U.S.C.S.	OH DO MATERIAL DESCRIPTION	Casing Top Elev: 0 (ft) Casing Type: 2" PVC Pipe WELL DIAGRAM Top of Casing (estimated 2.5" ags)
	SANDY SILT, with gravel, nonplastic, noncohesive; hard (>4.0 qu tsf), moist, brown	
ML	III 7.0 SILTY SAND, very fine to medium sand, with subrounded gravel; hard, moist, brown 10.0 SANDY SILT, with gravel, nonplastic; hard, moist, brown	4564.3 4561.3
SONIC 100	16.0 SILTY SAND, very fine to medium sand, with subrounded gravel; hard, moist, brown 20.0	4555.3
ML 	SANDY SILT, very fine to fine sand, with gravel, nonplastic, noncohesive; hard, moist, brown 22.0 SILTY SAND, very fine to fine sand, with subrounded gravel; hard, moist, brown 27.0	4549.3
	WELL GRADED GRAVEL, subrounded gravel, with very fine to medium sand; loose, moist, gray	
35 SONIC 100 	SILT, with sand, nonplastic, noncohesive; hard, wet (due to drilling), brown	

A	;CO/	N				WELL N	UMBER MW-48B PAGE 2 OF 3
	IT <u>Basi</u> ECT NU			6860		PROJECT NAME Laramie River Station PROJECT LOCATION Wheatland, Wyoming	
4 DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM
40			QL	JARTZ	TE	41.0 QUARTZITE; hard, moist, brown 453	0.3
				ML		SANDY SILT, nonplastic, noncohesive; soft, moist, brownish gray	
_ <u>45</u> - - - - 50	SONI	C 100	_ SAM	NDSTO) Ine	45.0 45.0 SANDSTONE, thinly bedded, sand grains visible; hard, moist, brown	6.3
 - 55 	SONI	C 100		SM		54.545 SILTY SAND, very fine to fine sand; loose, moist, brown	<u>6.8</u>
RIC_001017.6PU	SONI	C 100	-			60.0 45 WELL GRADED SAND, very fine to fine sand, with some silt; loose, moist, brown	1.3 • Neat Cement (0' - 71' bgs)
T 2017/BASINELECT			_	sw		S.A.A., .5" layers of sandstone S.A.A., .5" lenses of quartzite	
WELL LOGGDT - 9/21/17 11:28 - C:\BASIN ELECTRIC SEPT 2017/BASINELECTRIC_091917.GPJ	SONI	C 100				S.A.A., no quartzite or sandstone	 3/8" Bentonite Chips (71-75' bgs) 2" Sch. 40 PVC Pipe (2.5' ags - 100' bgs)
WELL LOG -: GDT - 9/21/17	SONI	C 100	-	SW GW- GM SW- SM		80.5 😤 WELL GRADED SAND: medium stiff, moist to wet, brown	10.8 10.3 1020 Silica Sandpack (75' - 100' bgs) 0.010 Slotted Pipe (80' - 100' bgs)



	Electi	605068	860					MBER MW-49E PAGE 1 OF 3 R TYPE Not Applicable
DRILLING CON	ITRAC	TOR _)'Kee	efe Dri	ling	GROUND WATER LEVELS:		
DRILLING MET						AT TIME OF DRILLING		
LOGGED BY _						A. Lanning AT END OF DRILLING Y AFTER DRILLING _79.22 ft / Elev 44		
DEPTH (ft) SAMPLE TYPE NUMBER	% RECOVERY	цŖ	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION	+07.75 IL	Casing Top Elev: 0 (ft) Casing Type: 2" PVC Pipe WELL DIAGRAM
 			SW- SC			WELL GRADED SAND, very fine to fine sand, with clay and gravel; medium stiff, moist, brown	4504.0	(flush mount)
 	100		ML		5.0	SILT, with very fine to fine sand, nonplastic, noncohesive; medium stiff, moist, brown	<u>4564.0</u> <u>4562.0</u>	
 			SW- SC			WELL GRADED SAND, very fine sand, with clay; medium stiff, moist, brown S.A.A., little gravel		Neat Cement (0' - 74' bgs)
 	100				12.0	SILT, with very fine to fine sand, little gravel; hard, moist, grayish brown @17-17.4' bgs: S.A.A., organic-like odor, dark gray	4555.0	2" Sch. 40 PVC Pipe (0' bgs - 100' bgs)
 20 			ML		24.0	S.A.A., no odor S.A.A., with gravel	4543.0	
	100		SW- SM			WELL GRADED SAND, very fine to fine sand, with silt; hard, moist, brownish gray SILT, with sand, nonplastic, noncohesive; hard, moist, brownish gray	4541.0	
	100		SW- SM		32.0	WELL GRADED SAND, very fine to fine sand, with silt, no gravel; hard, moist, brownish gray	4535.0	
			GW ML		38.0 39.0 40.0	WELL GRADED GRAVEL, some very fine to fine sand, trace fines; loose, moist, brown	<u>4529.0</u> <u>4528.0</u> 4527.0	

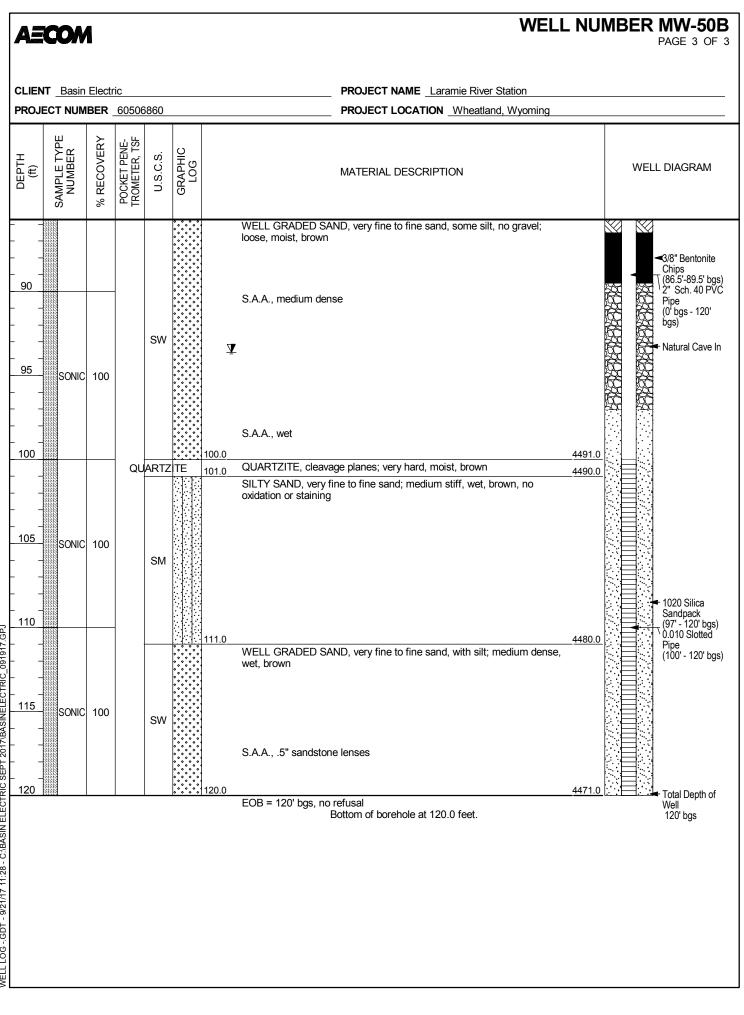
AE	CON	1				WELL	NUN		/W-49B AGE 2 OF 3
CLIEN	IT Basir	1 Electr	ic			PROJECT NAME Laramie River Station			
PROJ	ECT NUN	BER .	60506	6860		PROJECT LOCATION Wheatland, Wyoming			
(ff) 6	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION		WELL [DIAGRAM
 - 45 50	SONIC	70		SW- SM		SILT, with sand, nonplastic, noncohesive; hard, moist, light brown WELL GRADED SAND, very fine to fine sand, with silt; dense, moist, brown	4517.0		
 	SONIC	2 100		sw		WELL GRADED SAND, very fine to fine sand, little silt; loose, moist, brown			
	SONIC	: 100	SAN	<u>USTO</u> SW		63.0 63.5 SANDSTONE, horizontal fractures, trace sand; hard, dry, white brown- WELL GRADED SAND, very fine to fine; loose, moist, brown	<u>4504.0</u> <u>4503.5</u>		leat Cement 0' - 74' bgs)
	SONIC	: 100	SAN	IDSTO SW		76.0 SANDSTONE, horizontal fractures; hard, moist, whitish brown WELL GRADED SAND, very fine to fine sand, little silt; medium dense, moist, brown ¥	<u>4492.0</u> <u>4491.0</u>	C C C C C C C C C C C C C C	6/8" Bentonite hips 74-78' bgs) "" Sch. 40 PVC bipe 0' bgs - 100'
80 80 87 87 87 85	SONIC	: 100	SAN	SW		WELL CRAPER CAND your fire to fire could stiff resist known	<u>4487.0</u> 4486.0	b 1 S ((ogs) 020 Silica Sandpack 78' - 100' bgs) 0.010 Slotted Pipe 80' - 100' bgs)

AE	CON	1				WELL	NUME	BER MW-49B PAGE 3 OF 3		
	IT <u>Basin</u> ECT NUM			860		PROJECT NAME Laramie River Station PROJECT LOCATION Wheatland, Wyoming				
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION		WELL DIAGRAM		
90	SONIC	100		IDST(_		476.0 475.0			
- - 100						100.0 44 EOB = 100' bgs, no refusal Bottom of borehole at 100.0 feet.	467.0	Total Depth of Well 100' bgs		

WELL NUMBER MW-50B AECOM PAGE 1 OF 3 **PROJECT NAME** Laramie River Station CLIENT Basin Electric PROJECT NUMBER 60506860 PROJECT LOCATION Wheatland, Wyoming COMPLETED _7/6/2017 GROUND ELEVATION 4590.95 ft HAMMER TYPE Not Applicable DATE STARTED 7/6/2017 DRILLING CONTRACTOR O'Keefe Drilling **GROUND WATER LEVELS:** DRILLING METHOD Sonic Track Rig AT TIME OF DRILLING _---LOGGED BY C. Ahrendt ___ CHECKED BY _A. Lanning AT END OF DRILLING ----COORDINATES 586231.9 N 728742.5 E **AFTER DRILLING** <u>93.61 ft / Elev 4497.34 ft</u> Casing Top Elev: 0 (ft) Casing Type: 2" PVC Pipe POCKET PENE-TROMETER, TSF SAMPLE TYPE NUMBER RECOVERY GRAPHIC LOG U.S.C.S. DEPTH (ft) WELL DIAGRAM MATERIAL DESCRIPTION % 0 SILT, with sand, little gravel, nonplastic, noncohesive; stiff, moist (wet (flush mount) due to drilling), light brownish gray 5 SONIC 80 ML S.A.A., increasing gravel Neat Cement 10 10.0 4581.0 (0' - 86.5' bgs) GW 11.0 WELL GRADED GRAVEL, rounded gravel, with silt and very fine to fine 4580.0 2" Sch. 40 PVC sand; loose, moist, light brownish gray Pipe SILT, with very fine sand, nonplastic, noncohesive; stiff, moist (wet due (0' bgs - 120' to drilling), light brownish gray bgs) ML 15 SONIC 100 16.0 4575.0 SANDY SILT, very fine sand, nonplastic, noncohesive; stiff, moist, light brownish gray LOG -. GDT - 9/21/17 11:28 - C:\BASIN ELECTRIC SEPT 2017\BASINELECTRIC 091917. GP. 20 ML 25 SONIC 100 27.0 4564.0 GRAVELLY SILT, nonplastic, noncohesive; stiff, moist, brown ML 30 30.0 4561.0 SANDY SILT, with gravel, nonplastic, noncohesive; stiff, moist, light brown 35 SONIC 100 ML WELLI 40 40.0 4551

A	COA	A				WELL NU	MBER MW-50B PAGE 2 OF 3
CLIEN	NT Basii	n Elect	ric			PROJECT NAME Laramie River Station	
				6860		PROJECT LOCATION Wheatland, Wyoming	
(ff) 40	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM
	-			sw		WELL GRADED SAND, very fine to medium sand; medium stiff, moist, brown 44.0 4547.0	
<u>45</u> 50	SONIC	100		ML SW		45.0 SILT, with very fine sand, trace rounded gravel, nonplastic, noncohesive; medium stiff, moist, light brown WELL GRADED SAND, very fine to medium sand; loose, moist, brown, no oxidation	
 - 55 60	SONK	100	SAN	NDSTO		50.5 4540.5 SANDSTONE; very stiff, dry, brown 4540.5 59.0 4532.0 60.0 WELL GRADED SAND, very fine to medium sand, little silt, trace gravel; 4531.0	
	SONI	100				loose, moist, brown WELL GRADED SAND, very fine to fine sand, with <1" interbedded sandstone lenses, some silt; loose, moist, brown	Neat Cement (0' - 86.5' bgs)
MELLLOG - 601 1128 - C: BASIN ELECTRIC SEPT 2017 BASIN ELECTRIC 691917 642	SONIC	2 100		SW		S.A.A., with fine silt and .5-1" sandstone lenses	2" Sch. 40 PVC Pipe (0' bgs - 120' bgs)
3 - GDT - 9/2 [.]			SAN	NDST	<u>XNE</u>	81.0 4510.0 82.0 SANDSTONE, blocky, with little sand; ahrd, dry, brown 4509.0 WELL GRADED SAND, very fine to fine sand, some silt, no gravel; loose, moist, brown 4509.0	
85	SONIC	100		SW		(Continued Next Date)	

(Continued Next Page)



WELL LOG -.GDT - 9/21/17 11:28 - C:\BASIN ELECTRIC SEPT 2017\BASINELECTRIC_091917.GPJ

CLIENT Basin Electric	WELL NUMBER MW-51 PAGE 1 OF PROJECT NAME Laramie River Station
PROJECT NUMBER _60506860	
	GROUND ELEVATION 4591.51 ft HAMMER TYPE Not Applicable
DRILLING CONTRACTOR O'Keefe Drilling	GROUND WATER LEVELS:
DRILLING METHOD Sonic Track Rig	AT TIME OF DRILLING
LOGGED BY C. Ahrendt CHECKED BY A. Lanning	
COORDINATES 586215.1 N 729312.9 E	AFTER DRILLING 95.22 ft / Elev 4496.29 ft
DEPTH SAMPLE TYPE NUMBER % RECOVERY POCKET PENE- TROMETER, TSF U.S.C.S. GRAPHIC LOG	Casing Top Elev: 0 (ft) Casing Type: 2" PVC Pipe MATERIAL DESCRIPTION WELL DIAGRAM
GRAVELLY CL/	2" Sch. 40 PV Pipe (0' bgs - 120' bgs)
GC Sand and clay; d	4571.5 D GRAVEL, rounded gravel, with very fine to coarse dense, moist, light brownish gray D SAND, very fine to medium sand, with clay; hard, vn
25 SONIC 100	
30 30.0	4556.5

(Continued Next Page)

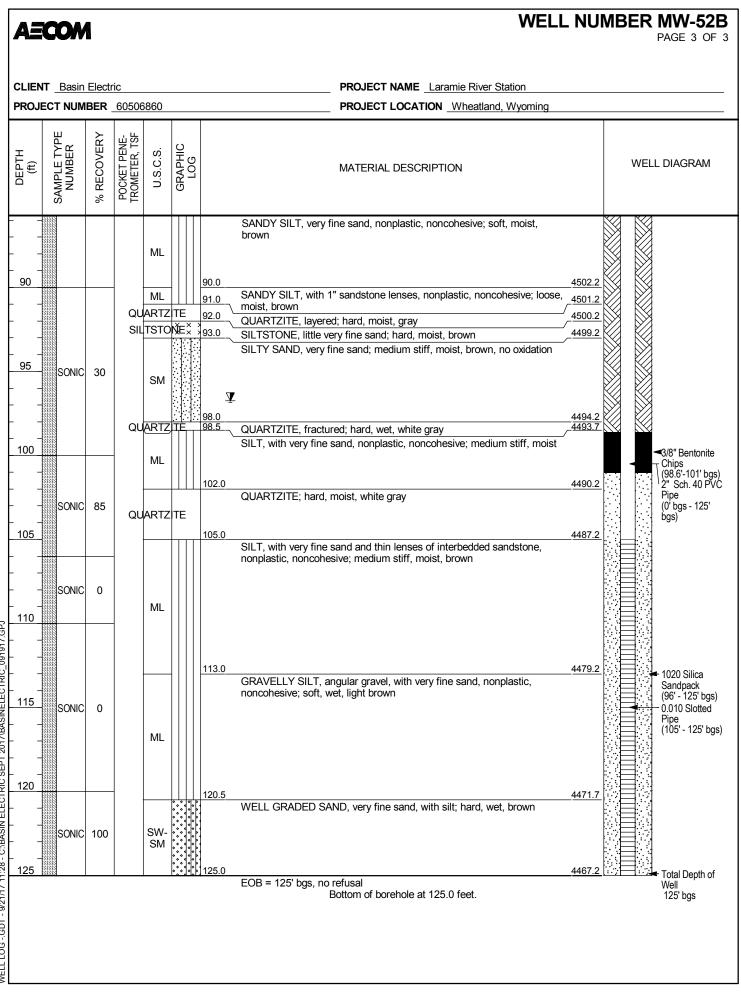
A		M						WELL	. NU	JMBER MW-51E PAGE 2 OF
CLIEI	NT _B	asin	Electr	ic				PROJECT NAME _ Laramie River Station		
PROJ		NUM	BER	60506	6860			PROJECT LOCATION Wheatland, Wyoming		
40 DEPTH	SAMPLE TYPE	NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION		WELL DIAGRAM
								SILT, with very fine sand, nonplastic, noncohesive; hard, moist, light brown		
	- - - so	ONIC	100		SW- SM		43.0	WELL GRADED SAND, very fine to medium sand, with silt; loose, moist, light brown, mottled gray and orange	4548.5	5
- · · - · ·	-				sw		48.0	WELL GRADED SAND, fine to coarse sand, little silt; loose, moist, dark gray	4543.5 4542.0	
	-						51.0	S.A.A., with rounded gravel SILT, trace very fine sand, nonplastic, noncohesive, soft; loose, moist, light brown	4540.5	5
 55	- - - so	ONIC	100				55.0	S.A.A., hard, moist	4536.5	5
 	-						60.0		4531.5	
WELL LOG - GDT - 921/17 11:28 - C:\BASIN ELECTRIC SEPT 2017\BASINELECTRIC 091917.GPU	- - - - - - - - - - - - - - - -	DNIC	100		ML		00.0	S.A.A., hard	4001.0	Neat Cement (0' - 93.5' bgs)
017/BASIN	-				ML		70.0	SANDY SILT, very fine sand, nonplastic, noncohesive; medium stiff, moist, light brown	4521.5	5 2" Sch. 40 PVC Pipe (0' bgs - 120' bgs)
SIBASIN ELECTRIC	- - SC -	ONIC	100	SAN	<u>UDSTO</u>	2NE	74.5	SANDSTONE; hard, moist, brown SILTY SAND, fine sand; medium dense, moist, light brown	4517.0 - 4516.5	
9/21/17 11:28 - 0					SM		80.0	S.A.A., little gravel	4511.5	5
MELL LOGGDT	- - - - SC	ONIC	70							

EA	CON						WEL	L NU	MBER MW-51B PAGE 3 OF 3
CLIEN	IT Basin	Electr	ic				PROJECT NAME Laramie River Station		
PROJE		BER	60506	6860			PROJECT LOCATION Wheatland, Wyoming		
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION		WELL DIAGRAM
				SM			/ SAND, fine sand, little gravel; medium dense, moist, light bro	own <u>4502.5</u>	
90			SAN	IDSTO		. 30.0	STONE, thinly bedded; hard, moist, brown nonplastic, noncohesive, massive; very stiff, moist, light grayish	4501.5	
<u>95</u> 100	SONIC	40		ML		⊻ 100.0		4491.5	 3/8" Bentonite Chips (93.5'-96.0' bgs) 2" Sch. 40 PVC Pipe (0' bgs - 120' bgs)
 <u>105</u>	SONIC	100		<u>SM</u>		stiff, w expan	SAND, very fine sand, little sandstone fragments; medium ret, brown, no oxidation (100-105' bgs: expansive soil, ded 10%)	4486.5	
 	SONIC	100				110.0 WELL	GRADED SAND, very fine to fine, with sandstone; very stiff,	<u>4481.5</u>	■ 1020 Silica Sandpack (96' - 120' bgs) ■ 0.010 Slotted Pipe
 	SONIC	100	MU	SW DSTC		113.0 MUDS very st 115.0 10%)	ght brown (110-115' bgs: expansive soil, expanded 10%) STONE, with very fine to fine sand, layered, thinly bedded, plat tiff, moist, light brown (115-120' bgs: expansive soil, expanded with sand, very fine sand, nonplastic, noncohesive; medium stiff,	4476.5	(100' - 120' bgs)
 120	SONIC	100		ML		moist, 120.0	light brown	<u>4471.5</u>	
						EOR =	E 120' bgs, no refusal Bottom of borehole at 120.0 feet.		Well 120' bgs

LL LOG -.GDT - 9/21/17 11:28 - C.\BASIN ELECTRIC SEPT 2017\BASINELECTRIC 0

EA	00	M							WEL	L NU	MBER MW-52 PAGE 1 OF
CLIENT	Г_Ва	asin E	Electr	ric					PROJECT NAME Laramie River Station		
									PROJECT LOCATION _ Wheatland, Wyoming		
DATE S	STAF	RTED	6/2	23/2017	•		COMP	LETED <u>6/27/2017</u>	GROUND ELEVATION _4592.21 ft	HAMME	ER TYPE Not Applicable
									GROUND WATER LEVELS:		
				Sonic ⁻							
								A. Lanning			
	JINA	IES	_586	5198 N	72	9809.	/ E		AFTER DRILLING 96.75 ft / Elev	1495.46 f	
(ft)	SAMPLE TYPE	NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG			MATERIAL DESCRIPTION		Casing Top Elev: 0 (ft) Casing Type: 2" PVC Pipe WELL DIAGRAM
0					CL	////	0.8	Top .75" road bed		4591.5	Top of Casing (flush mount)
- - - - - - - - -	sc	DNIC	80		ML			SANDY SILT, very fi grayish brown	ne sand, nonplastic, noncohesive; soft, moist,		- Neat Cement
10							10.0			4582.2	
					ML			SILT, with sand, non brownish gray	plastic, noncohesive; very stiff (3.0 qu tsf), moist,		2" Sch. 40 P
	sc	NIC	90				12.0	SANDY SILT, trace	gravel, nonplastic, noncohesive, no odor or staining	<u>4580.2</u>	(0' bas - 125'
-					ML		14.0	medium dense, mois	st, brownish grat	4578.2	bgs)
15								SANDY CLAY, lean,	subrounded gravel, nonplastic, noncohesive, no soft, moist (wet due to drilling), light grayish brown		
- - - 20	sc	NIC	90		CL		20.0	S.A.A., decreasing g		4572.2	
20				1			20.0	SANDY CLAY, lean;	hard (>4.0 qu tsf), moist, light brownish gray		
 30	sc	DNIC	95		CL		30.0	S.A.A., increasing sa	and, little subangular gravel; moist	4562.2	
		+		1	GW		31.0	WELL GRADED GR	AVEL, subrounded gravel; loose, moist (wet due to		
- - - 35			0.5		CL		35.0	drillering), gray SANDY CLAY, with	gravel; very stiff, moist, grayish brown	4557.2	
-	SC	NIC	80		SW				ND, very fine to fine sand, with white deposits ff, moist, brown, no oxidation		

A	ECON	1				WELI	_ NUMI	BER MW-52B PAGE 2 OF 3
CLIE	NT Basir	Electr	ric			PROJECT NAME Laramie River Station		
PRO.	JECT NUN	IBER	60506	860		PROJECT LOCATION Wheatland, Wyoming		
4 (ff)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION		WELL DIAGRAM
- - - - - - - - - - - 50		80		SW		42.0 S.A.A., dark brown SILTY SAND; loose, moist (wet due to drilling), light brownish gray S.A.A., very fine sand; hard, brown, no oxidation	4550.2	
- - - <u>55</u> - - -		100	SAN		NIE.	S.A.A., thin lenses of 1-2 mm of calcite deposits S.A.A., light olive green with brown oxidation spots 58.5 S.A.A., trace gravel; light brown SANDSTONE, very fine to medium sand, thinly bedded; hard, moist,	4533.7	
06 		100	QU	ARTZ SW- SM		60.3 light brown 3" of quartzite, hard, laminated, visible sand SAND, with silt; loose, moist, light brown S.A.A., blocky lenses of sand with silt, very fine to fine sand; moist S.A.A., hard S.A.A., very fine to fine sand, soft; loose S.A.A., with little gravel	4532.2 -4532.0/	Neat Cement (0' - 98.6' bgs)
WELL LOGGDT - 9/21/17 11:28 - C:\BASIN ELECTRIC SEPT 2017\BASINELECTRIC_091917.GPJ 9		30	QU	SP- SM		70.3 3" of quartzite, hard, laminated, visible sand SAND, with silt, very fine to fine sand, no odor or stain; loose, moist (we due to drilling), brown 80.0 SANDY SILT, very fine sand, nonplastic, noncohesive; soft, moist, brown	4522.0 t 4512.2	2" Sch. 40 PVC Pipe (0' bgs - 125' bgs)

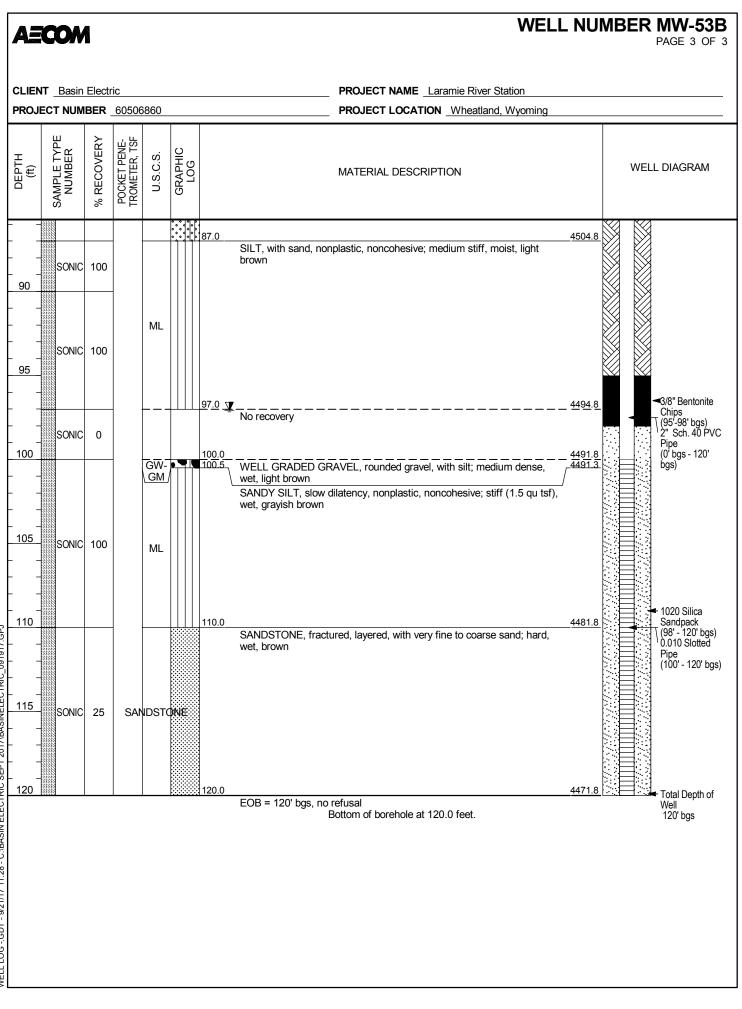


WELL LOG - GDT - 9/21/17 11:28 - C:\BASIN ELECTRIC SEPT 2017\BASINELECTRIC_091917.GPJ

	<u> </u>	-	ic					PRO	WE ECT NAME Laramie River Station	LL NUI	MBER MW-53I PAGE 1 OF
PROJECT				3860					ECT LOCATION Wheatland, Wyoming		
						COMP	LETED 6/28/2017		GROUND ELEVATION _4591.84 ft		R TYPE Not Applicable
									GROUND WATER LEVELS:		
DRILLING									AT TIME OF DRILLING		
LOGGED	BY _(C. Ahr	endt	(CHECK	(ED B)	A. Lanning		AT END OF DRILLING		
COORDIN	IATES	586	189 N	73	0213.8	B E			AFTER DRILLING 97.06 ft / Ele	v 4494.78 ft	t
O DEPTH (ft) SAMPI F TYPF	NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG			MATE	RIAL DESCRIPTION		Casing Top Elev: 0 (ft) Casing Type: 2" PVC Pipe WELL DIAGRAM
 	SONIC	10		<u>GW</u>		0.5			road bed), with sand; loose, dry, gray nplastic, noncohesive; stiff, dry, brown	4591.3 /	Top of Cacing (flush mount)
						<u>10.0</u>	SILT, with sand, bec brown @10-12' bgs: with V		nplastic, noncohesive; soft, moist, light POTS	<u>4581.8</u> _	- Neat Cement (0' - 95' bgs) - 2" Sch. 40 PVC Pipe (0' bgs - 120' bgs)
 	SONIC	100				16.5	WELL GRADED SA moist, brown, no oxi	ND, very dation or	/ fine to fine, with silt; hard (>4.0 qu tsf), staining	4575.3	
-	SONIC	100					S.A.A., medium den	-	brown hite calcite deposits, no odor or staining		
- - - 30 -				SW- SM							
	SONIC	60	N 41	DSTC		<u>36.5</u>	S.A.A., decreasing f		l; hard (>4.0 qu tsf), moist, brown (10YR	4555.3	
_]					1 N⊑	37.5	4/3)			4554.3	
			SAN	NDSTO	NE	40.0	SANDSTONE, little	silt and s	and; dense, moist, light brown	4551.8	

(Continued Next Page)

AE	COA	1				WELL NU	MBER MW-53B PAGE 2 OF 3
	T <u>Basir</u> ECT NUN			6860		PROJECT NAME _Laramie River Station PROJECT LOCATION _Wheatland, Wyoming	
DEPTH (ft)	SAMPLE TYPE NUMBER	% RECOVERY	POCKET PENE- TROMETER, TSF	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	WELL DIAGRAM
40 - 45 - 50	SONIC	30				No recovery <u>17.0</u> <u>SANDSTONE, some very fine sand, thinly bedded; dense, moist, light brown </u>	
 - 55 	SONIC	30	SAN	NDSTO SW		57.0 4534.8 WELL GRADED SAND, very fine to fine sand, trace silt; medium dense, moist, grayish brown 5.A.A., dense, moist, light brown S0.0 4531.8	
	SONIC	50		ML		GRAVELLY SILT, subrounded gravel, with very fine to fine sand, nonplastic, noncohesive; soft, moist, light brown	Neat Cement (0' - 95' bgs)
WELL LOG -GD1 - 9/21/17 11:28 - C:NANSIN ELECTRIC SEPT 2017/BASINELECTRIC_091917.GPU 38 0 24 0 0 49 1 1 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1	SONIC	: 100		SW- SM		70.0 4521.8 WELL GRADED SAND, very fine sand, with silt; loose, moist, light brown, no oxidation or staining	- 2" Sch. 40 PVC Pipe (0' bgs - 120' bgs)
MELL LOG - GDI - 9/21/17-17	SONIC	100				S.A.A., medium stiff, light brown	



WELL LOG -.GDT - 9/21/17 11:28 - C.\BASIN ELECTRIC SEPT 2017\BASINELECTRIC_091917.GPJ

Δ.					t Name:	Basin Electric Laramie River Station		
Bo	ring	Loc	y	Client:		Basin Electric Laramie River Station 60577052	Boring ID:	MW-38C
Date(s)					t Number:	J. Hurshman Checked	Total Depth of	Depth to
Drilled Drilling			9 - 4/6/1		By Diameter of	Ву	Borehole (ft) Ground Surface	Water (bgs)
Method Drill Rig		Hollow	Stem Au	ger	Borehole (in)	8 1/4 inch OD	Elevation (ft-msl) Groundwater	TBD
Туре			CME		Drilling Company	Inberg Miller	Elevation (ft-msl)	TBD
Driller's Na B	ame larry		Sampler Type			5 foot core barrel	Measuring Point Elevation (ft-msl)	TBD
Desc	ription	of Sam	ole Locat	ion			Northing Easting	TBD
	SA	MPL	ES	ō				
Depth (ft-bgs)	Run Number	Recovery (%)	Sample ID	USCS Symbol	PID (ppm)	MATERIAL DESCRIPTION		Well Construction
- - 1 -	-					Blind Drilled to 75 ft See MW-38B boring log for details	-	
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Accord Partial Contraction Boring ID: MVX-3 Date() 4007-4015 19804 1	Δ=				Project	Name:	Basin Electric Laramie River Station		
Bit Process Bit Process Deck Number / I way was an or of the first on the part of	Bo	rina	Lo	7	Client:		Basin Electric Laramie River Station	Boring ID:	MW-38C
Note: Note: <th< th=""><th></th><th></th><th></th><th><u> </u></th><th>Project</th><th>Number:</th><th>60577052</th><th></th><th></th></th<>				<u> </u>	Project	Number:	60577052		
protein transfer p pc protect (P) protec (P)	Date(s)			0 4/0/1	0	Logged	J. Hurshman Checked	Total Depth of	Depth to
Interdection Transition Because intraging 100 Cont Description Transition Description Transition			4/4/1	9 - 4/6/1	9	Ву	Ву		Water (bgs)
Diff PB Coc Pering Locg Mar Perindent mark Too Name Provident mark Non- Perindent mark 100 Name Provident mark Non- Perindent mark 100 Decident mark Provident mark Non- Perindent mark 100 Perindent mark Provident mark Non- Perindent mark Non- Perindent mark Provident mark Non- Perindent mark Non- Perindent mark Perindent mark Non- Perindent mark Non- Perindent mark Perindent mark Non- Perindent mark Non- Perindent mark Perindent mark Perindent mark Non- Perindent mark Perindent mark Perindent mark Perindent mark Perindent mark Perindent mark Perindent mark Perindent mark Perindent mark Perindent mark Perindent mark Perindent mark Perindent mark Perindent mark Perindent mark Perindent mark Perindent mark Perindent mark Perindent mark<			Hollow	Stem Au	iger		8 1/4 inch OD		TBD
Dirth Barget 16 of a lore bartef Meacure of a lore of a rate Meacure of a lore of a rate Top Image: State of a lore bartef Image: State of a lore	Drill Rig			CME		Drilling	Inberg Miller	Groundwater	TBD
Number line Set Marting Total Image: Solution of set in the set in the set of set in the set in the set of set in the	Driller's Na	ame larry		Sample Type	r		5 foot core barrel	Measuring Point	TBD
SAMPLES Bit of Difference 17.81 MATERIAL DESCRIPTION Well Construction 21 1<					tion			Northing	TBD
view view <th< th=""><th></th><th></th><th></th><th></th><th>T</th><th></th><th></th><th></th><th></th></th<>					T				
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21 See MW-388 boring log for datalis 22	<u>∩</u> €	Ř	Ř	ŝ	⊃ø	Ö	Diad Dellad to 75 ft		
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			_	Project	t Name:	Basin Electric Laramie River Station			1	
A	ΞC		1	Client:		Basin Electric Laramie River Station	Boring ID:		MW038C	
BC	oring	JLO	9	Project	Number:	60577052				
					1	l Huntman				
Date(s)		4/4/	19 - 4/6/1	9	Logged	J. Hurshman Checked	Total Depth of		Depth to	
Drilled Drilling					By Diameter of	Ву	Borehole (ft) Ground Surface		Water (bgs)	
Method		Hollow	Stem Au	iger	Borehole (in)	8 1/4 inch OD	Elevation (ft-msl)	TBD		
Drill Rig Type			CME		Drilling Company	Inberg Miller	Groundwater Elevation (ft-msl)	TBI)	
Driller's N	Name Barry		Sample Type	r		5 foot core barrel	Measuring Point Elevation (ft-msl)	TBI		
		n of Sarr	ple Loca	tion			Northing Easting	TBI)	
		SAMP			1				 	
				USCS Symbol	ő					
	- Per	Recovery (%)	۵	Syn	Graphic Log	MATERIAL DESCRIPTION				
oth		ver	ple	S	ihq			Well Cons	truction	
Depth (ft-bgs)	Run Number	Reco	Sample ID	NS N	Gra					
	-					Blind Drilled to 75 ft	-			
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				Projec	t Name:	Basin Electric Laramie River Station		
A	EC	ON		Client:		Basin Electric Laramie River Station	Boring ID:	MW-38C
Bo	oring	Lo	9		t Number:	60577052		
Date(s)		4/4/1	9 - 4/6/1	9	Logged	J. Hurshman Checked	Total Depth of	Depth to
Drilled Drilling			~ •		By Diameter of	By	Borehole (ft) Ground Surface	Water (bgs)
Method Drill Rig			Stem Au	ger	Borehole (in) Drilling	8 1/4 inch OD	Elevation (ft-msl) Groundwater	TBD
Type Driller's N	lamo		CME Sampler		Company	Inberg Miller 5 foot core barrel	Elevation (ft-msl) Measuring Point	TBD
	Barry		Type			S TOOL COTE DATTER	Elevation (ft-msl) Northing	TBD
Des	cription	of Sam	ple Loca	tion			Easting	TBD
	S	AMPI	ES	lo lo	5			
Depth (ft-bgs)	Run Number	Recovery (%)	Sample ID	USCS Symbol	Graphic Log	MATERIAL DESCRIPTI	ION	Well Construction
	-	œ	<i>i</i> n	-	Ŭ	Blind Drilled to 75 ft	_	
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	-							
75	-					end of blind drilling - switch to 5 foot core barrel very fine to fine sand, well sorted, homogeneous, brown, wet, loos	elv nacked few interhedded white	
-	-					burrow casts 79-80 ft (1-3 mm diameter)	ory passed, fow interbedded write	
76	-						-	
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77	1	75	N/A	SP	N/A			
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Dot Inig LOg Project Number: 60577052 Date(s) 4/4/19 - 4/6/19 Logged J. Hurshman Checked Total Depth of Depth to		-	~		Project	t Name:	Basin Electric	Laramie River Station		
Under the sort of a large thread of the lar	A=		N		Client:		Basin Electric	Laramie River Station	Boring ID	: MW-380
Multiple	Bor	ing	LΟį	9	Project	Number:	6	0577052		
Multiple						1				
Mode Denote the form Denote the form the	Date(s)		4/4/1	19 - 4/6/1	9	Logged	J. Hurshman	Checked	Total Depth of	Depth to
Matche Indian Transmission Benefitie (and the final point) Benefitie (and the final point) The point of the final point of the final point) The point of the final point								Ву		Water (bgs)
The base is a construction of a signal is construction is in any is a construction is a construction is a construction is a construction in any is a construction is construction is a construction is a construction is a constructi	Method		Hollow	Stem Au	iger		;	8 1/4 inch OD		TBD
Differ Display Display <thdisplay< th=""> <thdisplay< th=""> <thdi< td=""><td></td><td></td><td></td><td>CME</td><td></td><td>Drilling Company</td><td>I</td><td>nberg Miller</td><td></td><td>TBD</td></thdi<></thdisplay<></thdisplay<>				CME		Drilling Company	I	nberg Miller		TBD
Bit Perception Steps Locative Total Status No. Attribute Status Attribute Status No. Attribute Status Attr	Driller's Na				r	oompany	5 foot core barrel		Measuring Point	TBD
Number of starget Locator Teaching Teaching <thteaching< th=""> Teaching Teach</thteaching<>	Ba	arry		Туре						
····································	Descr	ription	of Sam	ple Loca	tion					IBD
view view <th< td=""><td></td><td>SA</td><td>AMPI</td><td>ES</td><td>1</td><td>1</td><td></td><td></td><td></td><td></td></th<>		SA	AMPI	ES	1	1				
81 2 60 NA 50 NA 82 2 60 NA 50 62.11 free sand as above, well fee interbedded burrow casts while 83 60 62.11 free sand as above, well sorted, well easy to drill, flowing sand free 85-86.5 ft bgs, minor sitt or wergine sand 84 65 65.01 fr. fine sand as above, well sorted, well easy to drill, flowing sand free 85-86.5 ft bgs, minor sitt or wergine sand 86 65.01 fr. fine sand as above, well sorted, well easy to drill, flowing sand free 85-86.5 ft bgs, minor sitt or wergine sand 86 65.01 fr. fine sand as above, lightly packed, alow to drill, not comanted 91 65.00 fr. fine sand as above, lightly packed, alow to drill, not comanted 91 65.01 fr. fine sand as above, lightly packed, alow to drill, not comanted 91 65.01 fr. fine sand as above, lightly packed, alow to drill, not comanted 91 65.01 fr. fine sand as above, light sand, interbedded white burrow casts, wet, top foot is liquified (souply), possibly dub 93 65.100 fr. fine sand os above, light sand, interbedded white burrow casts, wet, top foot is liquified (souply), possibly dub 94 95 100 fr. fine sand or above, light sand, interbedded white burrow casts, wet, top foot is liquified (souply), possibly dub 96 100 fr. f	- t -				lod	Ð				
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81 2 60 NA 50 NA 82 2 60 NA 50 62.11 free sand as above, well fee interbedded burrow casts while 83 60 62.11 free sand as above, well sorted, well easy to drill, flowing sand free 85-86.5 ft bgs, minor sitt or wergine sand 84 65 65.01 fr. fine sand as above, well sorted, well easy to drill, flowing sand free 85-86.5 ft bgs, minor sitt or wergine sand 86 65.01 fr. fine sand as above, well sorted, well easy to drill, flowing sand free 85-86.5 ft bgs, minor sitt or wergine sand 86 65.01 fr. fine sand as above, lightly packed, alow to drill, not comanted 91 65.00 fr. fine sand as above, lightly packed, alow to drill, not comanted 91 65.01 fr. fine sand as above, lightly packed, alow to drill, not comanted 91 65.01 fr. fine sand as above, lightly packed, alow to drill, not comanted 91 65.01 fr. fine sand as above, light sand, interbedded white burrow casts, wet, top foot is liquified (souply), possibly dub 93 65.100 fr. fine sand os above, light sand, interbedded white burrow casts, wet, top foot is liquified (souply), possibly dub 94 95 100 fr. fine sand or above, light sand, interbedded white burrow casts, wet, top foot is liquified (souply), possibly dub 96 100 fr. f	ept js)	N N	SOV	du	ŝ	rap				
81 9 NA 9P NA 82 9 NA 9P 62.1: fine sand as above, well kentees from 62.7 to 82.8 ft. 84 9 84 9 84 9 84 9 9 9 9 9 86 9 9 9 9 9 87 3 100 NA 9 9 88 9 9 9 9 9 89 9 9 9 9 9 91 9 9 9 9 9 91 9 9 9 9 9 91 9 9 9 9 9 91 9 9 9 9 9 9 91 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 10 10 10 10 10 9 9	کَ کَ	Ru	Re	Sa	D	G	as above little recivery 80-82 f	t		
82 2 00 NA SP 62 ft. fine sand as above, well, few interbedded burrow casts, while 84 9 9 9 9 9 9 84 9 9 9 9 9 9 90 9 9 9 9 9 9 91 9 9 9 9 9 9 94 90 90 9 9 9 9 94 90 9 9 9 9 9 9 94 90 9 9 9 9 9 9 9 94 5 100 NA 9 9 9 9 9 94 5 100 NA 9 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td>N/A</td> <td></td> <td></td> <td>-</td> <td></td>						N/A			-	
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2 60 N/A 83 - - 84 - - 84 - - 84 - - 84 - - 85 - - 86 - - 87 3 100 N/A 86 - - 87 - - 88 - - 89 - - 90 - - 91 - - 92 - - 93 - - 94 - - 95 - - 96 - - 97 - - 98 - - 98 - - 98 - - 98 - - 98 - - 98 - - 98 - - 98 </td <td> -</td> <td></td> <td></td> <td></td> <td>SP</td> <td></td> <td>_</td> <td></td> <td>-</td> <td></td>	-				SP		_		-	
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a -	-	2	60	N/A			82 ft: fine sand as above, wet,	few interbedded burrow casts-white	-	
a -	., -				~		82.7: grow this alow loss d	a thick soft clay four locase from 00.7	7 to 82 8 ft	
85	°3 _				UL	1	oz.r. gray tilli tiay lens, ~1 Ch	in thion, build day, rew relises from 82.7	10 02.0 IL	
85	-								_	
86 1 100 N/A 87 3 100 N/A 88 90 5 100 N/A 90 90 90 90 90 90 90 90 90 90 91 90 90 90 90 92 4 50 N/A 90 93 4 50 N/A 90 94 90 90 90 90 93 4 50 N/A 90 94 50 N/A 90 90 95 100 N/A 90 90 94 5 100 N/A 100 95 100 N/A 100 100 96 100 N/A 100 100 98 5 100 N/A 100 100 N/A 100 100 100 100 100 100 100 100 100 100 100 100<	84								_	
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36 3 100 NA 87 3 100 NA 90 - - 90 - - 90 - - 90 - - 91 - - 92 - - 93 - - 94 - - 95 - - 96 - - 97 - 5 100 NA - 96 - - 97 - - 98 - - 5 100 NA 100 NA - 101 - - 102 - - 103 - - - 104 - - - 105 100 - - 104 - - - 105 100 NA - 104 -	85				1		85-90 ft: fine cond on obsure	all corted wet eacy to drill flowing as	and from 85-86 5 ft hos minor silt or use if	
87 3 100 NA 88 9 1 1 90 1 1 1 91 1 1 1 92 1 1 1 93 4 50 NA 94 1 1 1 95 1 1 1 96 1 1 1 97 5 100 NA 98 5 100 NA								en sonteu, wet, easy to drill, flowing sa	and noni 00-00.0 it bys, minor slit of very fi	
87 3 100 NA 88 9 1 1 90 1 1 1 91 1 1 1 92 1 1 1 93 4 50 NA 94 1 1 1 95 1 1 1 96 1 1 1 97 5 100 NA 98 5 100 NA	86								-	
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3 100 NA 88 - - 99 - - 90 - - 90 - - 91 - - 92 - 4 93 - - 94 - - 95 - - 96 - - 97 - - 98 - 100 98 - 100 98 - 100 98 - 100 98 - 100 98 - 100	87						-		-	
89 -	, °, _	3	100	N/A			=			
89 -							-		_	
88 -	88				SP				_	
88 -	-						-		-	
90 90-95 ft: fine sand as above, tightly packed, slow to drill, not cemented 91 91 92 4 93 4 94 50 95 100 96 5 97 5 98 5 100 N/A	89									
90 91 91 92 94 94 95 96 96 97 98 5 100 N/A 91							-		-	
91 -	90				1		00.05 ftt fina and an all in the	ability people of a low to down a state of the		
92 4 50 N/A 93 4 50 N/A 94 94 95 100 f: as above, tight sand, interbedded white burrow casts, wet, top foot is liquified (soupy), possibly due to drilling - slight gray color 96 97 5 100 N/A 98 5 100 N/A							90-95 ft: fine sand as above, ti	gnuy packed, slow to drill, not cemente	eu	
92 4 50 N/A 93 4 50 N/A 94 94 95 100 f: as above, tight sand, interbedded white burrow casts, wet, top foot is liquified (soupy), possibly due to drilling - slight gray color 96 97 5 100 N/A 98 5 100 N/A	91								-	
92 4 50 N/A 93 4 50 N/A 94 94 95 100 ft: as above, tight sand, interbedded white burrow casts, wet, top foot is liquified (soupy), possibly due to drilling - slight gray color 96 97 5 100 N/A 98 5 100 N/A	-				1		=			
93 4 50 N/A 93 - - - 94 - - - 95 - - - 96 - - - 97 - - - 98 - 5 100 N/A	92						-		-	
93 93 94 94 95 95 96 96 97 97 98 5 100 N/A	<u> </u>		50	N/A			-			
94 -		4	50	IN/A			-		_	
95 -	93 _						_		-	
95 -	-						-		-	
96 - 5 100 N/A 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	94				1		F		-	
96 - 5 100 N/A 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4							=		-	
96 - 97 - 5 100 N/A 1ast 2 feet slow hard drilling	95				_		-	for a state of solution to the state	ten fast in Devillad (n. 1997)	
96 -	-						95-100 ft: as above, tight sand to drilling - slight gray color	, interpedded white burrow casts, wet,	top root is liquified (soupy), possibly due	
97 -	96								-	
98 5 100 N/A 98 1 1 1 1 1 1 1							-			
98 5 100 N/A 98 1 1 1 1 1 1 1							=		-	
98	9/						-		-	
_ last 2 feet slow hard drilling	-	5	100	N/A	1				-	
	98 _						last 2 feet slow hard drilling			
							-		-	
	99 _				1		-		-	
	-						-		-	
	100						-			
				1	1					I I

				Project	t Name:	Basin Electric Lara	amie River Station			
A Bor		DN		Client:		Basin Electric Lara		Boring ID:		MW-38C
Boi	ring	Lo	9		Number:	6057		Bornig ib.	-	
				Појес	Number.					
Date(s)		<u> </u>	9 - 4/6/1	٩	Logged	J. Hurshman C	thecked	Total Depth of	I	Depth to
Drilled			3 - 4/0/1	5	Ву	в	ly .	Borehole (ft)	1	Water (bgs)
Drilling Method		Hollow	Stem Au	ger	Diameter of Borehole (in)	8 1/4	inch OD	Ground Surface Elevation (ft-msl)	TBD	
Drill Rig			CME		Drilling	Inberg	g Miller	Groundwater	TBD	
Type Driller's Na	me		Sampler	r	Company	5 foot core barrel	·	Elevation (ft-msl) Measuring Point	TBD	
В	arry		Туре					Elevation (ft-msl) Northing		
Desc	ription	of Sam	ple Loca	tion				Easting	TBD	
	S	١٩M	ES	1	1					1
÷				-						
(f		_		USCS Symbol	bo					
	ber	(%)	۵	Syn	C C	M	ATERIAL DESCRIPTION		Well Const	ruction
f ~	Nun	very	ple I	ŝ	phi					
Depth bgs)	Run Number	Recovery (%)	Sample ID	nsc	Graphic Log					
		-	N/A	SP	N/A	as above, fine to very fine sand, we	t, trace white burrow casts, few hard nodule ace silt in sand	s of sand,		
101 -			1975	°,	INVA	top toot to soupy satiu (ilquilled), the	and one in Janu	-		
101				1		⊨- -		_		
<u> </u>						L		_		
102						 _		_		
- 1	6	90		1		L_ _		-		
103								-		
				1				-		
104						-		-		
-						-		-		
105						silty fine sand very soft and souny	wet 105 to 107 ft, then more compact and	tight sand light gray/medium gray		
- 1						thin lens at 107.5 ft in sand. Thinly	bedded silt lenses < 2 mm thick from 108 to	o 109 feet.		
106						-		_		
						=		-		
107						=		-		
	7	100				-		-		
108						-		-		
-						=				
109						=		-		
-						-				
110						<u>-</u> -				
-						as above, top 2.1 feet is wet and so	upy fine sand			
111						⊨- F		=		
^m -						┝─ ┍		-		
						┝─ ┍		-		
112		50				112-115: fine sand with interbedded	2 mm thick silt lessen every 1 to 1.5 cm, w	vet, tan to brown color		
	8	50		1		 -		-		
113						-		-		
- 1								-		
114								-		
-								1		
115						as above, top 2.5 feet very soupy, fa	alls out of core barrel	-		
-								-		
116				1		F F		=		
-						+		-		
117						F		-		
- 1	9	60		1		- -		-		
118						-		7		
- 1				1		118-120: interbedded very fine sand	d and silt, wet, brown to tan,			
119						-				
-						F F		-		
100 -						⊨ -				
120				1	1	<u> </u>			1 1	

				Projec	t Name:	Basin Electric Laramie	e River Station			
A	ring	MC		Client:		Basin Electric Laramie	e River Station	Boring ID	: MW-	-38C
	iiig	20	1	Projec	t Number:	60577052	2			
Date(s)					Logged	J. Hurshman Checke		Total Depth of	Depth to	,
Drilled		4/4/1	9 - 4/6/1	9	Ву	Ву		Borehole (ft)	Water (b	
Drilling Method		Hollow	Stem Au	ger	Diameter of Borehole (in)	8 1/4 inch 0	OD	Ground Surface Elevation (ft-msl)	TBD	
Drill Rig Type			CME		Drilling Company	Inberg Miller	er	Groundwater Elevation (ft-msl)	TBD	
Driller's Na	ame Barry		Sampler Type			5 foot core barrel		Measuring Point Elevation (ft-msl)	TBD	
Desc	ription	of Sam	ple Loca	tion				Northing Easting	TBD	
	S	AMPL	ES	1				1		
Ι.										
ŧ				lođe	ő	MATE				
	nber	Recovery (%)	۵	USCS Symbol	Graphic Log	MATE	ERIAL DESCRIPTION		Well Construction	on
Depth bgs)	Run Number	over	Sample ID	SCS	aph					
bg	Rur	Rec	San	SN	ē	fine sand with silt as above, flowing sand	ds			
-	10	100	N/A	SP	N/A					
121	-					-				
122	1					-				
						harder drilling 122-126 feet				
123						minor sandstone chunks stuck on augers	s and core barrel			
- 1	1					-				
124						-		_		
						- 605)5	_		
125						blind drilled 125 to 130 feet, no recovery	/			
126						-				
-						-		-		
127						-		-		
-						-		_		
128										
129						-				
-								-		
130						END OF BORING = 130 FEET BGS				
-						_		-		
131						 Well Completion 0.0-1.0 ft: cement for above-grade well pi 	protection	-		
132						1.0-107.5 ft: bentonite grout with cement 107.5-112.3 ft: 3/8" diameter bentonite pe	t pellets	-		
-]						C, threaded riser pipe			
133 _	1					 114.0-125.0 ft: Pre-Packed Well Screen (Note: Drillers overdrilled well to compens 	(Two 5-foot segments with 3/4' riser in	between segments)		
-							Sate for nowing sente	-		
134						-				
135	1					-		-		
-]									
136	1					-				
						-				
137						-				
138	1					-				
- 130						-				
139	-					-				
-						-				
140	1									
	_									

Basin Electric LRS Well Replacement PROJECT NAME: WELL ID: MW-53BR David CHECKED BY: PROJECT NO: 60727167 4/20/2024 INSTALLED BY: DATE INSTALLED: Buhl DEPTH BELOW OR ABOVE **ELEVATION** CASING AND SCREEN DETAILS (BENCHMARK: USGS) **GROUND SURFACE (FEET)** 0 TOP OF CASING TYPE OF RISER: PVC Schedule 40 PIPE SCHEDULE: N/A 0 PIPE JOINTS: **0** GROUND SURFACE SOLVENT USED: N/A Slotted SCREEN TYPE: 10 CEMENT SURFACE PLUG SCR. SLOT SIZE: 0.010 INCH 6 3/8 IN. FROM 0 TO 120 FT. IN. FROM TO FT. **RISER PIPE LENGTH** BOREHOLE DIAMETER GROUT/BACKFILL MATERIAL bentonite grout GROUT/BACKFILL METHOD tremie 8 SURF. CASING DIAMETER2IN. FROM0TO120FT.IN. FROMTOFT. 93 GROUT BENTONITE SEAL MATERIAL bentonite chips WELL DEVELOPMENT 98 BENTONITE SEAL DEVELOPMENT METHOD: Not developed at time of installation TIME DEVELOPING: HOURS 100 TOP OF SCREEN WATER REMOVED: GALLONS WATER ADDED: GALLONS SCREEN LENGTH FILTER PACK MATERIAL 20 WATER CLARITY BEFORE / AFTER DEVELOPMENT 10/20 silica sand 120 BOTTOM OF SCREEN CLARITY BEFORE: 120 HOLE BOTTOM COLOR BEFORE: CLARITY AFTER: NOTES: COLOR AFTER: Materials used in annulus: 14 bags of bentonite grout ODOR (IF PRESENT): • 11 bags of 10/20 silica sand • 5 bags of cement for surface plug WATER LEVEL SUMMARY 2 bags of bentonite chips SWE MEASUREMENT DATE TIME BEFORE DEVELOPING T/PVC T/PVC AFTER DEVELOPING: T/PVC OTHER OTHER T/PVC

WELL CONSTRUCTION DATA

PERMANENT, LEGIBLE WELL LABEL ADDED?

PROTECTIVE COVER AND LOCK INSTALLED? 🛛 🔽 YES 🥅 NO

Δ		D N		Project	Name:	Laramie River Station Well Replacement				
Bor	ing	Log		Client: Proiect	Number:	Basin Electric 60727167	Boring ID	:	M	W-53BR
Date(s)	-		7-19, 202		Logged	David Buhl Checked	Total Depth of	120	Dep	th to Water (bgs)
Drilled Drilling		-	Sonic		By Diameter of	By 6 3/8	Borehole (ft bgs) Ground Surface		4590.	106 8
Method Drill Rig		Pr	o Sonic		Borehole (in) Drilling	Cascade	Elevation (ft-msl) Groundwater		4484.	3
Type Driller's Nar	me		Ryan Mil	ller	Company Sampler	6 inch diameter, 2.5 foot long sample bags	Elevation (ft-msl) Measuring Point		4590.	3
Description of	of Samp	ole Loca	ition			berm road between two coal-ash settling ponds, approximately 500 feet west of the northeast	Elevation (ft-msl) Northing		86183.	
	-		/PLES	s	1	pond berm road.	Easting		30230.	4//
Depth (ft-bgs)	Run Number	Recovery (%)	Core Length (inches)	Core Depth (ft bgs)	USCS Symbol	MATERIAL DESCRIPTION		Well C	Cons	truction
1 1 1 1 1 2 1 1 3 1 4 1 4	ogs via Hydrovac	ic excavation				No recovery through the first 10 feet bgs due to drilling into the hyd	rovac pothole	Concrete		Concrete
	Excavated to around 10 feet bgs via Hydrovac	No recovery from hydrovac excavation	N/A	N/A	N/A			Bentonite Chips	PVC Casing	Bentonite Chips
	1	100	24* 30*	12	ML	10 YR 5/3 brown sandy silt, mottled, soft, moist, medium plasticity, ~20% fine sand		te Grout	Schedule 40	te Grout
	2	100	<u>30"</u> 30"	17	sw	10 YR 4/3 brown silty, gravelly sandstone, loose, moist, very fine - very coarse grai sand grains are high spericity, subrounded, well graded		Bentonite Grout		Bentontie Grout
		93	28"	22	CL	10 YR 6/2 light brownish gray sandy clay, medium stiff, moist, medium plasticity, ~2	20% fine sand			

Δ	EC	0	4	Project	Name:	Laramie River Station Well Replacement	[
	ring			Client:	Numbor	Basin Electric	Boring ID:		M	W-53BR
Date(s)		_		-	Number:	60727167	Total Depth of	120	Dep	th to Water (bgs)
Drilled Drilling		-	17-19, 202	24	By Diameter of	Bavid Buni By	Borehole (ft bgs) Ground Surface	120		106
Method Drill Rig			Sonic		Borehole (in) Drilling	6 3/8	Elevation (ft-msl) Groundwater		4590.	
Type Driller's Na	me	Pi	o Sonic		Company Sampler	Cascade	Elevation (ft-msl) Measuring Point		4484.	
<u> </u>			Ryan Mil	ller	Туре	6 inch diameter, 2.5 foot long sample bags I berm road between two coal-ash settling ponds, approximately 500 feet west of the northeast	Elevation (ft-msl)		4590. 586183.	
Description	of Sam					ettling pond berm road.	Easting		730230.	
		SA	MPLES	S						
Depth (ft-bgs)	Run Number	Recovery (%)	Core Length (inches)	Core Depth (ft bgs)	USCS Symbol	MATERIAL DESCRIPTION		Well C	Cons	truction
-					CL	Same as above with some gravel, mottled	-			
21		93				10 YR 5/3 brown clayey, gravelly sand, mottled loose, moist, very fine - very coarse	e, ~15% clay, ~20% gravel,			
			0.0"			high sphericity, subrounded, well graded.				
22	1		28"	22	sw	├ F				
23						- -				
	2	93				10 YR 6/3 pale brown sandy silt, mottled, medium stiff, medium - low plasticity, slig fine	htly moist, sand is very fine -			
24						-	-			
-			28"	24.5		10 YR 5/3 brown sandy silt, stiff, moist, low plasticity, ~20 fine sand, grayish white i	nodules (possibly gypsum)			
25						-	-			
26		93			ML	F 7	-			
27 _			28"	27			1			
						-	4			
28		100				-				
		100			СН	 10 YR 5/4 yellowish brown sandy clay, mottled, soft, high plasticity, moist, ~20 very 	r fine - fine sand		ing	
29 _			30"	29.5		10 YR 5/3 brown sandy silt, dry, soft - medium stiff, mottled, low plasticity, ~20% fir	ne sand	out	PVC Casing	out
30					ML	-	_	te Gr	PVC	te Gr
-						<u>-</u>	-	Bentonite Grout	iedule 40	Bentonite Grout
31 _		100				- 10 YR 4/3 brown silty sand, moist, loose, fine, ∼20% silt, high sphericity, subrounde	ed, poorly graded	Be		Be
-						10 YR 6/2 light brownish gray silty sand, slightly moist, very loose, very fine sand, -	~20% silt		Scl	
32	3		30"	32	-	<u> </u> -				
33	1					F -				
		93			SM]			
34	1					F	1			
-			28"	34.5	-	Ē				
35					ML	10 YR 3/4 dark yellowish brown sandy silt, slightly moist, stiff, low - medium plastic	ity, ~20% very fine - fine sand			
36		90				10 YR 5/3 brown silty sand, moist, dense, very fine - fine sand, between 10% - 20%	6 silt content			
]									
37			27"	37			1			
						F	-			
38 _		100			SM		1			
-	4		18"	38.5	-]			
39		93]			
40			28"	41		⊢ r	-			
	-			• • •	1		<u></u>		i .	

A-		244		Project	Name:	Laramie River Station Well Replacement			
A Bor	ring	Log		Client:	Numbor	Basin Electric	Boring	ID:	MW-53BR
Date(s)			7-19, 202		Number:	60727167 David Buhl Checked	Total Depth of	120	Depth to Water (bgs)
Drilled Drilling			Sonic		By Diameter of	By 6 3/8	Borehole (ft bgs) Ground Surface		106 4590.8
Method Drill Rig			o Sonic		Borehole (in) Drilling	Cascade	Elevation (ft-msl) Groundwater		4484.3
Type Driller's Nan	me		Ryan Mil	ller	Company Sampler	6 inch diameter, 2.5 foot long sample bags	Elevation (ft-msl) Measuring Point		4590.3
Description o	of Samp	ole Loca	ation		Type On an elevated	berm road between two coal-ash settling ponds, approximately 500 feet west of the northeast ttiling pond berm road.	Elevation (ft-msl) Northing Easting		586183.818 730230.477
		SA	MPLES	s	I		Lasung		100200.411
Depth (ft-bgs)	Run Number	Recovery (%)	Core Length (inches)	Core Depth (ft bgs)	USCS Symbol	MATERIAL DESCRIPTION		/ell Constr	ruction
		93				Same as above		_	
41 _			28"	41	-	-		-	
								-	
42		90							
43	4					F 		_	
			27"	43.5	-			-	
44						-			
45		80						-	
						-		_	
46			24"	46	-	Slightly moist, sand is very fine, otherwise same as above		_	
47		100	12"	47		-		-	
								-	
48		90						-	
49					SM	-		-	sing
			27"	49.5				irout	C Cas
50						-		Bentonite Grout	ledule 40 PVC Casing Bentonite Grout
51		87				- -		Sento	dule 4
	5					-		3	Sche
52	2		26"	52	-	becoming more silty		_	
								-	
53 _		93				+ - -		-	
54]	
			28"	54.5	-			-	
55 _						more sandy		-	
56		93						1	
						- -		-	
57			28"	57		10 YR 4/2 dark grayish brown sandy clay, moist, soft, high plasticity, between 30% -	40% fine sand	.]	
58								-	
]	6	97			СН]	
59 _									
60		93	29 28	59.5 62	SM	10 YR 6/3 pale brown silty sand, loose, moist - dry, very fine sand, ~20% silt and cla	ay, some gravelly zones	·	
	1		20	1 32	I				I I

Δ		٦ <i>N</i>			t Name:	Laramie River Station Well Replacement				
Bo	ring	Loc	,	Client:		Basin Electric	Boring ID:		M	W-53BR
Date(s)					t Number:	60727167	Total Depth of		Den	th to Water (bgs)
Drilled		April 1	17-19, 20	24	Logged By	David Buhl By	Borehole (ft bgs)	120	Deb	106 106
Drilling Method			Sonic		Diameter of Borehole (in)	6 3/8	Ground Surface Elevation (ft-msl)		4590.	8
Drill Rig Type		Pr	o Sonic		Drilling Company	Cascade	Groundwater Elevation (ft-msl)		4484.	3
Driller's Na	ime		Ryan Mi	ller	Sampler Type	6 inch diameter, 2.5 foot long sample bags	Measuring Point Elevation (ft-msl)		4590.	3
Description	of Sam	ple Loca	ation		On an elevated	berm road between two coal-ash settling ponds, approximately 500 feet west of the northeast ttling pond berm road.	Northing		586183.	
	<u> </u>	SA	MPLE	s			Easting		730230.	477
Depth (ft-bgs)	Run Number	Recovery (%)	Core Length (inches)	Core Depth (ft bgs)	USCS Symbol	MATERIAL DESCRIPTION		Well (Cons	truction
-						10 YR 6/3 pale brown silty sand, loose, moist - dry, very fine sand, ~20% silt and c	lay, some gravelly zones			
61		93			SM	-	-			
-	1					- 10 YR 6/3 pale brown sandy silt, medium stiff - very stiff, dry, low plasticity, very fir	e sand ~40% sand some			
62			28"	62	-	gravel				
						-				
63	1	87				<u>L</u> -	-			
64	6					-	-			
-			26"	64.5		-	-			
65					ML	-	_			
-						-	_			
66		83				-	_			
-						-	-			
67			25"	67	-		-			
-]	100				-				
68		100		07-7		10 RY 6/3 pale brown silty sand, loose, dry, slightly moist, fine sand, ~20% - 30% s	silt, hard while drilling			
69	1		18"	68.5	1	<u>L</u> -	-		ing	
	1					-	-	out	Cas	out
70		93						Bentonite Grout	edule 40 PVC Casing	Bentonite Grout
-						-		Itoni	le 40	Itonit
71			28"	71	4	-	3	Ben	Inbər	Ben
-	7					L 	-		Sch	
72		07				• 	-1			
-		87					-			
73]			
	1		26"	73.5	1	-	1			
74					SM		1			
75]	90					1			
						-				
76			27"	76	1]			
-						Same as above	1			
77	1	100				- -	-			
-	1		18"	77.5	-	-	1			
78	8									
		100				- -]			
79							E			
80 -	1		30"	80		-	-			
	-		50	30		<u> </u>				

AE Bori	ng			Client:		Basin Electric	Boring ID:		IV	W-53BR
ite(s)		Log		Project	Number:	60727167				
illed		April 1	7-19, 202	24	Logged By	David Buhl Checked By	Total Depth of Borehole (ft bgs)	120	Dep	oth to Water (bgs 106
illing ethod		5	Sonic		Diameter of Borehole (in)	6 3/8	Ground Surface Elevation (ft-msl)		4590	
ill Rig		Pro	o Sonic		Drilling	Cascade	Groundwater		4484	.3
riller's Nam	ne		Ryan Mil	ller	Company Sampler	6 inch diameter, 2.5 foot long sample bags	Elevation (ft-msl) Measuring Point		4590	.3
escription of	6 C				Type On an elevated	berm road between two coal-ash settling ponds, approximately 500 feet west of the northeast	Elevation (ft-msl) Northing	5	586183	
scription of	r Samp				corner of the se	ttling pond berm road.	Easting	7	730230	.477
(ft-bgs)	Run Number		Core Length dinches)	Core Depth (ft bgs)	USCS Symbol	MATERIAL DESCRIPTION		Well C	Cons	truction
						-				
81		100				-	-			
82			24"	82		-				
	Ī				1	-				
83						-]			
1	9	100				-	-			
84										
4	-		30"	84.5	-	-	1			
85						-	4			
-		00				-	-	ŧ		ŧ
86		93				- —	-	Grot		Grot
						-		nite		nite
87			28"	87		moist, otherwise same as above		Bentonite Grout	1	Bentonite Grout
							-	B	1	œ
88		87				-				
89 -						-	-		ing	
			26"	89.5		-	-		edule 40 PVC Casing	
90						-			PVC	
					SM	-			e 40	
91		87]		edul	
-						-			Sche	
92	10		26"	92			<u> </u>		1	
4						1	-		1	
93 _						-	4		-	
-1		93				-	1			
94							1	ß		"
	ŀ		28"	94.5		-]	Chip		Chips
95						Some gravel, otherwise same as above	-	Seal - Bentonite Chips		Seal - Bentonite Chips
<u>_</u>		77				-	1	entor	1	antor
96						:	4	Ë-B	1	- Be
97			23"	97		-	1	Sea	1	Sea
~ 					1	becoming very moist - wet, otherwise same as above	1		1	
98		100					1			
-	11		18"	98.5		-	1	;	1	-)
99						-]	Pack		Pack
3		100]	Filter Pack - Sand		Filter Pack - Sand
100			30"	101			<u> </u>	ш]	ι. Έ

				Project	Name:	Laramie River Station Well Replacement			
A	CO ring	M		Client:	. Humo.	Basin Electric	Boring ID:		MW-53BR
B0	iiig	LUG	/	Project	Number:	60727167			
Date(s) Drilled		April 1	7-19, 202	24	Logged By	David Buhl By	Total Depth of Borehole (ft bgs)	120	Depth to Water (bgs) 106
Drilling Method			Sonic		Diameter of Borehole (in)	6 3/8	Ground Surface Elevation (ft-msl)		4590.8
Drill Rig		Pr	o Sonic		Drilling	Cascade	Groundwater		4484.3
Type Driller's Na	me		Ryan Mi	ller	Company Sampler	6 inch diameter, 2.5 foot long sample bags	Elevation (ft-msl) Measuring Point		4590.3
Description	of Com				Type On an elevated	berm road between two coal-ash settling ponds, approximately 500 feet west of the northeast	Elevation (ft-msl) Northing		586183.818
Description	or Sam			-	corner of the se	ettling pond berm road.	Easting		730230.477
Depth (ft-bgs)	Run Number	Recovery (%)	Core Length T (inches) T	Core Depth (ft bgs)	USCS Symbol	MATERIAL DESCRIPTION		Well (Construction
- 1		100				moist, otherwise same as above	_		
101 _	-		30"	101	-	-	_		
	11	100							
-			30"	103.5			-		
104									
105	•	100	0.01	100		some cobbles			
106			30"	106		wet, same as above, probable water level	_		
107		100	12"	107		-	-		
107 108 109		100	30"	109.5				Sand	Sand
110					SM	-		- Ick	tted .
- - 111 _		100				- 		Filter Pack - Sand	0.010 Slotted Screen Filter Pack - Sand
112	12		30"	112		-			
113 113 114		100							
- 1			30"	114.5	-	F F	1		
115		100				very fine - medium sand grains but larger grain size overall, otherwise same as abo	ve		
116							E		
447 -			30"	117		F	1		
117			30"	117	1				
118		0	0"	118		- No Recovery			
110 _	13	100				very fine - fine sand grains but smaller grain size overall, otherwise same as above			
-						-			
120			24"	120	L				

Descript Log Project Number: 0057022 Description Description <thdescription< th=""> <thdescription< th=""></thdescription<></thdescription<>	Δ	-0	ON	1	Project	Name:	Basin Electric Laramie River Station		_			
Sector Operation O	Boi	ring	Log		Client: Project	Number	D:					
Diffinition (and in the line August (body) Difficient of (body) Difficient of (body) <thdifficient of<br="">(body) Difficient of (body)</thdifficient>		-			Troject			Total Depth of	05.6	Depth to		
Introd Tube data transmission Becaute rise Decomposition Decompo									25 ft			
The information of the second second second process and process of the second second process of the second secon	Method				ger	Borehole (in)	8 1/4 inch OD	Elevation (ft-msl)				
Interview Pre- leverage Interview	Туре							Elevation (ft-msl)		TBD		
Interpretation of tample Lesson Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Construction Image: Constrution Image: Construction								Elevation (ft-msl)		TBD		
ugo ugo <thugo< th=""> <thugo< th=""> <thugo< th=""></thugo<></thugo<></thugo<>	Desc	cription	of Sam	ole Loca	tion					TBD		
1 2 5 NA 2 0 25 1 0 25 1 0 25 1 0 25 1 0 25 1 0 25 1 0 25 1 1 0 1 0 2 1 0 1 1 0 1 1 0 1 1 0 1 1 0 1	Depth (ft-bgs)				USCS Symbol	Blow Counts	MATERIAL DESCRIPTION		Well Co	onstruction		
2 1 40 N/A 0. 2 1 2	-					-	0.5-1 ft: dark brown top soil, silty sand with clay, rootlets, moist. 1-5 ft: clay with silt, brown, moist, slightly sticky at 2.5 ft, wet 2 to 2.5 ft, moderat	te plasticity, very soft,				
1 40 N/A CL 2	-					2	-	-	-			
3 -	- 1					2		_	-			
4 1 1 2 1 6 1 1 1 1 1 7 2 55 NA 5W 0 1 7 2 55 NA 5W 0 1 7 2 55 NA 5W 0 1 10 7 16 16 10 10 10 10 11 1 1 10 1	3	1	40	N/A	CL	F	-	-				
5 Image: Second Sec	- 1	1							2	-	-	
5 Image: Second Sec	4	1				[_	_]			
0 1 1 poor recovery 7 2 55 NA SW 0 7 2 55 NA SW 0 10 16 7 16 16 10 16 16 16 16 11 20 16 125 ft. frie sand grading into medium and coarse sand, poorly sorted, brown, sand appears granite, subrounded grains bentonite chips 13 3 50 NA SW 22 12.5 ft. frie sand grading into medium and coarse sand, poorly sorted, brown, sand appears granite, subrounded grains 1020 allice 14 10 NA SW 22 12.5 ft. frie sand grading into medium and coarse sand, poorly sorted, brown, sand appears granite, subrounded grains 1020 allice 15 29 few pubbles at 14.5 ft (1 inch diamteris) four pables/sand mix, very poorly sorted, rounded to subrounded grains 1020 allice 16 16 33 16-17 ft. coor change to reddish, fine all matrix 17.18 ft. light tan ma						2	-	-				
7 2 95 NA SW 0 Image: start s	-					1	poor recovery	-				
3 2 55 NA SW 0 - 9 - - - - - - 10 - - - - - - 10 - - - - - - - 11 - - - - - - - - 12 -	°					7	wet, coarse sand, subrounded, poorly sorted, sand is medium to fine at 7 ft for 6 then couarse sand with few gravels/pebbles, no silt/clay	6 inches				
8 9 1	7 _						-	-				
9 -	8_	2	55	N/A	SW	9	-	-				
10 10 16 16 16 17 12.5 ft; hie sand grading into medium and coarse sand, poorly sorted, brown, sand appears granitic, subrounded 12.5 ft; hie sand grading into medium and coarse sand, poorly sorted, brown, sand appears granitic, subrounded 12.5 ft; hie sand grading into medium and coarse sand, poorly sorted, rounded to subrounded grains 15.16 ft; coars change to reddish, fine silt matrix 16.17 ft; color change to reddish, fine silt matrix 10/20 silica sandpack 10						7	-	-				
11 1						16	-	-				
12 3 50 N/A SW 22 12.5 ft: fnie sand grading into medium and coarse sand, poorly sorted, brown, sand appears granitic, subrounded, saturated 13 14 28 12.5 ft: fnie sand grading into medium and coarse sand, poorly sorted, brown, sand appears granitic, subrounded, saturated 14 29 few pebbles at 14.5 ft (1 inch diamters) fowing sand 15 13 13 15.16 ft: coarse sand with 10% gravel, wet, poorly sorted, rounded to subrounded grains 10/20 silica sandpack 16 33 16-17 ft: color change to reddish, fine silt matrix 10/20 silica sandpack 10/20 silica sandpack 17 4 100 N/A SW 74 17-18 ft: light tan matrix, pebbles/sand mix, very poorly sorted, wet, rounded pebbles (1 inch diamter), fine to medium sand 00 ng gg 19 19 10 N/A SW 74 17-18 ft: light tan matrix, pebbles/sand mix, very poorly sorted, wet, rounded pebbles (1 inch diamter), fine to medium sand 00 ng gg	-					17	no recovery 10-12.5 ft, water on acetate sleeve is micaceous	-	bentonite chips			
12 3 50 N/A SW 22 12.5 ft: fnie sand grading into medium and coarse sand, poorly sorted, brown, sand appears granitic, subrounded. saturated 13 14 26 12.5 ft: fnie sand grading into medium and coarse sand, poorly sorted, brown, sand appears granitic, subrounded. Saturated 14 29 few pebbles at 14.5 ft (1 inch diamters) fowing sand 15 13 13 15-16 ft: coarse sand with 10% gravel, wet, poorly sorted, rounded to subrounded grains 16 33 16-17 ft: color change to reddish, fine silt matrix 10/20 silica sandpack 17 4 100 N/A SW 74 86 86 17-18 ft: light tan matrix, pebbles/sand mix, very poorly sorted, wet, rounded pebbles (1 inch diamter), fine to medium sand 00 ftm edium sand	11 _						-	-	-			
13 13 12.5 ft: Thie sand grading into medium and coarse sand, poorly sorted, brown, sand appears granitic, subrounded, saturated 14 26 29 14 29 few pebbles at 14.5 ft (1 inch diamters) 15 few pebbles at 14.5 ft (1 inch diamters) 16 13 13 16 13 16-17 ft: color change to reddish, fine silt matrix 17 33 16-17 ft: color change to reddish, fine silt matrix 18 4 100 N/A 18 86 17-18 ft: light tan matrix, pebbles/sand mix, very poorly sorted, wet, rounded pebbles (1 inch diamter), fine to medium sand 00 rot reduce to reddish, fine to medium sand	12					20	- - -	-				
14 26 14 29 15	13	3	50	N/A	sw	22	12.5 ft: fnie sand grading into medium and coarse sand, poorly sorted, brown, s saturated	and appears granitic, subrounde	d,			
15	-					26						
16 13 flowing sand 16 15-16 ft: coarse sand with 10% gravel, wet, poorly sorted, rounded to subrounded grains 17 33 17 4 4 100 N/A SW 74 17-18 ft: light tan matrix, pebbles/sand mix, very poorly sorted, wet, rounded pebbles (1 inch diamter), fine to medium sand 18 86	-					29	few pebbles at 14.5 ft (1 inch diamters)					
17 1 4 100 N/A SW 74 16-17 ft: color change to reddish, fine silt matrix 10/20 silica sandpack 18 1 1 1 1 1 1 1 18 1 1 1 1 1 1 19 1 1 1 1 1 1	-					13	flowing sand 15-16 ft: coarse sand with 10% gravel, wet, poorly sorted, rounded to subrounde	ed grains				
18 4 100 N/A SW 74 17-18 ft: light tan matrix, pebbles/sand mix, very poorly sorted, wet, rounded pebbles (1 inch diamter), fine to medium sand 0 18 1 4 100 N/A SW 74 fine to medium sand 0 19 1 1 1 1 1 1 1 1 0<	-					33	16-17 ft: color change to reddish, fine silt matrix		10/20 silica sandpack			
18 18 19 19 19 19.25 ft 19.25 ft	17	4	100	N/A	sw	74	17-18 ft: light tan matrix, pebbles/sand mix, very poorly sorted, wet, rounded pel fine to medium sand	bbles (1 inch diamter),				
19 1	18					86).01 slot s		
	19					96	no recovery 19-25 ft	-		creen		
	20					-			-			

				Project	t Name:	Basin Electric Lara	amie River Station			
A	ECC ring	ON		Client:						
Bo	ring	Log	9	Project Number: 60577052						
					T				0E (4	 _
Date(s)		02	-Apr-19		Logged	J. Hurshman Cr	hecked	Total Depth of	25 ft	Depth to
Drilled Drilling					By Diameter of	Ву		Borehole (ft) Ground Surface		Water (bgs)
Method Drill Rig			Stem Au	iger	Borehole (in) Drilling		inch OD	Elevation (ft-msl) Groundwater	TE	
Type Driller's N			CME		Company	Inberg	Miller	Elevation (ft-msl)	TE	
	ry/Jose		Sampler Type	ſ		2 inch acetate sleeve		Measuring Point Elevation (ft-msl)	TE	D
Des	cription	of Sam	ple Loca	tion				Northing Easting	TE	D
	S	AMPL	.ES	0	6					
	Ē	(%		Symbol	unts					
c îs	qui	ery (eID	s s	ပိ	MA	ATERIAL DESCRIPTION		Well Con	struction
Depth (ft-bgs)	Run Number	Recovery (%)	Sample ID	nscs	Blow Counts					
≙≞	ž	Re	Se	>		no recovery, refusal with 2 inch push	probe			
					100	- 		-		
21	1							-		
- 1	1									
22_	1									
- 1	5	0	N/A	SW					10/20 silica sandpack	
23										
- 1	1									
24						<u> </u>		_		
						<u> </u>		_		
25						Total Depth = 25 feet				<u> </u>
						Well completion:		_		
26						0-14.5: schedule 40 riser 14.5-24.5 ft: schedule 40 0.01 slot sc	creen			
						2-12.5 ft: medium bentonite chips 12.5-24.5 ft: 10/20 silica sandpack		-		
27						<u> </u>				
						sandpack.	ole in middle section of bentonite chips, min	imum of 1 ft bentonite on top of		
28								_		
-										
29						-		-		
20	_					- •		-		
30	1									
31	-									
	1									
32	1									
	1									
33	1					-				
	1									
34	-					- -				
35										
36	-							-		
	1							3		
37	1							3		
	1							3		
38]		
]	1							3		
39]		
						_				
40	-							_		
	-								·	

Δ	EC	0	A	Project	Name:	Basin Electric Laramie River Station			
Bo	oring	Lo	7	Client:	Number	Basin Electric Laramie River Station	Boring ID):	MW-55B
Date(s)				Project	Number:	60577052 J. Hurshman Checked	Total Depth of		Depth to
Drilled		03	3-Apr-19		Ву	By	Borehole (ft)		Water (bgs)
Drilling Method		Hollow	Stem Au	iger	Diameter of Borehole (in)	8 1/4 inch OD	Ground Surface Elevation (ft-msl)		TBD
Drill Rig Type			CME		Drilling Company	Inberg Miller	Groundwater Elevation (ft-msl)		TBD
Driller's N	lame Barry		Sample Type	r		5 foot core barrel	Measuring Point Elevation (ft-msl)		TBD
							Northing		твр
De		of Sam	ple Loca	1	1		Easting		
Depth (ft-bgs)	Run Number	<u> </u>	Blow Counts	USCS Symbol	PID (mpm)	MATERIAL DESCRIPTION		Well C	Construction
	-		0		N/A	 0-5 ft: silty fine sand, very soft from 0-3 ft, rootlets 0-0.5 ft, dark broan 0-4 ft, ha content/less silt 4-5 ft, medium sand, light tan color, dry 	arder from 4-5 ft, increased sand	-	
1	-					-	_		
	-		1			<u>-</u>	-		
2_	1			-		-	 	1	
· ·	1	70	3			-	-	1	
3	-			-		-	-		
· ·	-		8			-			
4_	_			-					
5	-		8	SM		-	-		
3 .	-		6	3111		as above to 9 ft bgs.	-		
6	-		Ū			-	-		
	-		9			-	-		
7	-					-			
	2	75				-	-		
8_	-		9				-		
	-		24			-	-		
9_	_		24			- 9-10 ft: sandy gravel, dry poorly sorted, hard to drill, rounded gravels broken up	by drill rig sand is fine to medium	j	
· ·	-		38]	
10_	-			-		as above to 12.8 ft, sandy gravel with pebbles, broken by drilling, poorly sorted	- -		
· ·	-		23	GM		-			
11	1			-		-	-	grout	
	1		34			-	-		
12_	3	100				-		1	
13	1		88			12.8 - 13.5 ft: fine sand, tight, well sorted, dry, light tan, refusal at 13.5 ft with a	cetate sleeve.		sche
	-		100/6"]		-		schedule 80 riser
14	<u> </u>			SP		-	-		30 rise
	1	0	NA			-	-		а,
15	1					-	-		
	1		29			15-15.5 ft: fine sand with rounded gravel, poorly sorted, dry, light tan color 15.5-17 ft: fine sand, dry, well sorted, homogeneous, soft, refusal at 17 ft.	-		
16	4	100		_			-	1	
	1		100/11"			-	-	1	
17]	<u> </u>		-		overdrilled to 20 ft with solid stem auger, no recovery	-		
-						-	-		
18	1	~	N/A			-	-		
19	1	0					-		
19	1					-	1	1	
20	1					-	-	1	
L_*`-	_	•			•			•	

				Project	Name:	Basin Electric Laramie River Station			
AECOM Boring Log			1	Client:	Name.	Basin Electric Laramie River Station Boring ID			MW-55B
Bo	Boring Log			-	Number:	60577052		•	
				FTOJECI	Number.	00311032			
Date(s)		0.	3-Apr-19		Logged	J. Hurshman Checked	Total Depth of		Depth to
Drilled			57491 10		Ву	Ву	Borehole (ft)		Water (bgs)
Drilling Method		Hollow	/ Stem Au	uger	Diameter of Borehole (in)	8 1/4 inch OD	Ground Surface Elevation (ft-msl)	Т	BD
Drill Rig Type			CME		Drilling Company	Inberg Miller	Groundwater Elevation (ft-msl)	Т	BD
Driller's N			Sample Type	r	company	5 foot core barrel	Measuring Point	Т	BD
	Barry						Elevation (ft-msl) Northing	Т	BD
De			ple Loca	ition			Easting		
	S		LES	USCS Symbol	ŋ				
	a c	(%)	_	M m	۲ ۲				
gs) th	1	very	ole IC	ŝ	phic	MATERIAL DESCRIPTION		Well Cor	struction
Depth (ft-bgs)	Run Number	Recovery (%)	Sample ID	lso	Graphic Log				
	- "		0)	-	N/A	as above fine sand to 24 ft, well sorted, dry, homogeneous, light tan			
	_				N/A				
21							_		
I				SP		-	_		
22_	5	100		1		-	_		
							_		
23	_						_		
				1			_		
24_		+	<u> </u>	+	łF	hard layer at 24 ft, light tan to gray sandstone lens, fine grained, well sorted, hor	nogeneous, blind drilled to 25 ft		
	_	0		SS	-		-		
25	-				-	25-29 ft: fine sand, homogeneous, well sorted, hard packed, light tan, moist	-		
	-				-		-	grout	
26	_				-	_	_	5	
	_			SP	-		-		
27	6	100			-	hard lens at 27 ft for 2 inches	-		
	Ĩ	100					-		
28	_				-	_	-		
	_				-		-		
29	_				-	hard light tan/gray fine grained sandstone lens	-		
	_				-		-		
30	-					blind drilled to 35 ft (solid tip) harder sand/sandstone material	-		
	_						-		
31	1			SS/SP			-		
	1			33/38			-		
32_	7	0		1		_	-		
	_			1		<u>.</u>	-		
33	1						-	sc	
	1						-	schedule 80 riser	
34	1			1			-	le 80	
	_			1			-	riser	
35	1						-		
	1			1		fine sand as above, tight, hard to 38 ft, softer 38-40 ft, light tan, dry to moist, well	I sorted, homogeneous		
36	_			SP			-		
	_			1			-		
37	1				l F				
	8	100		1			-		
38	7			1					
	-						-		
39	-						-		
- 35	1			1			-		
40	1			1					
40	_	1	1	1		_		l	1

A	ΞC	0/ g L c	A	Projec Client:	t Name:			aramie River Station		Boring ID):		MW-55B
Bo	orinę	g Lo	g	Project Number: 60577052						,			
Date(s) Drilled			03-Apr-19		Logged By		J. Hurshman	Checked	_	Total Depth of Borehole (ft)			Depth to
Drilling		Hollo	w Stem A	uger	Diameter of		8	1/4 inch OD		Ground Surface		TBD	Water (bgs)
Method Drill Rig			CME		Borehole (in) Drilling		Ini	berg Miller		Elevation (ft-msl) Groundwater		TBD)
Type Driller's N			Sample Type	er	Company	5 foot	t core barrel			Elevation (ft-msl) Measuring Point Elevation (ft-msl)		TBD)
	Barry		mple Loca	ation						Elevation (itt-ms) Northing Easting		TBD)
De			PLES	ation		1				Easung			
Depth (ft-bgs)	and Mundar	_		USCS Symbol	Graphic Log			MATERIAL DESCRIP			Well C	ons	truction
	-		N/A	SP	N/A	as	s above, increasing moisture co edium to tan color	ontent, appears almost wet at 41	ft to 42 ft. hard	_	grout		
41]					-					5		
	9	100				-				-			
42_	- "	100				-							
43	-					-							
	4									-	hydrated bentonite seal		
44_	1					- 					Sentonite Seal		
	-									-			
45]		_					creased moisture, hard packed,	stiff				
						fe	w interbedded white burrow cas	sts 1-2 mm in diameter				sch	
46_	-					-						schedule 80 riser	
47	-					_				-		80 ris	
	10	100				-				-		èr	
48_	_					-							
	-					-							
49	-					_							
· ·	-					-							
50_	_		_			as	s above, core is wet						
51	1					-				-			
	-					_					10/20 silica		
52_	_					<u>_</u>				-			
	11	100				-				-			
53	4					-				-			
· ·	-					Ē							
54	1					F				-		s	
55	-					-				-		shedu	
55	-	1				so	s above, wet, ofter from 55-56.5 ft, then hard	packed,		-		le 80 (
56	-					sli	ightly cemented, few interbedde	ed white burrow casts in sand nea	ar 58-59 ft			0.01 sl	
	1					-				-		schedule 80 0.01 slot screen	
57	-					-				-		en	
· 1	12	100				-							
58	1					-				-			
59	1					-				-			
35	-					<u> </u>				-			
60	-									-			
-		-											

210 3				Project	Name:	Basin Electric Laramie River Station		
A=COM Boring Log				Client:		Basin Electric Laramie River Station	Boring ID	: MW-55B
Bor	ing	Log	7		Number:	60577052		
						J. Hurshman Cherked		
Date(s)		03	-Apr-19		Logged	Uncoked	Total Depth of	Depth to
Drilled Drilling		Hollow	Stem Au	ner	By Diameter of	By 8 1/4 inch OD	Borehole (ft) Ground Surface	Water (bgs)
Method Drill Rig				yei	Borehole (in) Drilling		Elevation (ft-msl) Groundwater	
Type Driller's Nar	ne		CME Sampler		Company	Inberg Miller 5 foot core barrel	Elevation (ft-msl) Measuring Point	TBD
			Туре				Elevation (ft-msl) Northing	TBD
Descr	ription	of Sam	ole Locat	ion	_		Easting	TBD
	S/	AMPL	.ES	ō				
	ē	(%)		USCS Symbol	Graphic Log			
ч (sg	qun	ery.	le ID	ŝ	hic	MATERIAL DESCRIPTION		Well Construction
Depth (ft-bgs)	Run Number	Recovery (%)	Sample ID	JSC	Brap			
	2	₩	N/A	SP	N/A	as shown wat hard packed aged 60.62.ft aster 62.65.ft ageuts drill	-	
			IN/A	or	N/A	as above, wet, hard packed sand 60-62 ft, softer 62-65 ft - easy to drill, brown color, few interbedded white burrow casts		
61						-		
						-		
62	13	100						
∥ 1						-	-	
63						-	-	
∥⊣						-	-	
64						-		10/00 - ""
						-	-	10/20 silica schedule 80 0.01 slot screen
65						as above, wet, flowing fine sand to 67.5 ft white burrow casts 69-70 ft - interbedded	-	ule 8
						white burrow casts 69-70 ft - interbedded	_	0 0.0
66							_	1 slot
								scree
67						_	-	en
-	14	100				-	-	
68						- —	-	
-						-	-	
69						-	-	
						-	-	
70						TD = 70 ft		
∥ –						Well Completion:	-	
71						Schedule 80 0.01 slot screen 50-70 ft 10/20 silica sand pack from 47.2-70 ft	-	
−						hydrated bentonite seal 41.8 - 47.2 ft schedule 80 riser 0-50 ft	-	
72						no centralizers used		
∥ –						-	F	
73						-		
┃ -						-		
74						-		
∥ –						-		
75						_	-	
−						-	4	
76						-		
−						-		
77						-	4	
−						-	4	
78						-	1	
_						-	1	
79						-	+	
_						-	+ +	
80					<u> </u>	-	<u> </u>	

A	EC	ON	1	Project Client:	Name:	Basin Electric Laramie River Station Basin Electric Laramie River Station	Boring I	D:	MW-56B
Boring Log Project Num						60577052			
Date(s) Drilled		02	-Apr-19		Logged By	J. Hurshman Checked	Total Depth of Borehole (ft)		Depth to Water (bgs)
Drilling		Hollow	Stem Aug	nor	By Diameter of	8 1/4 inch OD	Ground Surface		TBD
Method Drill Rig				901	Borehole (in) Drilling		Elevation (ft-msl) Groundwater		
Туре			CME		Company	Inberg Miller	Elevation (ft-msl)		TBD
Driller's Na Bi	me arry		Sampler Type			5 foot core barrel	Measuring Point Elevation (ft-msl)		TBD
							Northing		TBD
Desc	1		EC	1	<u>г</u> г		Easting		
Depth (ft-bgs)	Run Number	Recovery (%)	Blow Counts	USCS Symbol	PID (ppm)	MATERIAL DESCRIPTION		Well (Construction
_			3		N/A	silty fine sand, well sorted, dark brown 0-2 ft bgs, rootlets 0-1 ft, medium brown a omogeneous in appearance, moist, soft, crumbles easily	2-5 ft, no gravel, no clay	-	
1	2	90	3 5 4 4 3 2 2 4 5 5 4 4 5 4 4 19 76 94	SM	N/A	omogeneous in appearance, moist, soft, crumbles easily as above to 10 ft as above to 10 ft as above to 11 ft - softer, little recovery gravelly sand with pebbles/cobbles, dry, hard, broken pices of rock/non sandsto colors depending on rock/cobble type, poorly sorted	nein core, light tan to mottled		schedule 80 riser
14 15 15 16 17 17 18 18 19	4	100		SP		14.5 ft: light tan, fine sand, well sorted, homogeneous, softer 15-20 ft: light tan sand, very hard, lightly cemented/hard packed, slight moisture homogeneous, subournded to rounded grains	, well sorted, fine grained,		
-						-		-	
20				l	l t	hard sandstone lans at 20 ft.	-	J	

20.0				Project	Name:		Basin Electric Laramie River Station				
A	ring	DM		Client:			Basin Electric Laramie River Station	Boring ID):		MW-56B
BO	ring	Log	7	-	Number:		60577052				
					Γ						
Date(s)		02	-Apr-19		Logged		J. Hurshman Checked	Total Depth of			Depth to
Drilled					By Diamatas af		Ву	Borehole (ft)			Water (bgs)
Drilling Method		Hollow	Stem Au	ger	Diameter of Borehole (in)		8 1/4 inch OD	Ground Surface Elevation (ft-msl)		TBD	
Drill Rig Type			CME		Drilling Company		Inberg Miller	Groundwater Elevation (ft-msl)		TBD	
Driller's Na Bar	ame ry/Jose		Sampler Type			5	foot core barrel	Measuring Point Elevation (ft-msl)		TBD	
Dese	cription	of Samı	ole Locat	ion				Northing Easting		TBD	
		AMPL			I	1					
				Symbol	60						
	nbei	у (%	₽	Syı	ic L		MATERIAL DESCRIPTION				
Depth (ft-bgs)	Run Number	Recovery (%)	Sample ID	nscs	Graphic Log				well C	ons	truction
₫₩	Rui	Rec	Sar	ŝ	ษั		sandstone lens to 21 ft (solid tipon augers)				
- 1			N/A	SP	N/A	F		-			
21	1					F	21-25 ft: light tan very fine sand, homogeneous, well sorted, moist, no clay, no l	arger sand soft but tightly packed			
i -	1					F	agric tail for anno carra, nornogonoodo, weil outeu, moiat, nu oldy, nu i				
22	1					F					
i -	5	100				F		-			
23	1					F		-	1		
- 1	1					F		-			
24	1					L			1		
						L		-			
25						L		-			
						F	as above	-			
26						F		-			
						F		-			
27						F					
	6	50				F	hard sandstone lens from 27-28 ft				
28						F					
						F	aqs above fine sand, 1 cm thick sandstone lens at 28.7 ft, soft snd below				
29						F		-			
						F					
30						F		-	grout		
						F	as above, hard packed fine sand, light tan, moist, sandstone lenses at 30.2-30.	4 ft, and 30.9 to 31 ft			
31	1					F		-			
I _	1					F		-		sch	
32	1					L				ledule	
- 1	7	100				F		-	1	schedule 80 riser	
33	1 1	100				þ		-		iser	
- 1	1					þ		-			
34	1					F		-			
- 1	1					F		-			
35	1					F	as above, hard packed fine to very fine sand, medium brown, very poor to no cr	- 			
- 1	1					F	as asses, hard packed the to very the sand, medium brown, very poor to no o				
36	1					F		-			
	1					F		-			
37	1					F		-			
- 1	8	100				þ		-			
38 _	ĬĬ	.00				þ		-			
I -	1					F		-			
39	1					F		-	1		
- 1	1					F		-			
40						Ľ			1		
		_									

			_	Project	Name:		Basin Electric Lar	ramie River Station			
A	AECOM Boring Log		Client:				ramie River Station	Boring I) :	MW-56B	
ВО	oring	LO	3	Project	Number:		6057	77052			
							J. Hurshman				
Date(s)		02	2-Apr-19		Logged			Checked	Total Depth of		Depth to
Drilled Drilling		Hollow	Stem Au	aer	By Diameter of			By /4 inch OD	Borehole (ft) Ground Surface		Water (bgs)
Method Drill Rig			CME	0	Borehole (in) Drilling			erg Miller	Elevation (ft-msl) Groundwater		TBD
Type Driller's N			Sampler		Company	5 foot	t core barrel		Elevation (ft-msl) Measuring Point		TBD
	rry/Jose		Туре						Elevation (ft-msl) Northing		TBD
Des			ple Locat	tion					Easting		
	S	ampi I	ES	R	ß						
	ber	(%)	~	USCS Symbol	Graphic Log		N	MATERIAL DESCRIPTION			
ogs)	Run Number	Recovery (%)	Sample ID	SS	phid					Well C	onstruction
Depth (ft-bgs)	Run	Reco	Sam	nsı	Gra						
_	-		N/A	SP	N/A	as	s above, hard packed		-		
41	4					F			_	1	
- 1	1					F			-		
42	1						ightly darker brown 42-43 ft			1	
-	9	100				E			-]	
43 _						F			-		
44_	1					F			-	1	
44 <u>-</u>	-					so	oft sand 44-45 ft			-	
45	-					-			-	-	
	-						s above, hard packed 45-47.5 ft, s inor white streaks at 46 and 48 ft				
46	-					Ŀ				-	
-	-					-				-	
47	-					-			_	-	
-	10	100				-			-	-	
48]					E					
-						-			-	grout	
49	-					-			-	grout	
50	-					-			-	-	
	-					as	s above to 54.5 ft				
51	4					F			-		
- 1	1					F			-		
52_	-					F				1	
-	11	100				F			<u>-</u>		
53	1					F					
54	1					F					
<u> </u>	1			SS		F				bentonite seal	
55	-					F	most platey texture with depth				
	1			SP		- 54 - so	4.5 ft: hard sandstone, light gray, f blid stem to drill through sandston	fine grained, well sorted, well cemented, ne, wet beneath	nomogeneous		<i>a</i>
56	1			54			5-60 ft: well sorted, hard packed s	sandstone, wet at 55 ft, appears wet belo	w but very tightly packed, medium tai	1	sched
-	-					F "				1	schedule 80 riser
57]					E]	riser
	12	100				F				10/00 - "	
58	1					F				10/20 silica	
- 59	1					F				1	
35	1					F				1	
60	-					- No	ote: SS = sandstone		-]	
						_				_	

				Project	Name:	Basin Electric Lara	mie River Station			
A	A=COM		Client:			•	MW-56			
Bor	ing	Log	1		Number:	60577				
-				Project	Number:	60577	032			
Date(s)		02	-Apr-19		Logged	J. Hurshman Ch	hecked	Total Depth of		Depth to
Drilled		02	-Api-19		Ву	Ву	/	Borehole (ft)		Water (bgs)
Drilling Method		Hollow	Stem Au	ger	Diameter of Borehole (in)	8 1/4 ii	inch OD	Ground Surface Elevation (ft-msl)		TBD
Drill Rig			CME		Drilling	Inberg	Miller	Groundwater Elevation (ft-msl)		TBD
Type Driller's Nan			Sampler		Company	5 foot core barrel		Measuring Point		TBD
	arry		Туре					Elevation (ft-msl) Northing		TBD
Descr	ription o	of Samp	ole Locat	tion				Easting		IBD
	SA	MPL	ES	lo						
	Ŀ	(%)		USCS Symbol	Graphic Log					
Le Se	quin	ery (eID	ŝ	hic	MA	ATERIAL DESCRIPTION		Well C	onstruction
Depth (ft-bgs)	Run Number	Recovery (%)	Sample ID	SC	rap					
₫€	ž	å	ŝ		ن	sand as above, saturated, hard packe	ed,			
			N/A	SP	N/A	_		-		
61						-		-		
_								-		
62	13	100						-		
_	.5					-		-		
63						dark brown color change at 62.5 ft for	r 3 inches, softer than above 63.5-65 ft,	-		
_						slightly mottled light tan to dark brown	n color in this zone,	-		
64								-		
						-		-		
65 _						trace red granite gravel in bottom of c		-		
						65-70 ft: as above, medium tan sand,	, fine grained, wet, well sorted	-		
66								-		
_						-		-		
67						-		_		
_	14	100				-		_		
68								_		
						-		-		
69						-		-		
						-		-		
70						-		-		
-						as above to 72 ft		-		
71						-				
II ^{···} −						-		-		
72						-				
						72-74 ft: fine sand with interbedded cl	hips of brown/reddish almost chert, angular	r framents, poorly sorted		
73	15	100				-			10/20 silica	
						-		-		
74						-		-		sche
						74-75 ft: fine sand as above, wet, has	rd packed	-		schedule 80 0.01 slot screen
						-		-		30 0.4
75						75-79 ft: fine grained hard packed sar interbedded white burrow casts mainly	nd, very poorly to no cementation, well sort	ed, tan to light brown,		01 slo
							y 10.0 to 11.0 tt	-		t scn
76						-		-		een
∥ ┤		100				-		-		
77	16	100				-		-		
∥ –						-		-		
78						-				
−						-		-		
79						TD = 79 ft				
−						Well completion: schedule 80 0.01 slot screen 59-79 ft	t, schedule 80 riser 0-59 ft,	-		
80						10/20 silica sandpack 57-79 ft, bentor	nite seal 52-57 ft, grout 0-52 ft			

Appendix B Aquifer Test Procedures, Data and Analysis

page lot!

PUMPING TEST DATA FORM

Well ID	MW-33B	Personnel Hurshman	
Location	Busin Electric LRS	Static Water Level	68-8-62.88
Type of Well	Montoring well	Extraction Well Distance	
Test Date	8/22/16	Total Casing Depth	86,90
Measuring Point Elevation	n Toc - TBD	Borehole Diameter	6 inch
Type of Test	Shy Test	Casing Diameter	Zinch
Step Number		Screened Interval	
Data logger Test Run No.	1	Sand Pack Interval	
Pumping Rate		Lithology Tested	
Test Start Time		Test End Time	

Slug = 6 + + x linch

Time	Elapsed Time (min)	Water Depth (ft)	Time	Elapsed Time (min)	Water Depth (ft)
1321	Ø	69.80			
1322	Stat Jest	NM			
1323	Slug Th	NM			
1325	4 min	69.77			
1336	15 min	65 88			
1337	16 min	Slug out			
1339	18 min	70.09			
1345	Z4 mm	69.91			
1349	28 m.n	65.50			
1356	35 min	69.89			
1358	37 mm	Stopped test			

See transducer daite for details on Test.

PUMPING TEST DATA FORM	(Slug Test)
------------------------	-------------

Well ID	_	W-35B	_ Personnel		
Location	BAST	in Electric 17	_ Static Wat	er Level	66.43
Type of Well		iterry well	_ Extraction	Well Distance	
Test Date	81	22/14	Total Casi	ng Depth	88 31
Measuring Poir	t Elevation	C-TBD	Borehole I	Diameter	6 meh
Type of Test		Slug Test	Casing Dia	ameter	Z inch
Step Number		NA	Screened I	nterval	
Data logger Tes	t Run No.	L	Sand Pack	Interval	
Pumping Rate	_	·*	Lithology '		
Test Start Time		035	Test End T		1126
root Start Third		, = 6.14 x	linch	inne	1100
Time	Elapsed Time (min)	Water Depth (ft)	Time	Elapsed Time (min)	Water Depth (ft)
1054	Ø	66.42			
1055	Start Slug Test	NM			
1102	Frin	66.41			
1 () (16 mm	66.42			
1112	17 min	removed sky			
1114	19 min	66.59			
1122	27 min .	66 43			
1124	29 m.m.	66 43			
1126	31 m.n	stpped Test.			
			1	tails	

Page lof1

Well ID	MW-373	Personnel Hushmun	
Location	Basin Electric LES	_ Static Water Level	62.32
Type of Well	hear, tory well	_ Extraction Well Distance	
Test Date	×119116	Total Casing Depth	78.53
Measuring Point Elevatio	n TOC - TBD	Borehole Diameter	6 mm
Type of Test	Slug Test	Casing Diameter	Z inch
Step Number		Screened Interval	
Data logger Test Run No.	l	Sand Pack Interval	
Pumping Rate	NX (slug)	Lithology Tested	
Test Start Time	118	Test End Time	1143
	Slug = 6 lt x 1	mch	

Time	Elapsed Time (min)	Water Depth (ft)	Time	Elapsed Time (min)	Water Depth (ft)
1117	Ø	62.36			
1118	Stat slus	Test -			
1123	5 min	6235			
1128	10 mm	62.36			
1129	Slug aut	NM			
1139	ZI min	62.37			
1141	23 min	62.37			
1143	Stopped slu	Test			

see transducer dute for duta. 1.

page lot 1

PUMPING TEST DATA FORM (Slug Tast)

Well ID	MW-38B	Personnel Hurshman	
Location	BUSIN Electric LAS	Static Water Level	60.03
Type of Well	hundong will	Extraction Well Distance	×
Test Date	SIZZIC	Total Casing Depth	77.75
Measuring Point Elevation	786 - TBD	Borehole Diameter	6 meh
Type of Test	Slag Test	Casing Diameter	2 inch
Step Number		Screened Interval	
Data logger Test Run No.	1	Sand Pack Interval	
Pumping Rate		Lithology Tested	
Test Start Time		Test End Time	
\$	Huy = 6 + x 1	inch	
E1. 1 T	1 111		

Time	Elapsed Time (min)	Water Depth (ft)	Time	e	Elapsed Time (min)	Water Depth (ft)
1437	Ø	60.08				
1698	í m.n	Slug n well				
1625	2 min	59.76				
1644	7 m.n	5997				
1651	14 mon	60.05				
1653	16 min	60.07				
1659	22 min	60.07 - tru	-sduer	nay	have buyed	shahth
1700	23 min	Slug out				_0 /
1703	26 min	60.40				
1706	29 m.h	60.23				
1709	32 min	60.15				
1712	35 min	60.12				
1715	38 m.h	60.11				
1719	42 mm	60.10				
1722	45 m ~	60,09				
1723	46 m n	Stopped Test.				

SEC Transdure de datails.

Page lot 1

PUMPING TEST DATA FORM (Stug TEST)

Well ID	hw-39B	Personnel Hurshum, AL	redt
Location	Busin Electric LRS	Static Water Level	84.67
Type of Well	Mun.tory Well	Extraction Well Distance	
Test Date	8/28/16	Total Casing Depth	110.71
Measuring Point Elevation	TO C-TRD	Borehole Diameter	6 moh
Type of Test	Slug Test	Casing Diameter	Zinch
Step Number		Screened Interval	
Data logger Test Run No.	1	Sand Pack Interval	
Pumping Rate		Lithology Tested	
Test Start Time		Test End Time	
	Sug = 6 dt x IN	14	

Elapsed Time Water Depth Elapsed Time Water Depth Time Time (min) (ft) (min) (ft) Ø 81,69 0737 6738 Stat Fest styin 1 ann 6739 2 min 80.78 0743 6 min 31.54 81.67 0747 10 min 0751 81.69 15 min 0752 15 mil slug out 0754 17 min 82.42 0758 21 min 81.75 25 mil \$1.72 0802 0 804 27 mm \$1.71 0813 36 mm 51.71 08:5 56 mm Stopped Trut.

See transducer data for details.

Location $E_{acsive Errefue Uds}$ Static Water Level 44.37 Type of Well Max for a_{ac} for U Extraction Well Distance $$ Test Date $\P vn /\ell$ Total Casing Depth 70.07 Measuring Point Elevation $TDC = \tau \tau \sigma D$ Borehole Diameter $6 h A$ Type of Test $Slas_{acs} + r s^{acs}$ Casing Diameter $2 i \gamma A$ Step Number $$ Screened Interval Data logger Test Run No. 1 Sand Pack Interval Data logger Test Run No. 1 Sand Pack Interval Pumping Rate $$ $1 M ch$ Pumping Rate $$ $1 m ch$ $1 m ch$ $0 \eta \circ 7$ $51 m s - c H s + 1 M ch$ Time Elapsed Time 0544 $1 M ch$ $1 m ch$ $0 \eta \circ 7$ 0543 $0^{-} 4 + c S + s g ra m m H = 652$ $0 \pi m + 46.35$ $0 \pi + 46.35$	W	ell ID	<u> </u>	~w-42B	_ Personnel	Herstine	14
Test Date $\P1 \circ 1/L$ Total Casing Depth $70 \circ 7$ Measuring Point Elevation $T0 \leftarrow -\tau \circ D$ Borehole Diameter $6 h h h$ Type of Test $50 \leq 4 - 2^{\circ}$ Casing Diameter $2 - in k$ Step Number $$ Screened IntervalData logger Test Run No.1Sand Pack IntervalPumping Rate $$ Lithology TestedTest Start Time 0544 Test End Time 0543 046.34 Time 0907 0543 946.34 TimeUater Depth (ft) 0543 946.34 Screened Time (min)Water Depth (ft) 0543 946.34 Screened Time (min)Water Depth (ft) 0543 946.34 Screened Time (min)Water Depth (ft) 05473 946.34 Screened Time (min)Water Depth (ft) 05473 946.34 Screened Time (min)Water Depth (ft) 05473 946.34 Screened Time (min)Screened Time (min) 05473 946.34 Screened Time (ft)Screened Time (min) 0559 $16 min$ ($16 min$ 46.56 0707 5109 45.56 0707 5109 45.56 0707 5109 45.56 0707 5109 45.56 0707 5109 45.56 0707 5109 45.56 0707 5109 45.56 0707 5109 45.56 0707 5109 45.56 <td>Lo</td> <td>cation</td> <td>Br</td> <td>sin Electric LR</td> <td>Static Wat</td> <td>ter Level</td> <td>46.37</td>	Lo	cation	Br	sin Electric LR	Static Wat	ter Level	46.37
Measuring Point Elevation $TPC - TGD$ Borchole Diameter 6 M/h Type of Test $Slus + sot$ Casing Diameter 2 in/h Step Number	Ty	pe of Well	mon.	tory well	_ Extraction	Well Distance	
Measuring Point Elevation $TOC - TOD$ Borehole Diameter6 M/hType of TestSlug + x ctCasing Diameter2 in/hStep NumberScreened IntervalData logger Test Run No.ISand Pack IntervalPumping RateLithology TestedTest Start Time $OYO7$ Sites = C Lt w 1 MchTimeElapsed Time(nin)(ft) 05443 46.34 0554 16 m/h 0557 Slug out 0557 Slug out 0557 Slug out 0707 0557 Slug out 0557 Slug out 0707 0557 0756 13 min 46.35 0756 13 min 46.36 0707 0756 14 min 46.36 0707 0756 16 min 46.36 0707 5107 out 46.36 0707 5107 out 0707 <td< td=""><td>Te</td><td>st Date</td><td>8</td><td>lizhe</td><td>Total Casi</td><td>ng Depth</td><td>70.07</td></td<>	Te	st Date	8	lizhe	Total Casi	ng Depth	70.07
Type of Test Susc + cd Casing Diameter Z inch Step Number	Me	asuring Poi	int Elevation	DC-TJD			6 inch
Step Number Screened Interval Data logger Test Run No. I Sand Pack Interval Pumping Rate Lithology Tested Test Start Time 0844 Test End Time 0907 Stage C L4 x 1 Mch Time Elapsed Time (min) 046.3 L 0843 946.3 L Time Elapsed Time (min) Water Depth (ft) 0844 staft test, skg in yell. 6552 $9mn$ 46.3 L 6557 0566 $13mn$ 46.3 L 6557 $5lug out$ $0m$ 6551 6904 $21mn$ 46.3 L 0557 $5lug out$ $0m$ 46.5 L 6904 $21mn$ 46.5 L 6904 $21mn$ 46.5 L 6907 $5bop$ 45.5 6907 45.5 46.5 46.5 46.5 46.5 46.5 46.5 46.5 46.5 46.5 46.5 4	Ty	pe of Test	6	blug to st	Casing Di	ameter	
Data logger Test Run No. 1 Sand Pack Interval Pumping Rate		-	_		-		
Pumping Rate Lithology Tested Test Start Time 0844 Test End Time 0907 . $5145 = 6.44 \times 1 Mach$ Time Elapsed Time (min) Water Depth (min) (ft) 0843 0 46.360844 start test, stig in will. 0552 $7 Min$ 46.350156 $13 mn$ 46.360857 $5log out$ $UM0857$ $16 min$ 46.566904 $21 min$ 46.360907 $5top$ fast MM		•	est Run No.	1			<u> </u>
Test Start Time 05444 Shy = 6 H x 1 MdhTest End Time 0907 TimeElapsed Time (min)Water Depth (ft)TimeElapsed Time (min)Water Depth (ft)08443946.3608444start test, stap in will.08529win46.35015613mn46.36085751octon085716min46.56090421min46.360907Stop testNM							
TimeElapsed Time (min)Water Depth (ft)TimeElapsed Time (min)Water Depth (ft)0843946.360844544 test, sky in vell.08529 win46.35085613 win46.36085716 win46.56690421 win46.360907Stop test					Test End 7		0907
Ime (min) (f) Ime (min) (min) (f) 0843 9 46.36 0844 shaf test, sky in will. 0852 7 win 46.35 0856 13 ma 46.36 0857 16 min 46.56 6904 21 min 46.36 0907 5top f-st NM				$Sly = 6 H_{s}$	e 1 Mch		
0844 stat test, sty in will. 0852 9 min 46.35 0856 13 min 46.36 0857 Shuy out WM 0859 16 min 46.56 0904 21 min 46.36 0907 Stop test NM		Time		•	Time		
0852 7 min 46.35 0856 13 min 46.36 0857 Slug out UM 0859 16 min 46.56 6904 21 min 46.36 0907 Stop fest NM			Ø	46.36			
0856 /3 mn 46.36 0857 Slug oct UN 0859 /6 mn 46.56 6904 21 min 46.36 0907 Stop tost NM							
0857 Slug out un 0859 16 min 46.56 6904 21 min 46.36 0907 Stop fost NM			and the second se				
0859 16 min 46.56 6904 21 min 46.36 0907 Stop test NM							
6904 21 min 46.36 0907 Stop test NM			Slug out	NM			
0907 Stop test NM							
See trans ducer duta for details		0407	Stop test	NM			
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PUMPING TEST DATA FORM (Sug test)

page lot1

LocationBusin Electric LesStatic Water Level 74.59 Type of WellMain down townExtraction Well Distance $$ Test Date $\$[telta$ Total Casing Depth $91.1\$$ Measuring Point ElevationTDCBorehole Diameter 6 MathType of Test $5lag$ testCasing Diameter 2 MathStep Number $$ Screened IntervalData logger Test Run No. $$lag$ test 1Sand Pack IntervalPumping Rate NA Lithology TestedTimeElapsed TimeWater DepthTimeElapsed TimeTimeSlup rest 11343 5 Slup rest 11355 76.59 Slup rest 11355 76.63 Hypi utanyob1355 76.63 Hypi 18 76.63 Hypi 2 540 test						Cong 1	
Type of WellNume dense beingTest Date $gliclicTotal Casing DepthTest DategliclicTotal Casing DepthMeasuring Point ElevationTDCBorehole DiameterType of TestSling + e_2 + Casing Diameter2 - h_{ele} h_{ele}Step NumberScreened IntervalData logger Test Run No.Sling + e_2 + 1Sand Pack IntervalPumping RatehALithology TestedTimeElapsed TimeMater DepthTimeSling tract13439 + 46.59Sling tract13439 + 46.59Sling tract13439 + 26.53Sling tract13439 + 26.53Sling tract1352510696.62Sling Trat2100596.62Sling Trat2100596.62Sling Trat2100596.62Sling Trat2100596.62Sling Trat2100596.65Sling Trat2100596.65Sling Trat2100596.65Sling Trat2100596.65Sling Trat2100596.65Sling Trat2100596.65Sling Trat2100596.65Sling Trat2100596.65Sling Trat2100596.65Sling Trat2<$	Ţ	Well ID		MW-45B	Personnel Hurshman		
Test Date $g(tkl.c)$ Total Casing Depth 91.18 Measuring Point Elevation TDC Borehole Diameter 6 /h/kType of Test $5lag + t_2 + 1$ Casing Diameter 2 /h/kStep Number $$ Screened IntervalData logger Test Run No. $5lag + t_2 + 1$ Sand Pack IntervalPumping Rate μ/A Lithology TestedTest Start Time 1343 Test End TimeSlag + test in $6 + 1 + 1$ OctoacterTimeElapsed TimeWater Depth (min) me (R) 1345 $5lag$ 76.59 1345 $5lag$ 74.59 1345 $5lag$ 74.59 1345 $5lag$ 74.59 1345 $5lag$ 74.63 4401 18 74.63 402 $54e$ $4ee$ 1402 $54e$ $4ee$ 1402 $54e$ $4ee$ 1402 $54e$ $4ee$ 1402 $54e$ $4ee$ 1423 18 76.57 1423 18 76.62 1425 $54g$ $4ee$ 1435 30 74.65 1435 30 74.65 1435 30 74.65 1435 30 74.65 1435 30 74.65 1435 30 74.65 1435 30 74.65 1435 33 74.64	Ι	Location		Busin Electric LRS	Static Wat	er Level	76.59
Measuring Point ElevationTDCBorehole Diameter 6 Met Type of Test $Sleq + cyt$ Casing Diameter 2 Met Step Number $$ Screened IntervalData logger Test Run No. $3leg + rst + 1$ Sand Pack IntervalPumping Rate MA Lithology TestedTest Start Time 1343 Test End TimeSleg + rst + 1Sand Pack Interval $Macharter$ Pumping Rate MA Lithology TestedTest Start Time 1343 $G + 1 \text{ Meth}$ Sleg + rst + 1 1343 $G + 1 \text{ Meth}$ Sleg + rst + 1 1343 $G + 16.59$ Note: 1345 $Sleg + rst$ 1345 $Sleg + rst$ 76.55 Sleg + rst + 1 1352 $sleg + rst$ 1355 13 76.63 1401 18 76.63 1402 $Sleg + rst$ Met 1403 18 76.67 1423 18 76.65 1423 33 76.65 1435 33 76.65	Г	Type of Well		Mon ton well	Extraction	Well Distance	
Measuring Point ElevationTDCBorehole Diameter 6 huleType of Test $3\log + s + 1$ Casing Diameter 2 huleStep Number $$ Screened IntervalData logger Test Run No. $3\log + s + 1$ Sand Pack IntervalPumping Rate UA Lithology TestedTest Start Time 1533 Test End TimeSlog dimensional Coll Coll Coll Coll Coll Coll Coll Co	Г	Test Date		Slielic	Total Casi	ng Depth	91.18
Type of Test $3 \log + e_2 +$ Casing Diameter $2 \mod 4$ Step Number $$ Screened IntervalData logger Test Run No. $3 \log + e_3 + 1$ Sand Pack IntervalPumping Rate NA Lithology TestedTest Start Time 13×3 Test End TimeSlog dimension $2 \in 44 \times 1$ AchdimenterTimeElapsed TimeWater Depth (ft)Time 13×3 76.55 Slog trees 76.55 Slog trees 76.55 Slog trees 13×5 Slog trees 76.63 (up 2) $5 \log p \exp 4 \log 4$ 10×2 $5 \otimes 76.63$ (up 2) $5 \log p \exp 4 \log 4$ 10×2 $5 \otimes 76.63$ (up 2) $5 \log p \exp 4 \log 4$ 14×6 $5 \otimes 76.65$ 14×2 $5 \otimes 76.65$ 14×2 $5 \otimes 76.65$ 14×2 $24 \otimes 76.65$ 14×2 $24 \otimes 76.65$ 14×2 $24 \otimes 76.65$ 14×3 $33 \otimes 76.65$	Ν	Aeasuring Poir	nt Elevation	TOC			
Step Number $$ Screened IntervalData logger Test Run No. $3lug +rst + 1$ Sand Pack IntervalPumping Rate UA Lithology TestedTest Start Time $13:43$ Test End TimeSlug dimensions : $6 Jd \times 1$ Nuch dimenterTimeElapsed TimeWater Depth (ft)TimeElapsed Time (min)TimeSlug tract i 1343 9 76.59 1345 $5lug$ inSlug tract i 1352 $9lug$ out (ft)Note: 1352 $9lug$ out (ft)Slug tract i 1352 $9lug$ out (ft)Note: 1352 $9lug$ out (ft)Slug Trot 2 1405 9 1405 $9lug$ in 1252 $9lug$ in 1405 1425 $5lug$ in 1425 MM 1432 1425 $5lug$ out 1435 30 1425 $5lug$ out 1435 33 1438 33 $7l_{16}65$ 1438 33 $7l_{16}44$	Т	ype of Test		Slag test			
Data logger Test Run No. $3 \log 4 + s + 1$ Sand Pack IntervalPumping Rate NA Lithology TestedTest Start Time $13:43$ Test End Time $5 \log dimensions + 6 L4 + 1 MachdimenterTimeElapsed Time(min)Water Depth(ft)TimeTimeElapsed Time(min)76.5913455 \log in13455 \log in13455 \log in1348576.553 \log in1348576.6314011876.6314025 \log in14025 \log in14255 \log in14253 276.6514353 3$					-		
Pumping Rate $\begin{tabular}{ c c c c c } \hline \begin{tabular}{ c c c c c c c c c c c c c c c c c c c$			st Run No	stug test 1			<u> </u>
Test Start TimeTest Start TimeSlug dimensions : $6 \ L4 \ \times 1$ And dimenterTimeElapsed Time (min)Water Depth (ft)TimeElapsed Time (min)Water Depth (ft)Slug read 11343 $6 \ 76.59$ 76.59 76.55 Slug read 11348 $5 \ 76.55$ 76.63 Slug read 11352Slug out 1401 76.63 July suburged135613 76.63 July suburged135613 76.62 July Suburged13 76.657 July Size32 76.65 July Suburged13 76.657 July Suburged13 76.657 July Size21 76.657 July Size33 76.64			51 ICull 140.				
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$\begin{array}{cccccccccccccccccccccccccccccccccccc$			slug in				
$ \int u II \int SUBMIT = 15 + 15 + 16.63 [401 18 + 76.63 [402 + 164] [402 + 164] Slug Trot 2 1405 14 + 154 140 [1406 + 5164 + 1454 140] [1408 + 5164 + 1454 140] [1408 + 5164 + 164] [1408 + 5164] [1408 + 5164] [1408 + 5164]] [1408 + 5164]] [1408 + 5164]] [1408 + 516] [1408 + 516] [1408 + 516]] [1408 + 516]] [1408 + 516]] [1408 + 516] [1408 + 516] [1408 + 516] [1408 + 516] [1408 + 516] [1408 + 516]] [1408 + 516] [1408 + 516] [1408 + 516]] [1408 + 516] [1408 + 516] [1408 + 516]] [1408 + 516] [1408 + 516] [1408 + 516] [1408 + 516] [1408 + 516] [1408 + 516]] [1408 + 516] [1408 + 516]] [1408 + 516]] [1408 + 516]] [1408 + 516]] [1408 + 516] [1408 + 516]] [1408 + 516]] [1408 + 516]] [1408 + 516]]] [1408 + 516]] [1408 +$	Note:	1348	5	76.55			
$ \int u II \int subuli \int I S = 15 + 4.65 [401 I8 76.63 [402 Stop + 100 [40$	Slug hur be	m 1352	Slug out				
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	1.117 Subar	Jd 1356	13	76.63			
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$\begin{array}{cccccccccccccccccccccccccccccccccccc$		402	Stop toot				
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$\begin{array}{cccccccccccccccccccccccccccccccccccc$			stat test				
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$\begin{array}{cccccccccccccccccccccccccccccccccccc$		1418		76,57			
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1432 27 76.69 1435 30 76.65 1438 33 76.64			slug ont				
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		1438					
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PUMPING TEST DATA FORM (Slug Jest)

see transducer data for details.

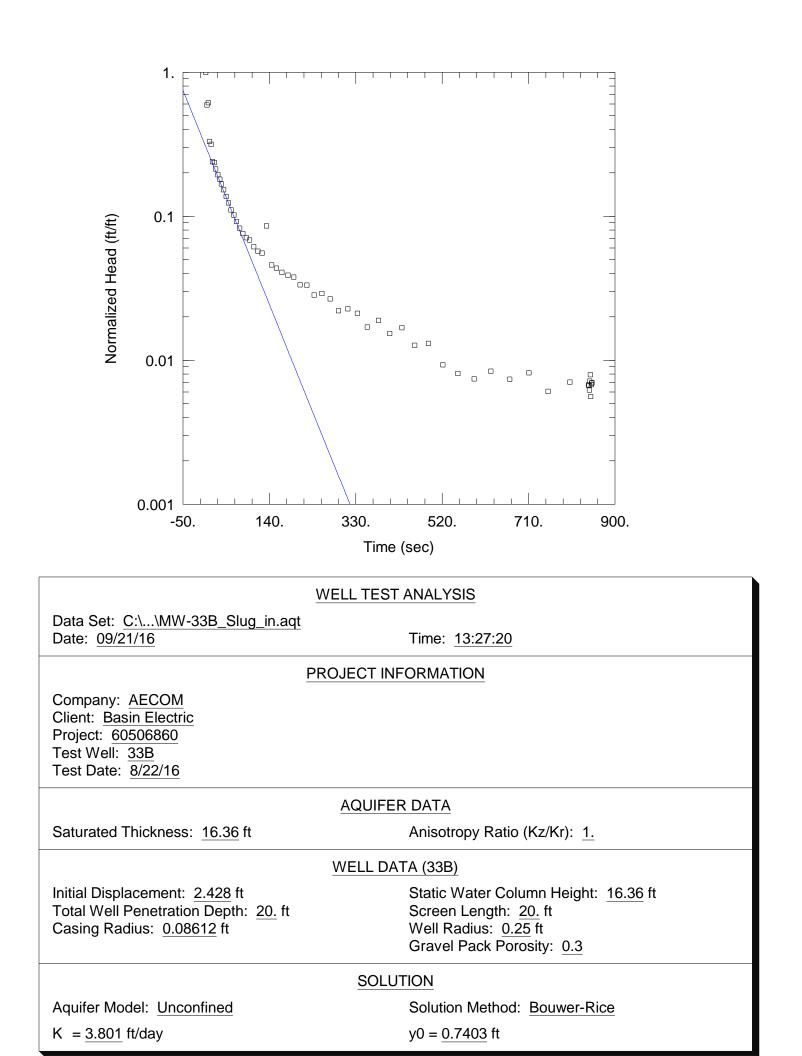
page 1071

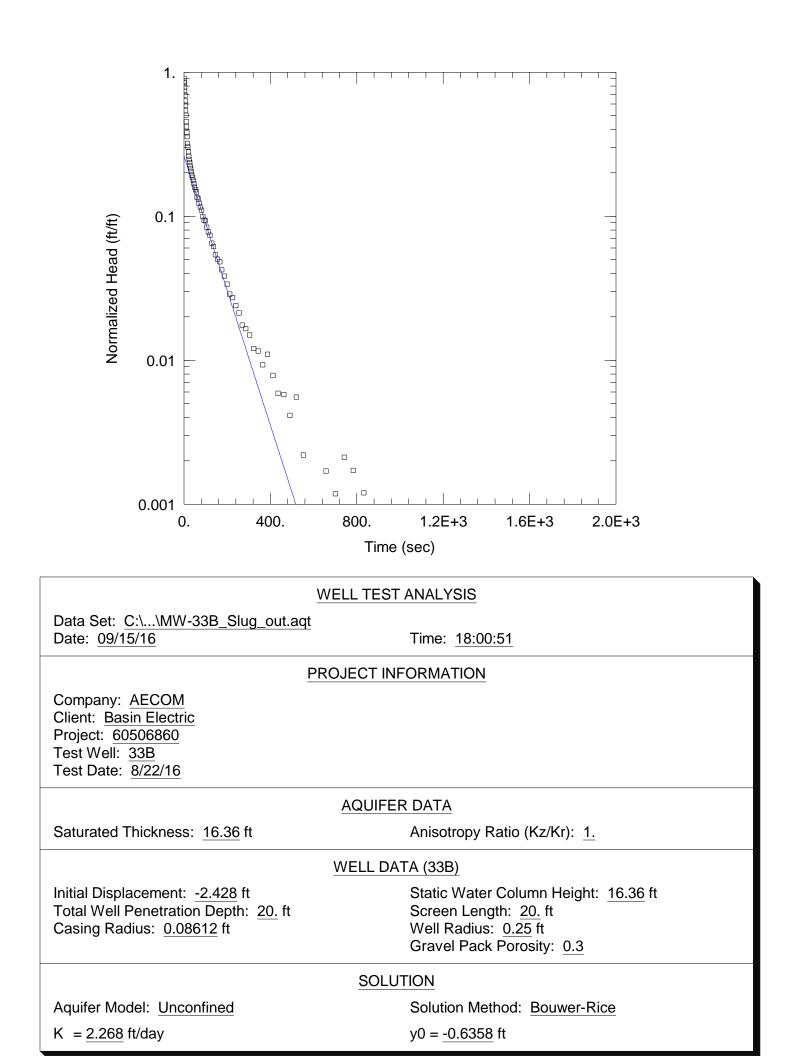
PUMPING TEST DATA FORM	(Slug Test)	
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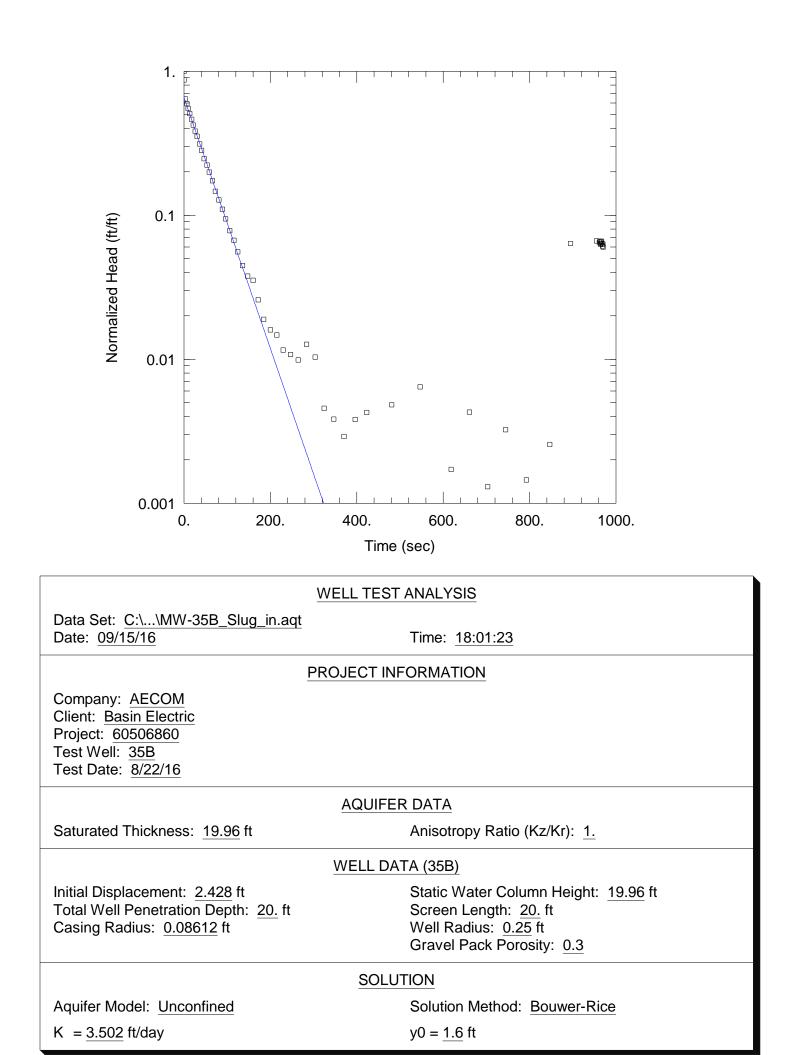
Well ID	MW-473	Personnel Hurshman	
Location	Basin Electric LRJ	Static Water Level	75.86 btoc
Type of Well	hun tony well	Extraction Well Distance	
Test Date	8/18/14	Total Casing Depth	90.76 bJoc
Measuring Point Elevation	TOC	Borehole Diameter	6 inch
Type of Test		Casing Diameter	2 Inch
Step Number		Screened Interval	
Data logger Test Run No.		Sand Pack Interval	
Pumping Rate	MA(slug)	Lithology Tested	
Test Start Time	1525	Test End Time	1610

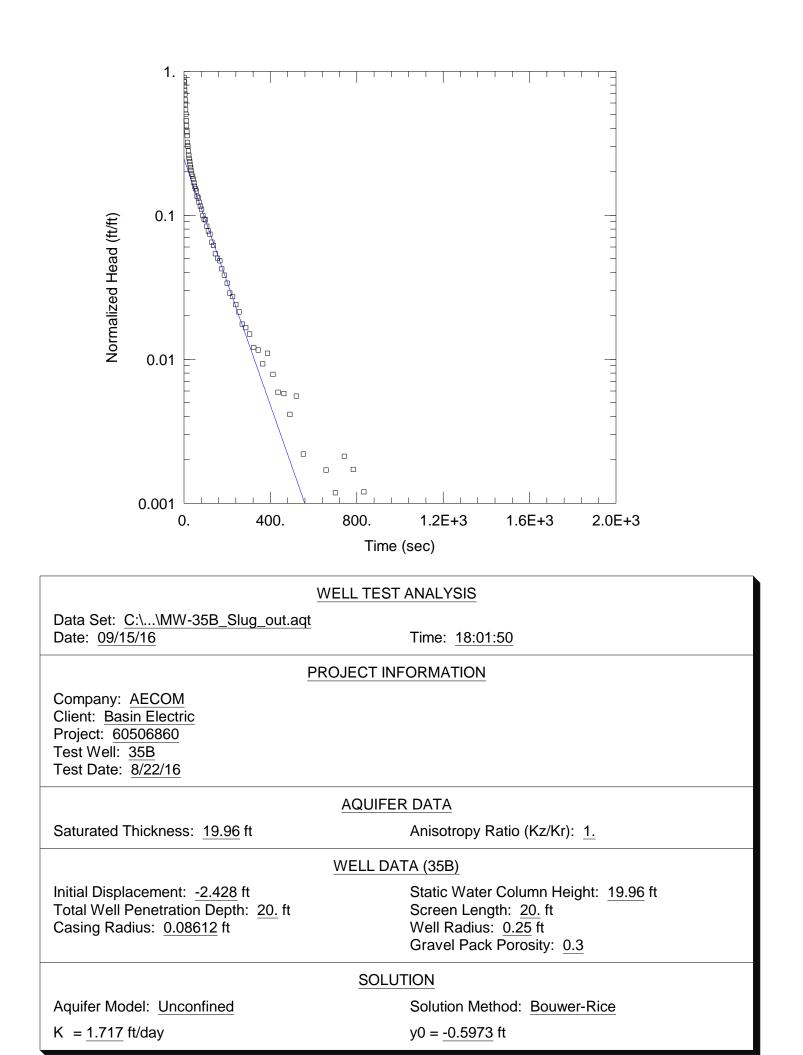
Elapsed Time Water Depth Elapsed Time Water Depth Time Time (min) (ft) (min) (ft) P test stated Test 1 1324 75.86 1575 NM 1526 slug m NM 1533 9 min 75.7.3 1528 75.81 14 min 1546 22 min 75.84 1549 25 min \$5.85 1551 Slugort NM 1555 31 min 76.05 36 m.n 75.12 1600 1604 75 89 40 m. 1609 45 min 75.89 NM 1610 Stop test

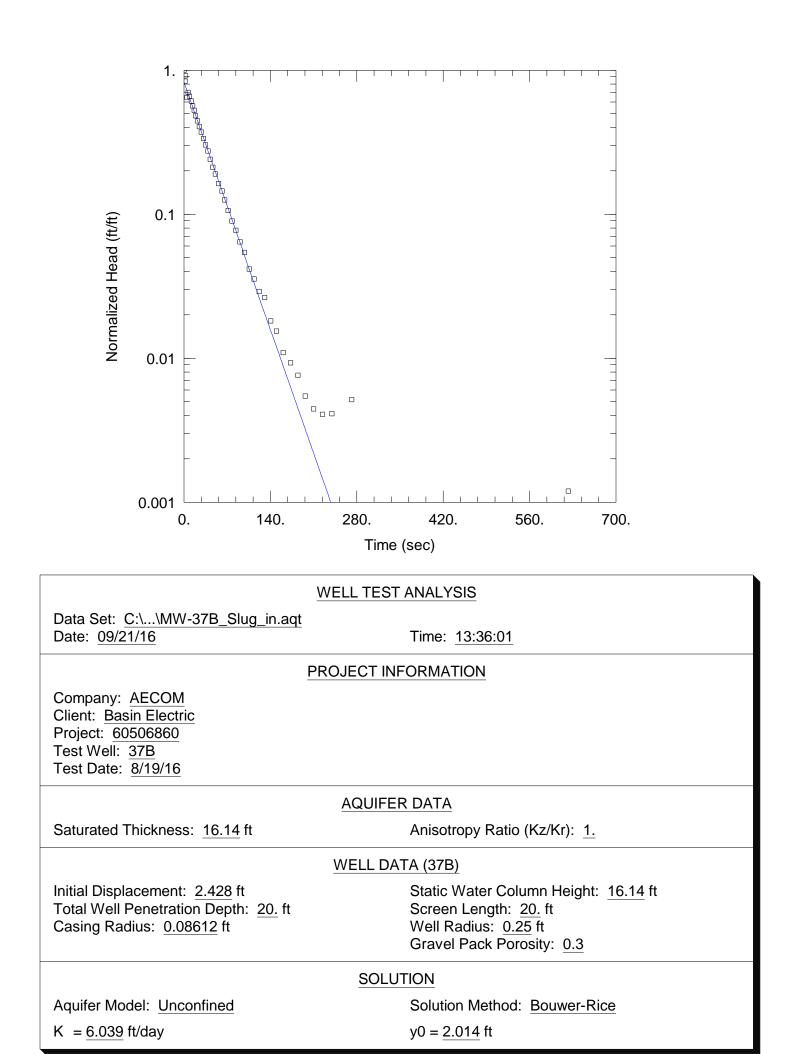
See transdace data for details

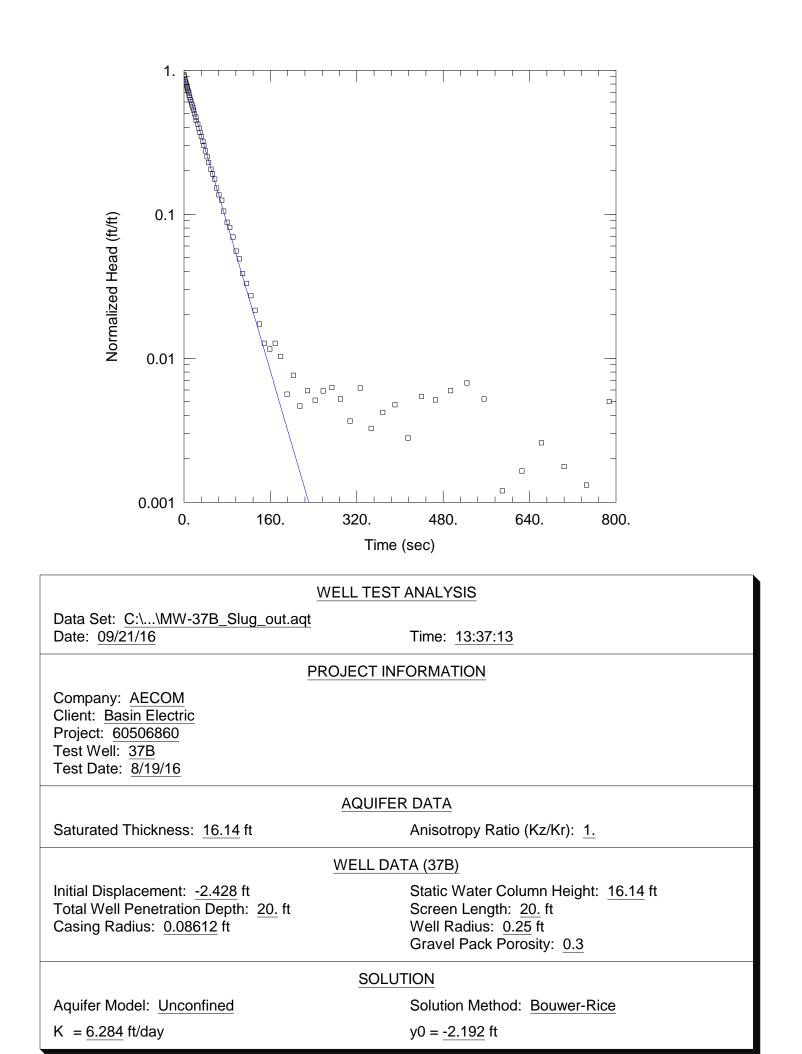


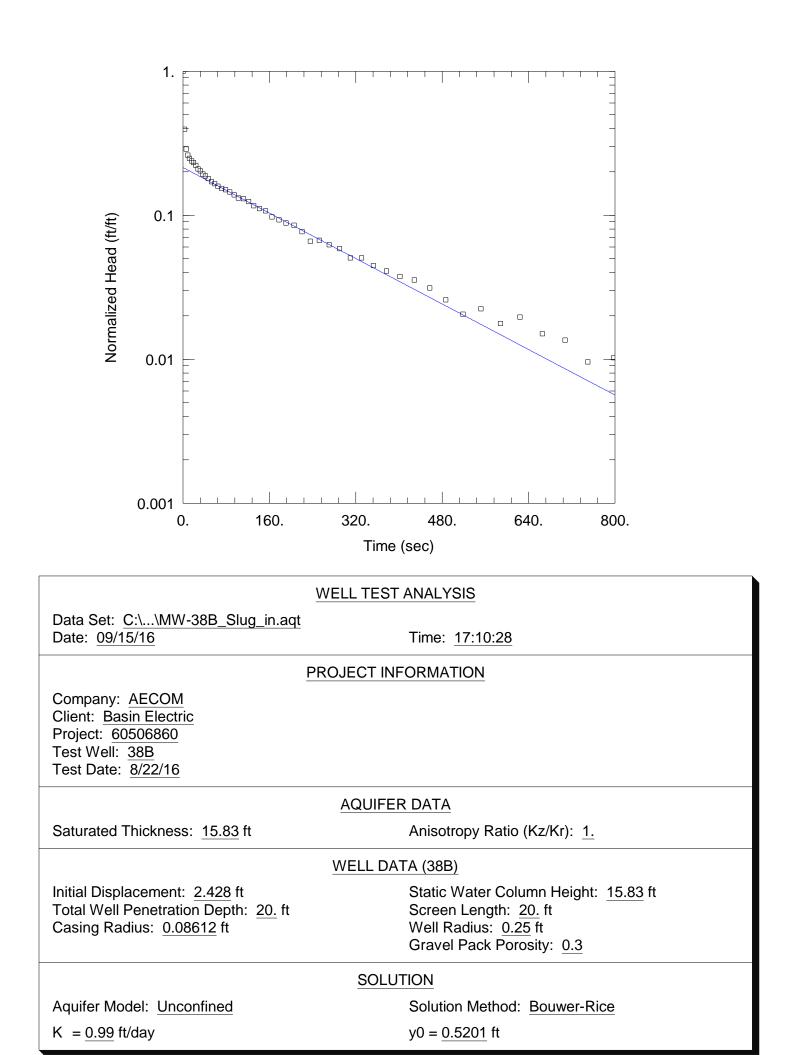


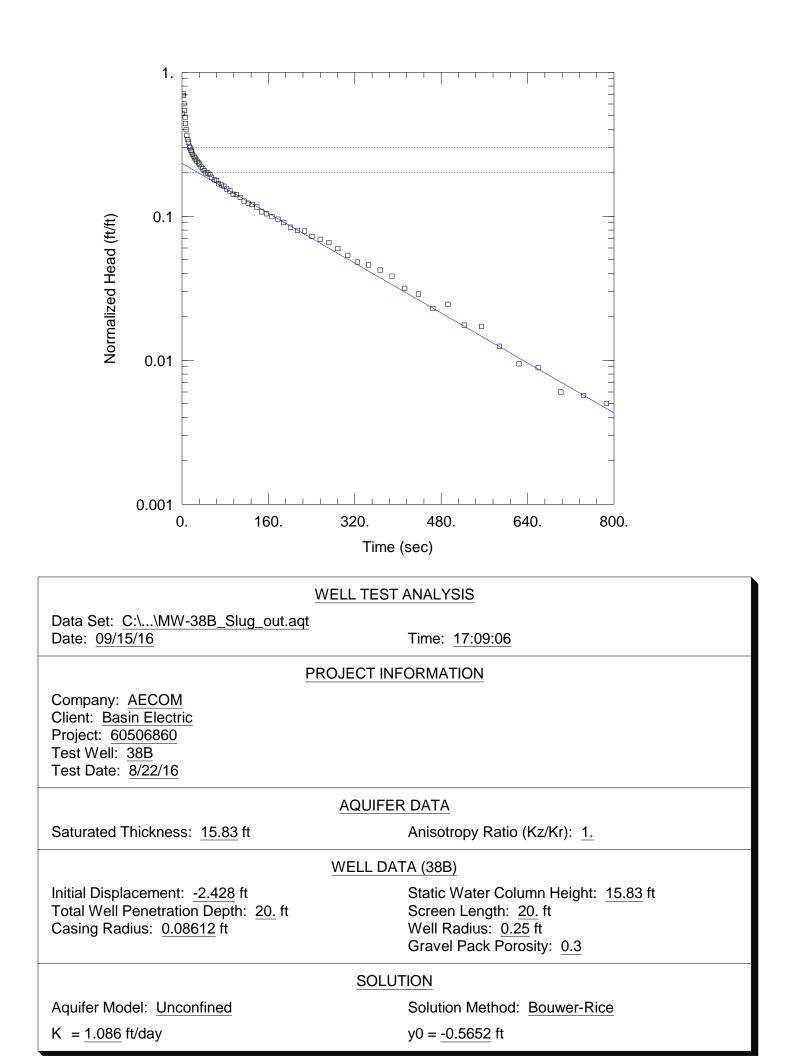


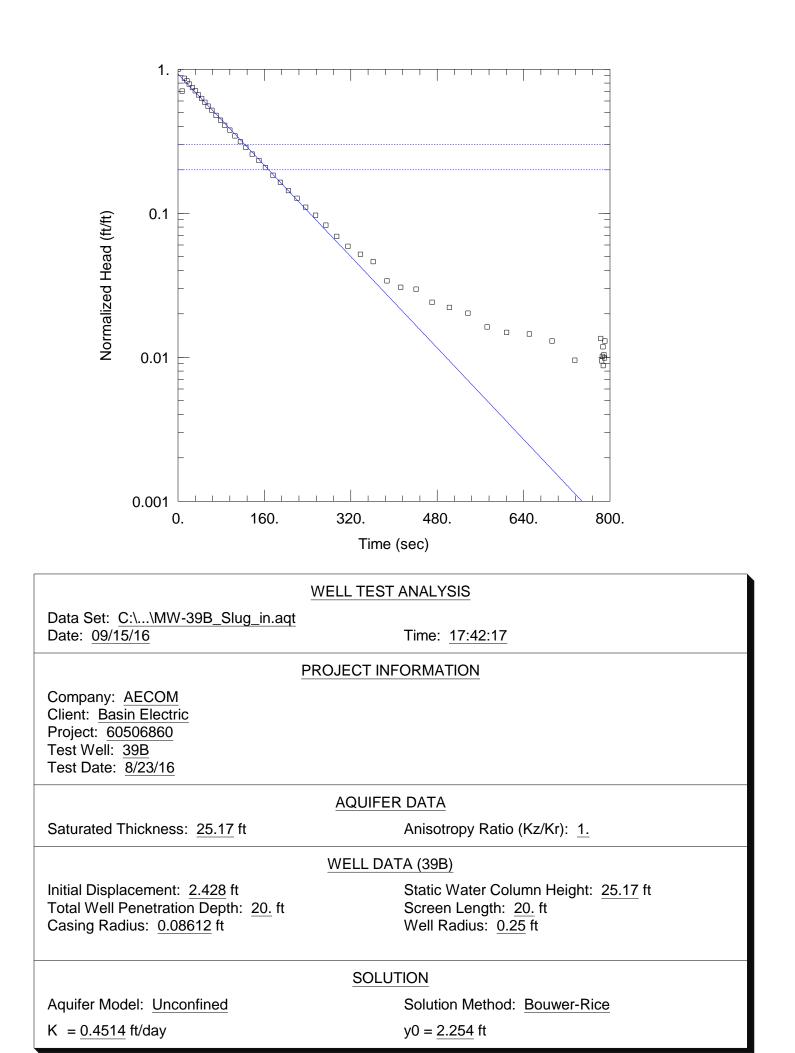


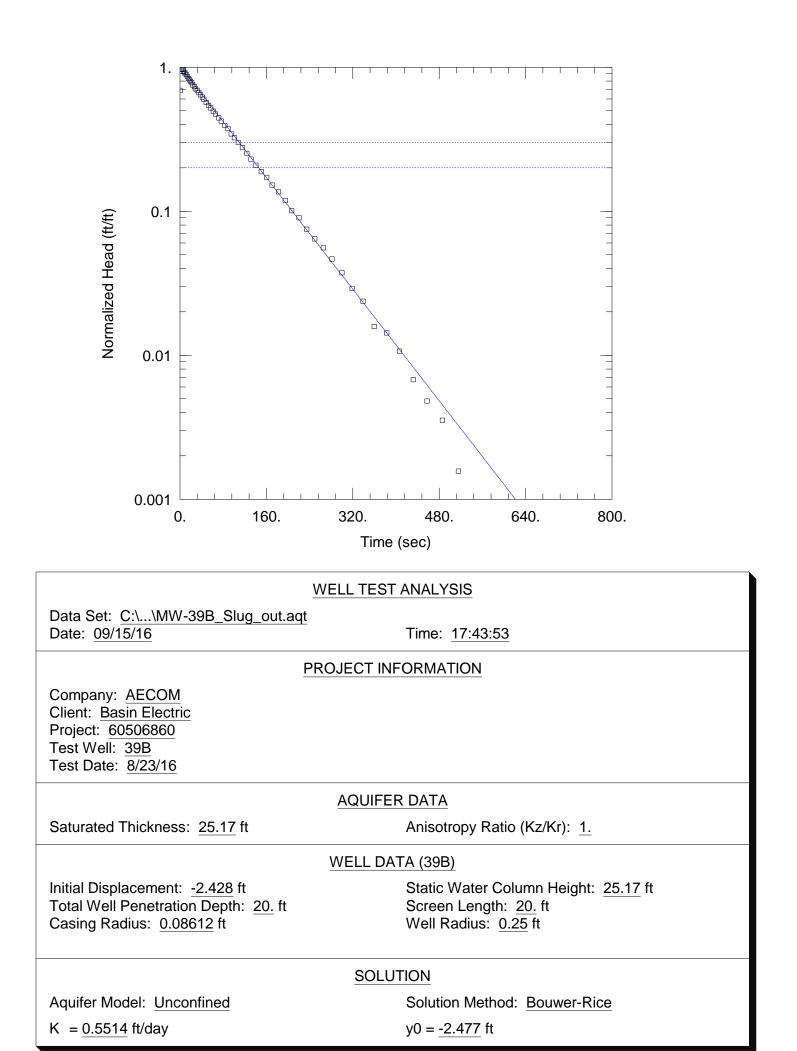


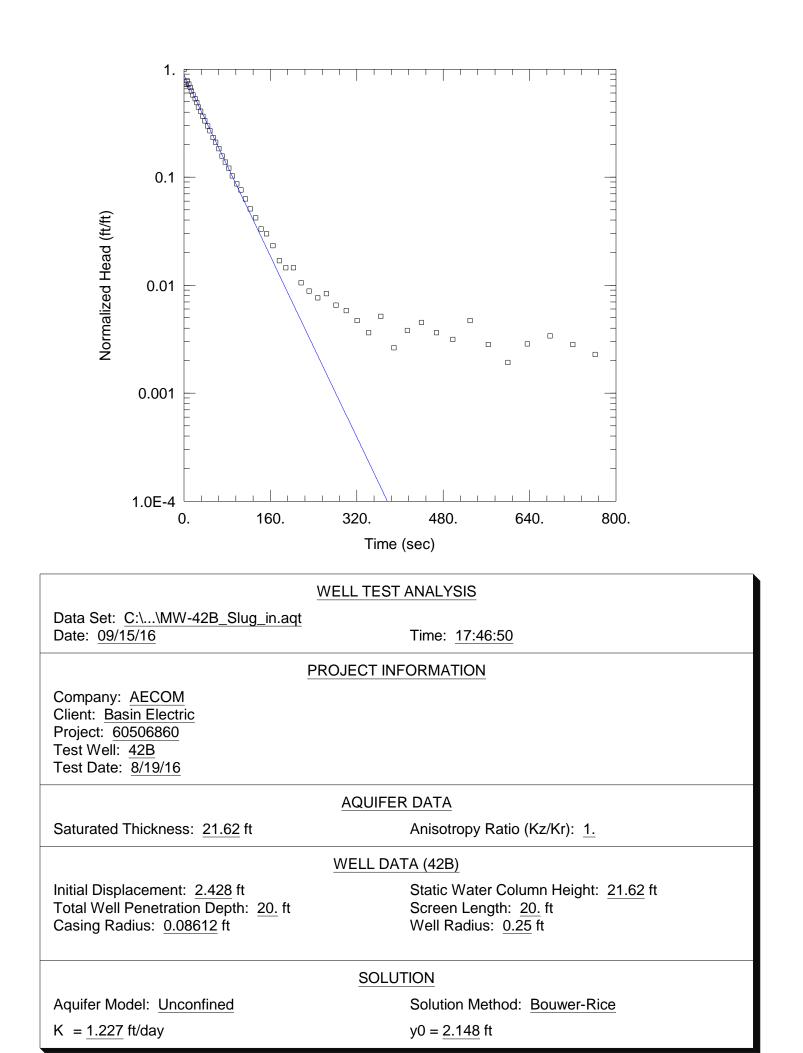


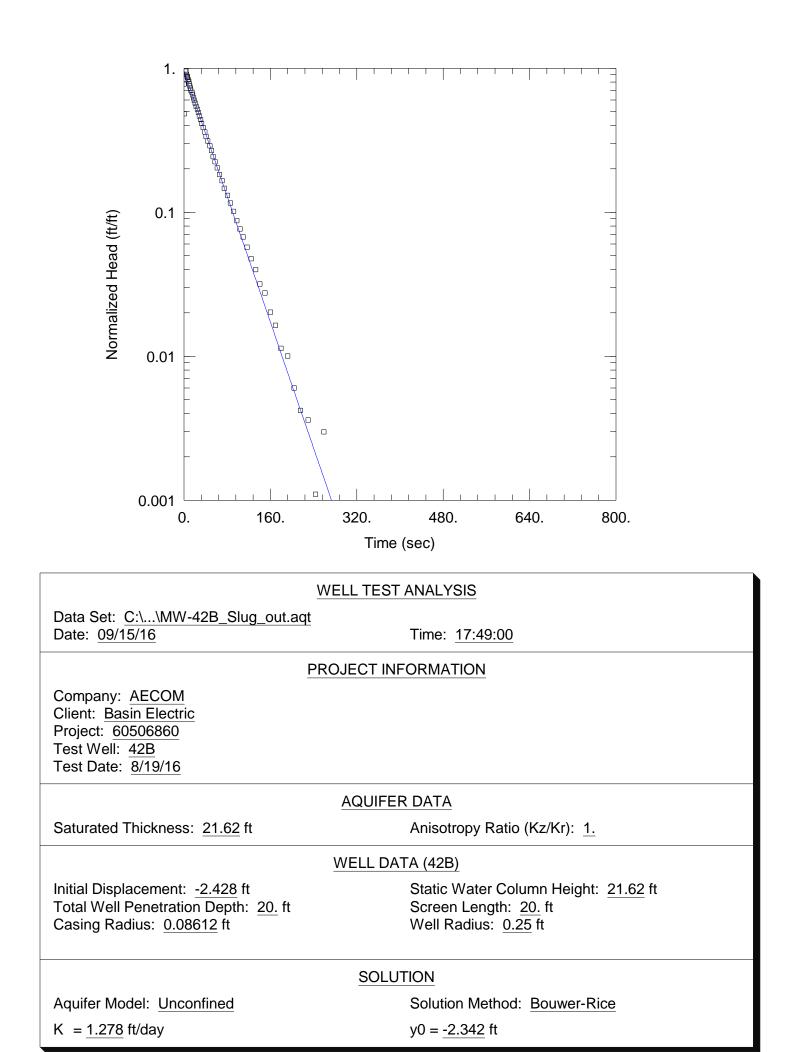


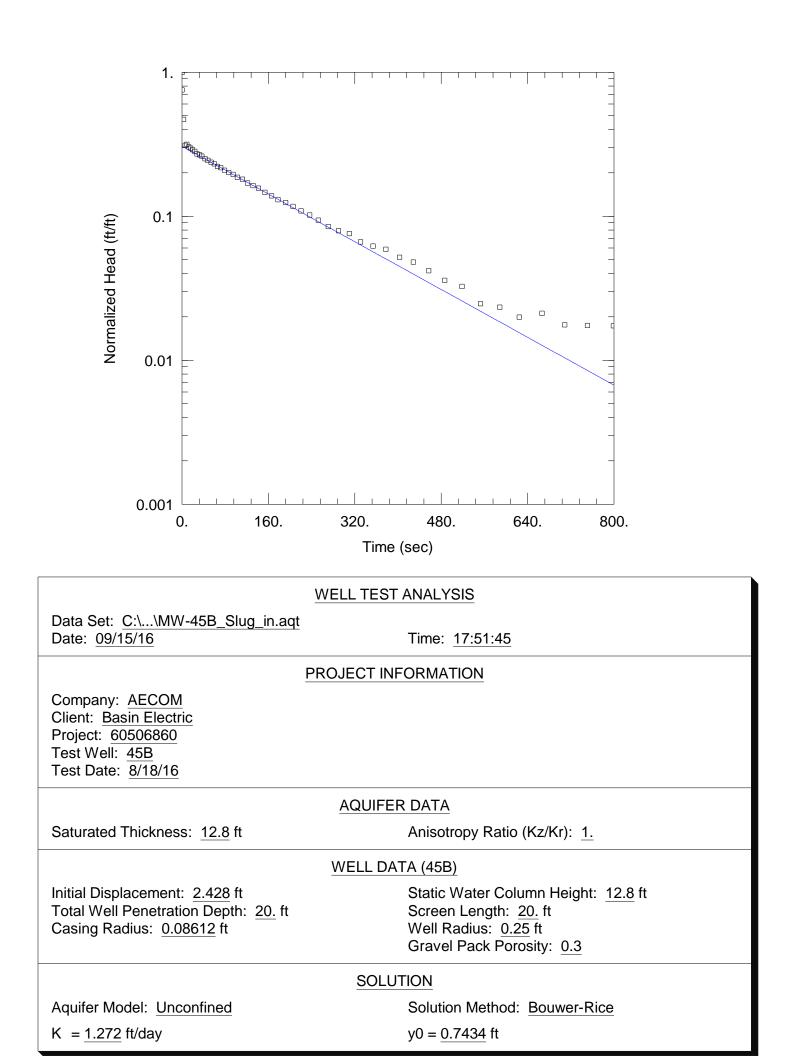


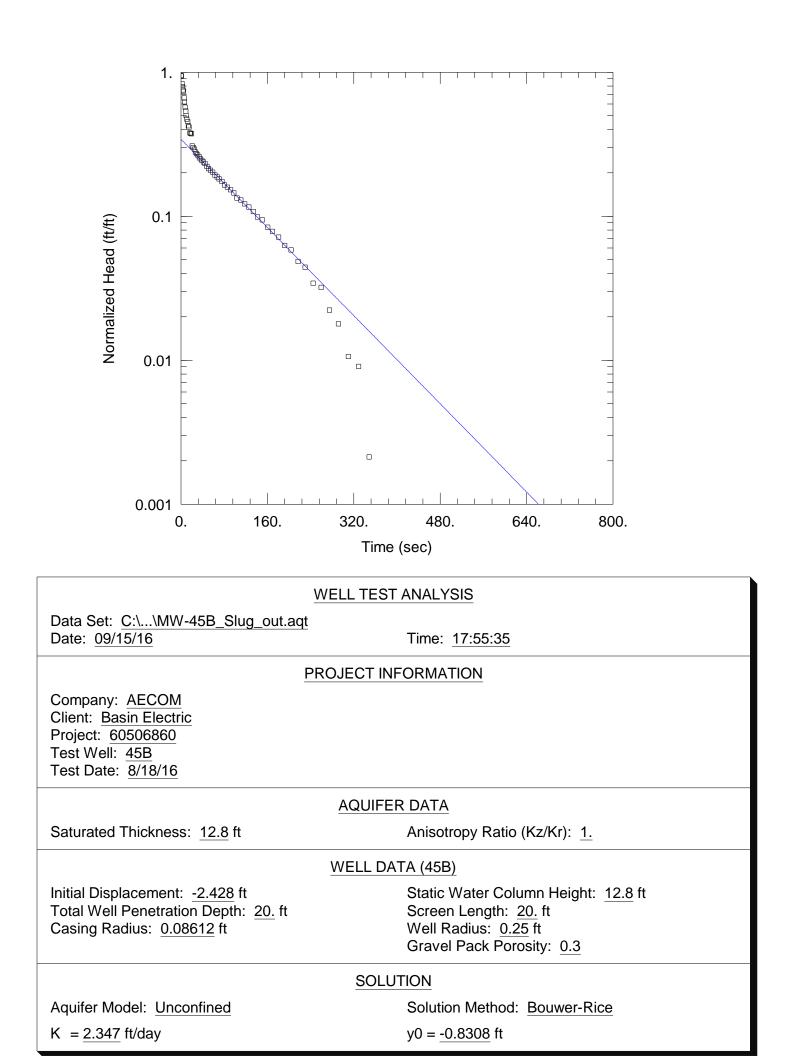


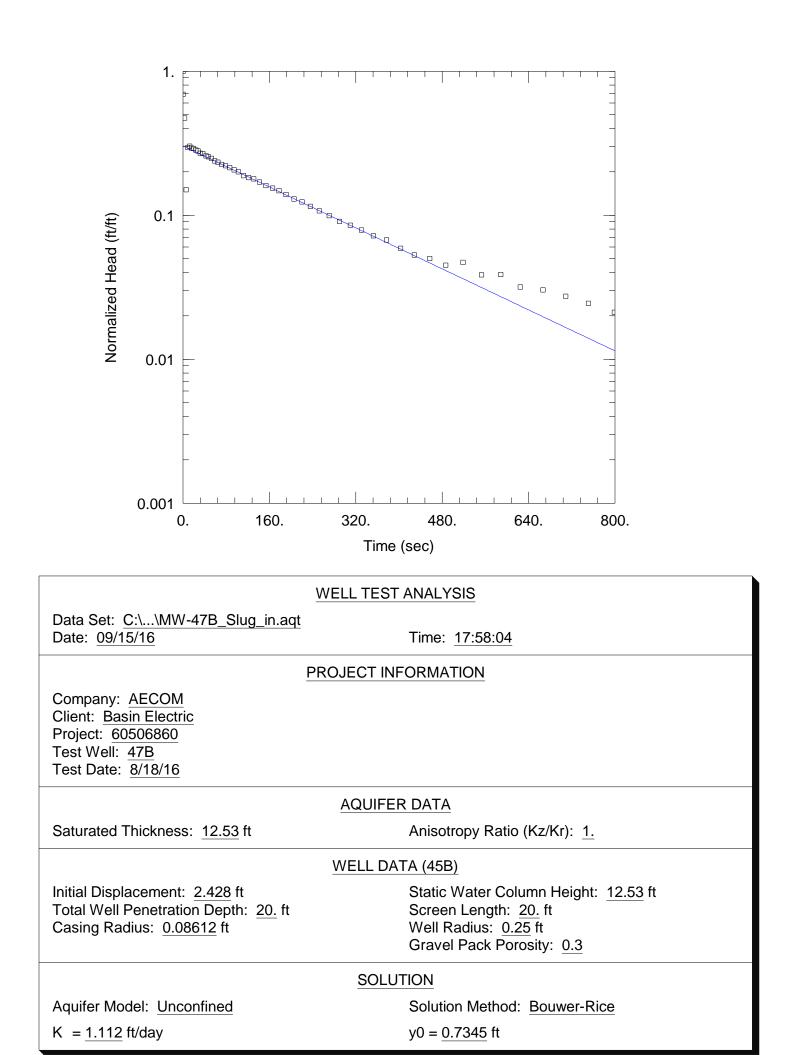


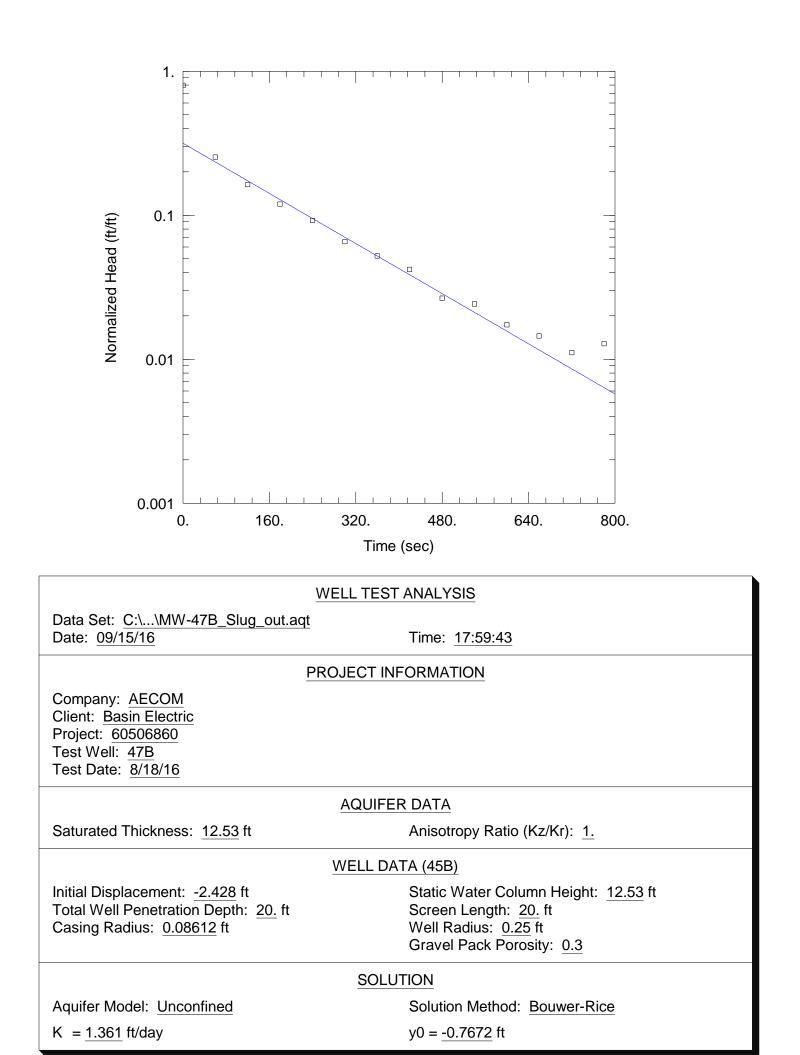












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Well ID	MW-32B	Personnel Church Almand	+ + Fereny Hurshman
Location		Static Water Level	60.43
Type of Well	PVC Schedule 40	_ Extraction Well Distance	
Test Date	8/22/2016	Total Casing Depth	76.74
Measuring Point Elevation	Top of Imer Caring	Borehole Diameter	611
Type of Test	Recovery	Casing Diameter	2"
Step Number		Screened Interval	
Data logger Test Run No.		Sand Pack Interval	
Pumping Rate	0.8 gpm	Lithology Tested	
Test Start Time	1604	Test End Time	1830

	1604AM	Time	Elapsed Time (min)	Water Depth (ft)	Time	Elapsed Time (min)	Water Depth (ft)
Stant	183	1604	Ø	60.30			
. AF		1620 1631 1644 1657 1709 1722 1735	145 381 567 89	10ga) 7.76 20gal 7.62 20gal 7.62 20gal 7.54 40gal 7.51 50gal 7.47 60gal 7.47			
print 1	735	1740	94	C1.22			
	- ,	1748	102	60.65			
		17:55	109	60.56			
		1759	113	60,51			
		1804	118	60.49			
		1814	128	60.45			
		1819	133	40.45			
		1824	135	60.43			

Well ID	MW. 34B	Personnel Chris Amer	et & Jevery Hurshman
Location	Basin Electric	Static Water Level	66.54
Type of Well	PC, Scholk 40	Extraction Well Distance	
Test Date	8 22 2016	Total Casing Depth	08.78
Measuring Point Elevation	Top of Innar Costing	Borehole Diameter	<u>L"</u>
Type of Test	Recoiles	Casing Diameter	2"
Step Number		Screened Interval	
Data logger Test Run No.		Sand Pack Interval	
Pumping Rate	1 gpm	Lithology Tested	
Test Start Time	12:52 PM	Test End Time	

Time Elapsed Time (min)		Water Depth (ft)	Time	Elapsed Time (min)	Water Depth (ft)
1251	ø	66.56			
1304	12	10gal 9.05			
1316	24	20gal 7.67			
1327	35	30901 7.70			
1339	47	40gal 17.72			
020	58	50 gal /7.78			
402	40	60ga1/7.71			
1409	79	7001 17.74			
1414	Stopped purp	UNM			
1416	86	72.40			
1423	93	67.22			
1432	102	66.69			
1439	109	66.67			
145 z	122	66.63			
1502	132	66.61			
1509	139	66.62			

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Well ID	MW-36B Personnel Oppils August & Turemy Hurshim
Location	Basin Electric LPS Static Water Level 61.21
Type of Well	PVC Schedule 40 Extraction Well Distance
Test Date	8 22 ZOVG Total Casing Depth 80.25
Measuring Point Elevation	Topos PVC Casing Borehole Diameter
Type of Test	Casing Diameter 2 ¹¹
Step Number	Screened Interval
Data logger Test Run No.	Sand Pack Interval
Pumping Rate	0.7gpm Lithology Tested
Test Start Time	10:17 AM Test End Time 12:12 PM

	Time	Elapsed Time (min)	Water Depth (ft) Healt of	Time	Elapsed Time (min)	Water Depth (ft)
AM	10:15 AM	D	61.22			
10:17 Jont	10.31	14	10yd, 12.40 +			
•	1046	29	20 co.1/ 12.95+			
	1100	43	30 Gal 12.87 1			
	1114	57	40gal 12.84 +			
	1128	+1	50gal, 12.84 -			
S - A DAMO (B	1143	86	60g1/ 12 951			
Stopped pumpe 1151 pm	1151		62. Soul/NR			
	1204		61.23			
stopped test	1212 PM		4122			

PUMPING TEST DATA FORM

Well ID	MW-40B	Personnel Ciwis Ahre	undt & Jevenny Hurshman
Location	Bosin Electric	Static Water Level	94.89
Type of Well	PVC, Schedule 40	Extraction Well Distance	
Test Date	8/23/2010	Total Casing Depth	111.10
Measuring Point Elevation	Top of Inner Casi	Borehole Diameter	(o ^{''})
Type of Test	Aquiter Test	Casing Diameter	2"
Step Number		Screened Interval	
Data logger Test Run No.		Sand Pack Interval	
Pumping Rate	~ 0,9 spm	Lithology Tested	
Test Start Time	·	Test End Time	

Time	Elapsed Time (min)	Water Depth (ft)	Time	Elapsed Time (min)	Water Depth (ft)
0939	Ø	94.88			
0940	lmin	Formp on,			
0951	[2 min	10gal/3.10			
1003	26 min	20gal 2.72			
1016	39 min	30gal /2.45			
1029	52 min	40ga(/2.61			
1042	65 min	50 gal 258			
1054	tt min	60%1/ 2.55			
(101	84 min	65gal /2.56			
1)04)	pump off	NM			
1106	89 min	98.58			
1112	95 min	96-62			
1118	104 mm	95.69 95.15			
1132	[18 min				
1138	12 Ge min	95-10			
1146e	134 min	95.05			
1157	145 min	100			
1206	154 min	95.02			

& see transdacer deute der details

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PUMPING TEST DATA FORM

Well ID	MW-41B	Personnel Chris Ahren	1dt	
Location	Basin Electric	Static Water Level	56.71	PRSH-UD
Type of Well	Scheduk 40 PVC	Extraction Well Distance	A.1	
Test Date	8/19/2016	Total Casing Depth	75,51	
Measuring Point Elevation	Tup of Inner Carry	Borehole Diameter	611	
Type of Test	Aquiter constant rate	,	2"	r ^B
Step Number	- 	Screened Interval	3	£
Data logger Test Run No.		Sand Pack Interval		
Pumping Rate	1 gpm	Lithology Tested		2.9
Test Start Time	1049	Test End Time		

Time	Elapsed Time (min)	Water Depth (ft)	Time	Elapsed Time (min)	Water Depth (ft)
1048	Ø	56.77			
1059	Tì	10gal / 12.81			
1107	19	20gal/12.80			
1117	29	30 gal/12.79			
1128	40	40 cal / 12 70	41		
1142	54	55gal/12.76	2.70		
1150	42	60gal 12.78			
1200	72	Flaal			
1201	73	7180			1.00
1224		56.81 4			
1238		56.814			
	1059 1107 1117 1128 1142 1142 1150 1200 1201 1224	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$

brub off

Temperature drop and wind increase occurred at [1:20-11:30 AM, strong Wind gusts

Well ID	MW-43B	Personnel Chris Ahrendt & Jeremy Hurshman
Location	Basin Electric	Static Water Level 7:384 6 1914 24.48 Top of Invercosing
Type of Well	PVC Schol 40	Extraction Well Distance
Test Date	8/19/2016	Total Casing Depth7-38" 0/14 79.15 Topol Timer Cosing
Measuring Point Elevation	Jop of PVC Casing	Borehole Diameter
Type of Test	Carobout Rate Reach	Casing Diameter 2"
Step Number		Screened Interval
Data logger Test Run No.	·	Sand Pack Interval
Pumping Rate	2 gpm start of tot	Lithology Tested
Test Start Time		Test End Time

	Time	Elapsed Time (min)	Water Depth (ft)	Time	Elapsed Time (min)	Water Depth (ft)
	8:18 AM	Ø	24,49-5474			
Start 8: 19 AM	8:29	10	10gal /43.95			
test 2 pump	840	21	ZU gal / 4380			
	848	29	30gal / 43.77			
~ 19pm	9858	39	40gal / 4377			
	0903	49	50gal/4377			
	0919	60	60gal/43-81			
	0930	71	7080143.79			
	0947	83 88	80gal (3-78 85al /4372			
	0947		ped - recovery			
	0951	51	24.91' Stoc			
	6959	99	24.56			
	1002	102	24,54			
	1607	107	24.52			
	1011	111	24.52			

Pase 1 of 2 countion ES' bgs.

Well ID	MW44B	Personnel Climic Ahren	24 & Foremy Hunshman
Location	Bosin Electric LB	_ Static Water Level	70,58' without quipmant
Type of Well	2" PVC	Extraction Well Distance	
Test Date	8 18 2016	Total Casing Depth	94.06
Measuring Point Elevation	Top of PVC Casing	Borehole Diameter	(e "
Type of Test	Constant Rate	Casing Diameter	2"
Step Number		Screened Interval	
Data logger Test Run No.		Sand Pack Interval	
Pumping Rate		Lithology Tested	
Test Start Time		Test End Time	

3	Time	Elapsed Time (min)	Water Depth (ft)	Time	Elapsed Time (min)	Water Depth (ft)
& Test !	09:18	ø	70.59 state			
& Test) B (Tria)	6937		71.34 serving			
A (19pm)	0744		70.65			
	0946		70.64			
3) Test Z	0955	Ø	70.63 state			
& CO.S SPM)	1009		70.64 reany			
8 11-						

page 20FZ

Well ID	MW-44B	Personnel Alwa	what & Jarony Hursman
Location	BOSINELECTIC LRS	Static Water Level	
Type of Well	Z"PVC	Extraction Well Distance	
Test Date	8/18/2016	Total Casing Depth	94.06
Measuring Point Elevation	Top of imer PVC	Borehole Diameter	6"
Type of Test	Constant Rate	Casing Diameter	21
Step Number		Screened Interval	
Data logger Test Run No.		Sand Pack Interval	
Pumping Rate	0.4 gpm	Lithology Tested	
Test Start Time	J.	Test End Time	

44B I)	Time	Elapsed Time (min)	Water Depth (ft)	Time	Elapsed Time (min)	Water Depth (ft)
Fest 3 Crest	1045	p 1 start test	70.58	1 1		

MW-44B	1121	Ø	70.59	
Test4	1122	start test	NM	
- 1.5 Spm	1130	lo Sulla-s	VM	
	1137	20 Sallors	NM	
good darty	1144	30 gallons	NH	
-	1151	40 gullors	NM	
	1188	50 sallous	NM	
	stopped 7	simp at 1158 -	- recovery time.	
	1202		70.84	
	1205		70.68	
	1210		20.64	
	1213		70.63	
	1235		70.60	
	1236	Stopped ta	uef	

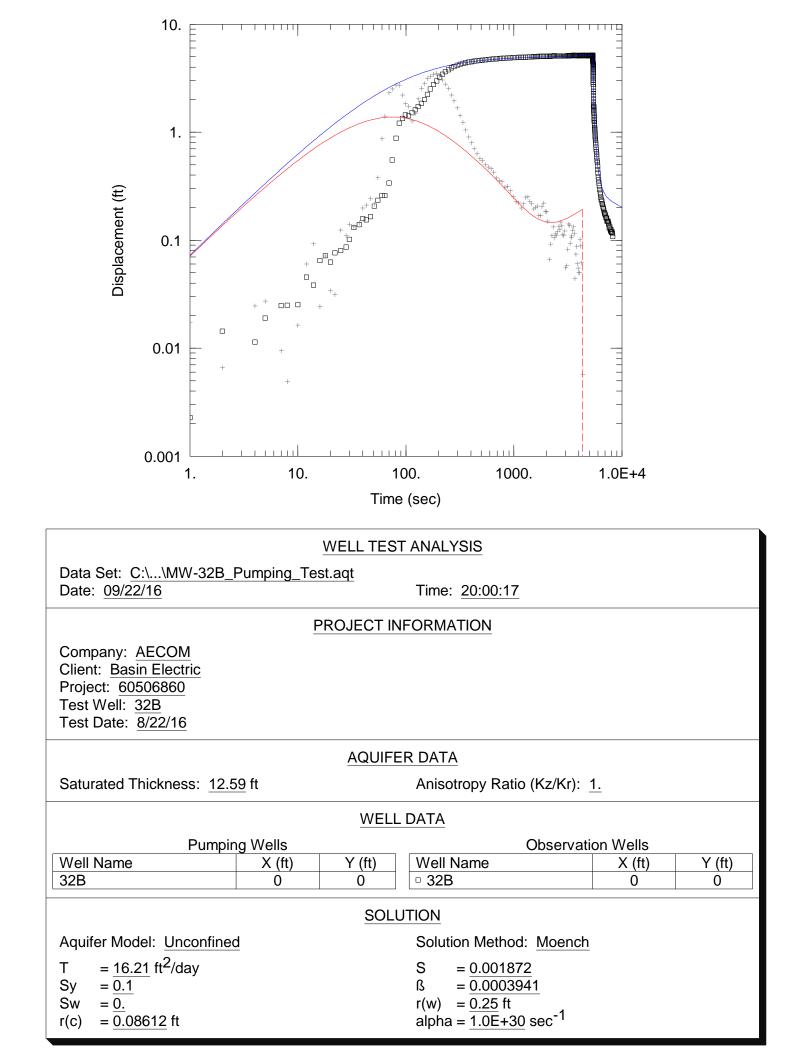
PUMPING TEST DATA FORM

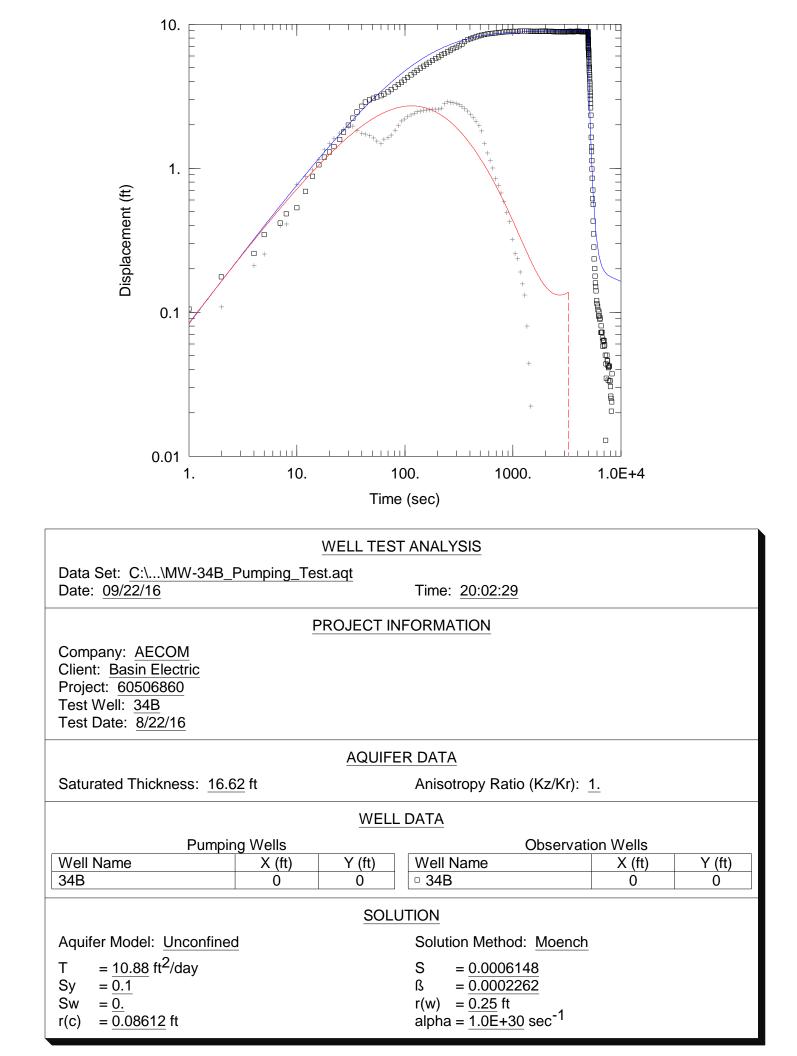
Well ID	MW-46B	Personnel Chris Phivand	F& Jeverny Hunshman
Location	BasinEkdnicLRS	Static Water Level	F6.00 76.00
Type of Well	PVC, Sch. 40	Extraction Well Distance	NA
Test Date	8/18/2016	Total Casing Depth	95,41
Measuring Point Elevation	Tupofimmer PY2 Co	Borehole Diameter	Git
Type of Test	Recording	Casing Diameter	2"
Step Number		Screened Interval	
Data logger Test Run No.		Sand Pack Interval	
Pumping Rate	~1.4 5pm	Lithology Tested	
Test Start Time	13:18	Test End Time	

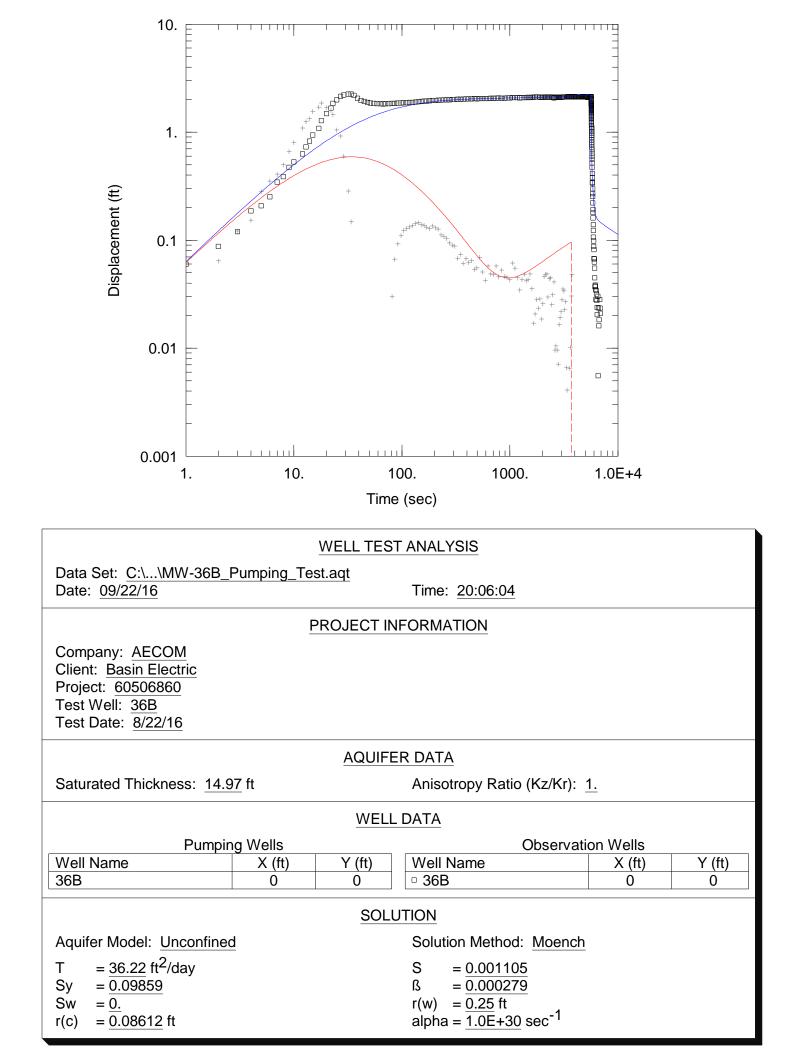
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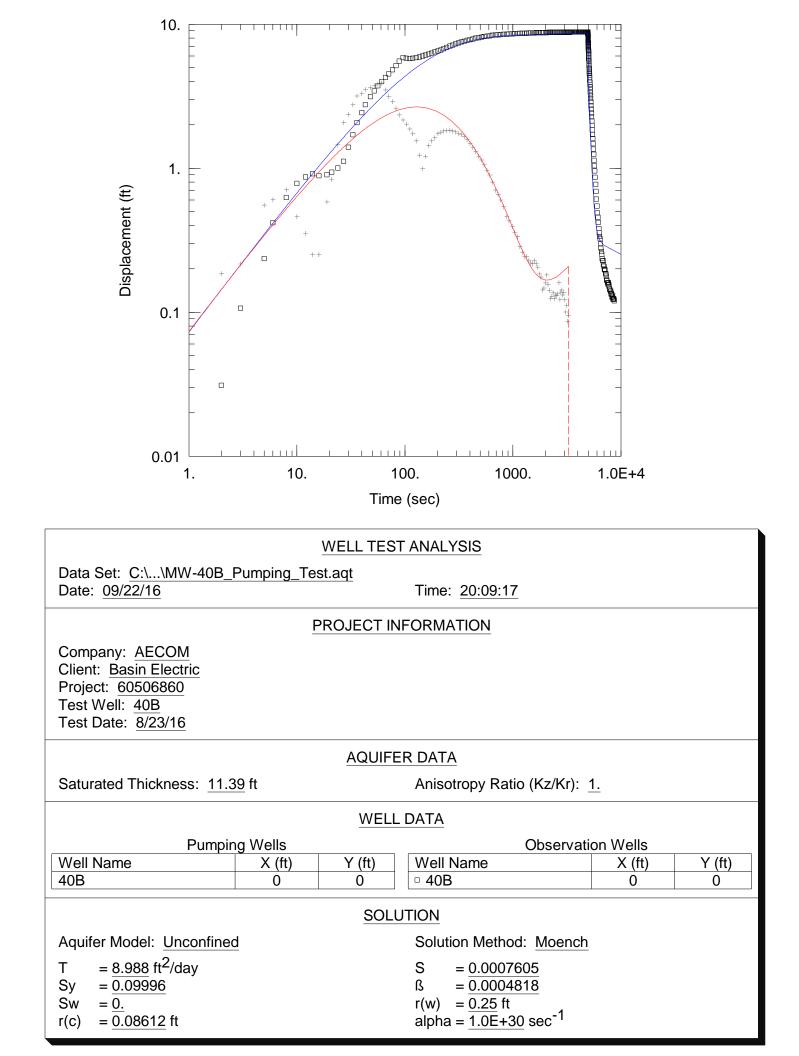
	Time	Elapsed Time (min)	(ft) / tonser	Time	Elapsed Time (min)	Water Depth (ft)
	1317	Ø	76.00			
	1318	sfart tot	NM			
	1327	9	Ngal / Tom			
	1335	17	20 gal / 7.97			
	1344	26	30gal /7.59			
	1353	- 35	40gal 7.44			
12 gpm	1401	43	50g1/7-39			
0.	HIO	52	60ga1 17.36			
	1419	61	70901 734			
	1427	69	806al /7.32			
	1435	77	90gal, 17.32			
	1444	86	100 gal/7-24			
	1454	96	110gal 17.25			
	1509	10-1	12122.5 7.28			
1.2gpm pumpare	1511	113	130/731			
	1519	121	140gal/730			
pumpate	1520M		142.5 total gallons			
	1526 PM		77.86" / 14.78'			
	1531 PM		76.87 / 16:27			
	1536 PM		76.41 / 16,52			
	1541 PM		76,23 / 16.83			
	1546 PM	stopped test	76.16/16.92			

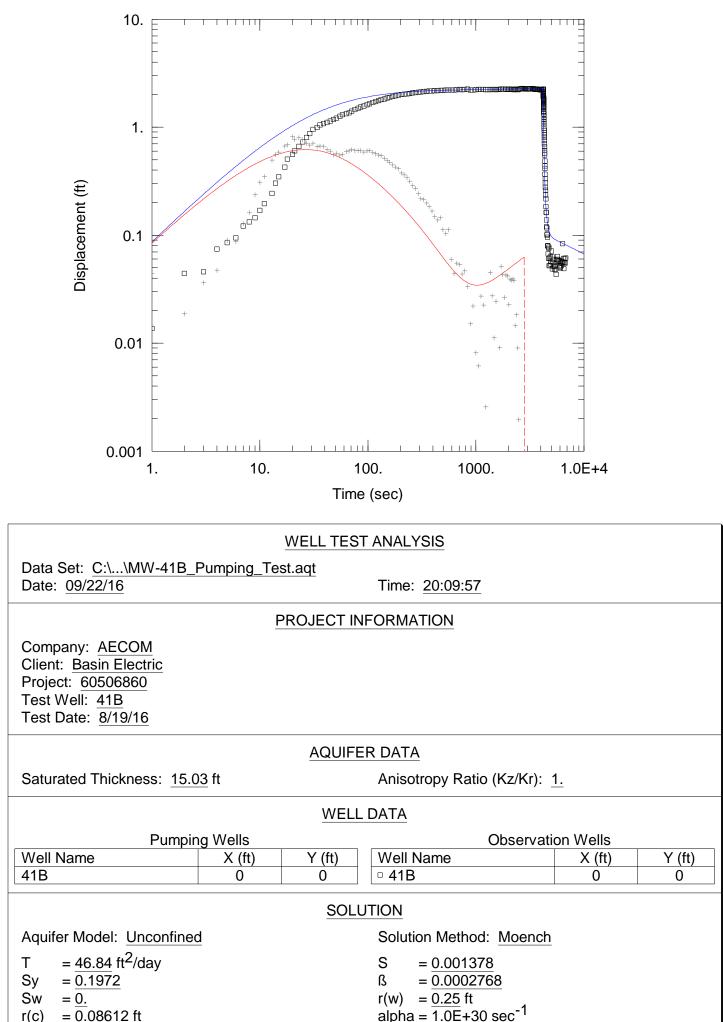
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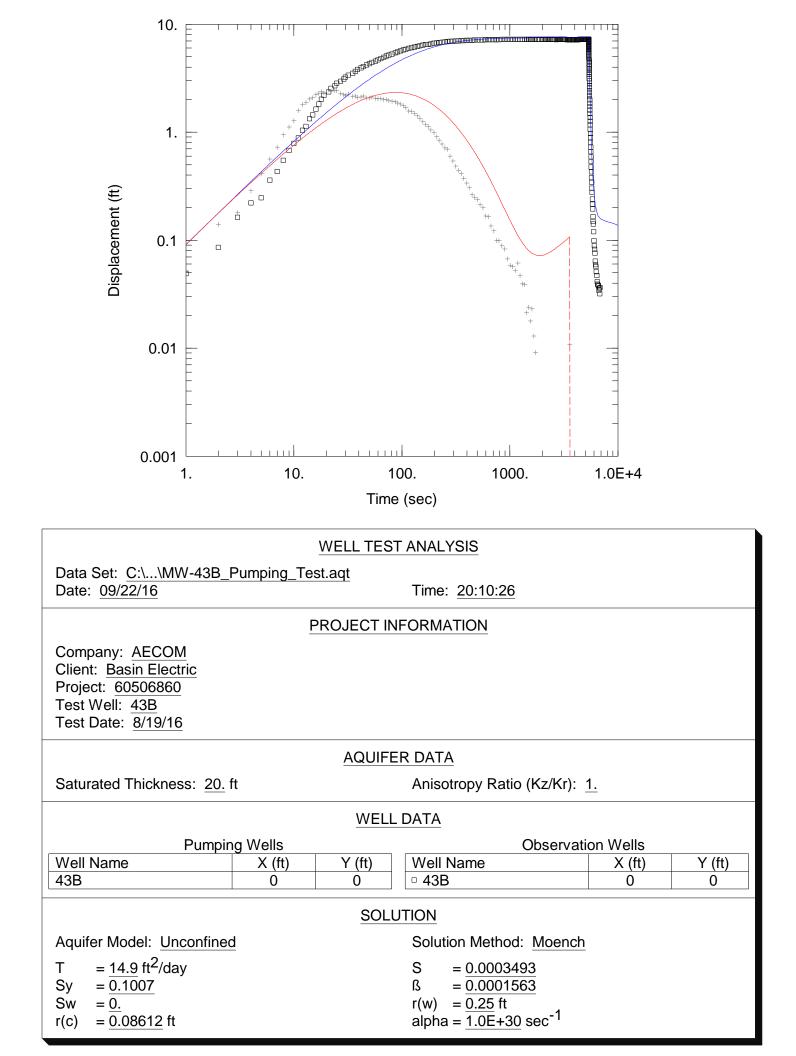


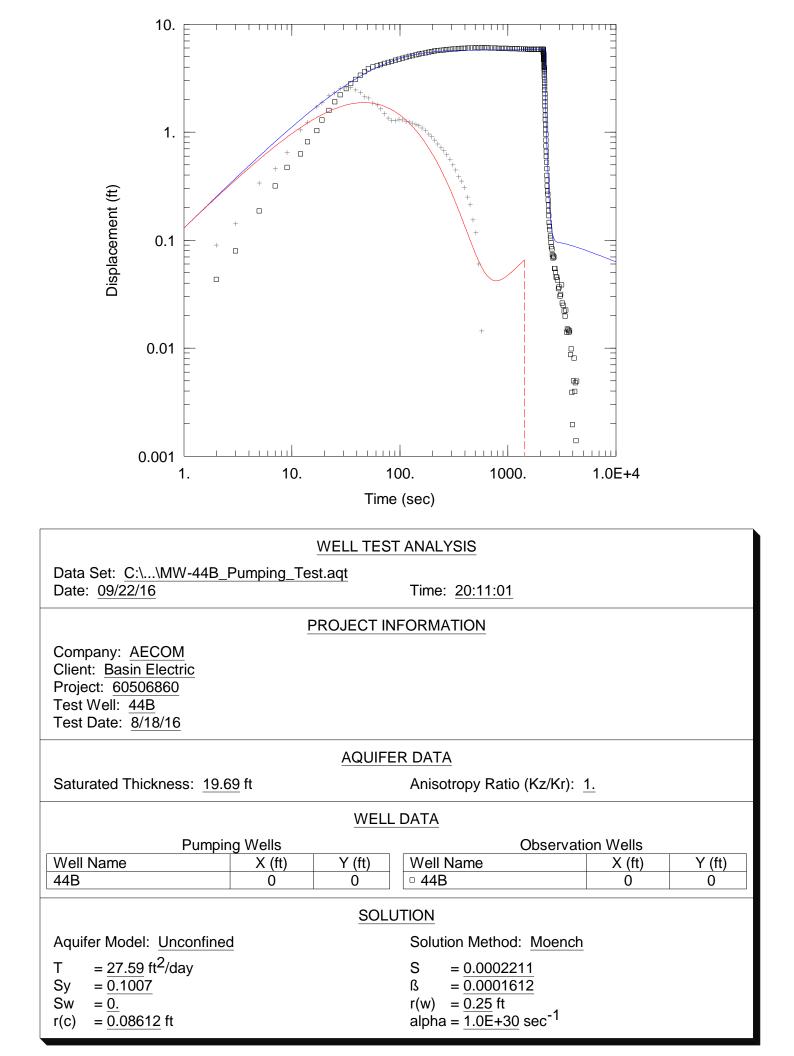


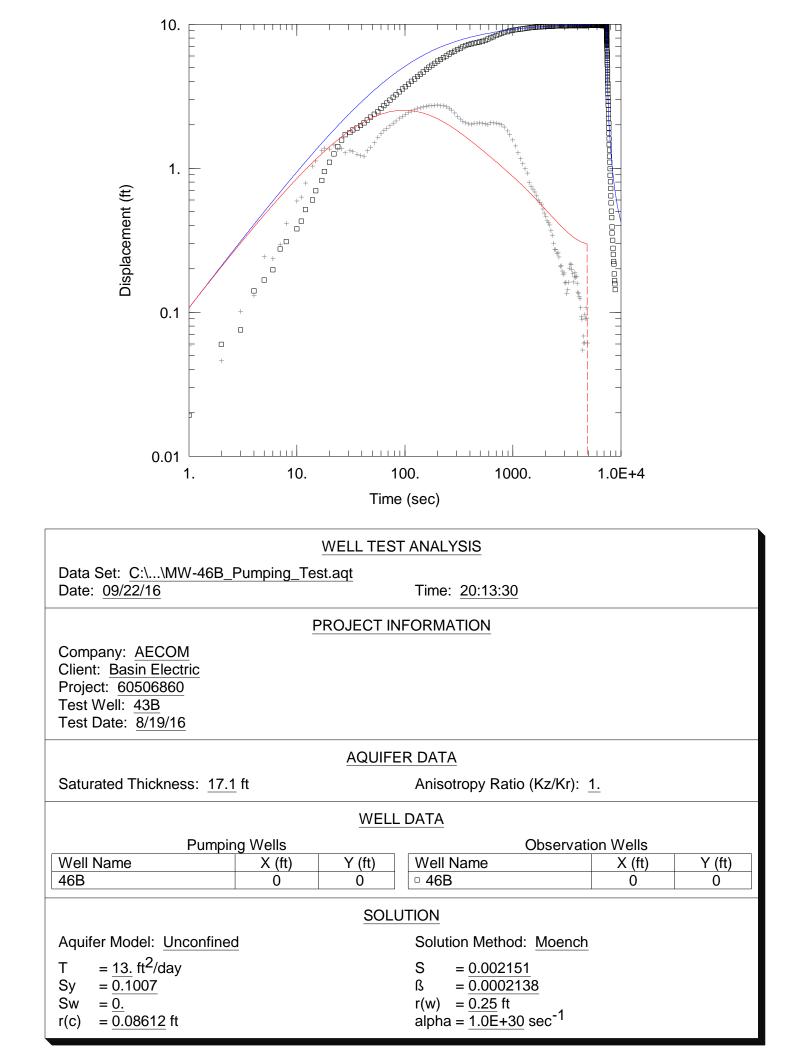




alpha = 1.0E + 30 sec⁻¹







Aquifer Pumping Tests

1.0	EQU	IPMEN	VT AND MATERIALS	
2.0	PUM	IPING T	TEST METHOD	
	2.1	PUMI	PING TEST PROCEDURES	
		2.1.1	Pre-Test Data Recording	
		2.1.2	_	
		2.1.3	General Setup	
		2.1.4	Conducting a Step-Discharge Test	
			Conducting a Constant-Rate Test	
			Conducting a Recovery Test	
	2.2		PING TEST DATA ANALYSIS	
	2.3	REPO	PRTING	
3.0	DOC	CUMEN	TATION	
	3.1	FIELI	O NOTES	
	3.2		D FORMS	
4.0	REF	ERENC	CES	

Attachment

Attachment 18-1	Example of Pumping Test Data Form
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Aquifer Pumping Tests

This Standard Operating Procedure (SOP) provides technical guidance and methods that will be used for performing aquifer pumping tests on groundwater monitoring wells. Pumping tests are conducted to determine aquifer transmissivity, hydraulic conductivity, storativity (or specific yield), anisotropy, and assess aquifer hydraulic connectedness. This SOP provides procedures for a step-discharge test to estimate the optimum pumping rate for the extraction well(s); a constant-rate pumping test (conducted at the rate selected from the step-discharge test); and a recovery test. The pumping tests will be run consecutively to reuse the test/monitoring equipment.

This SOP will provide descriptions of equipment, field procedures and documentation necessary to estimate the above hydraulic properties from step, constant rate, and recovery tests.

All activities will be conducted in accordance with the site-specific Health and Safety Plan (HASP).

1.0 EQUIPMENT AND MATERIALS

General equipment and materials used when performing pumping tests include:

- Boring logs, well construction and development records
- Pressure transducers of appropriate range and data logger
- Laptop computer for data logger
- Weather station with thermometer and barometer
- Electric water level meter
- Pumping test data forms (Attachment 18-1)
- Pump capable of pumping variable rates
- Associated pump control box and suspension cable or rope
- Generator or other appropriate power source for the pump
- Appropriately sized polyethylene discharge pipe, ball/gate valves, and check valve
- In-line flow meter with totalizer and flow measurements in gpm range
- Calibrated measuring volume and stopwatch
- Purge water collection system, as needed
- Tool box, hand tools (pliers, screwdrivers, cutting tools, duct tape etc.)
- Keys to well locks
- Decontamination equipment
- Appropriate health and safety equipment as required by the HASP
- Paper towels

Aquifer Pumping Tests

• Field log book

2.0 PUMPING TEST METHOD

An aquifer pumping test is a hydraulic well testing method in which groundwater is removed from an extraction (pumping) well to create a hydraulic stress on a water-bearing geologic unit, followed by monitoring of the changing hydraulic head in the pumping well and nearby observation wells. Pumping tests also commonly include a recovery phase where recharge is monitored in the test wells. Pumping tests are normally used to measure hydraulic conductivity as well as specific yield and other aquifer properties that are beyond the scope of less complicated slug tests.

Pumping tests may be performed on a single extraction well, however, only transmissivity and hydraulic conductivity values can be obtained from a single well test. The addition of one or more observation wells allows for the computation of specific yield or storativity of the aquifer and possibly for determination of anisotropy. Ideal testing conditions for determination of anisotropy include at least two pairs of in-line observation wells oriented perpendicularly to their radial offset from the extraction well. Constant-head or barrier boundaries within or close to the area affected by groundwater pumping can influence the drawdown and recovery observed in an aquifer pumping test, and may need to be addressed in the analysis.

Three types of aquifer tests may be conducted at each well selected for pumping. The three types of tests are step-drawdown, constant-rate, and recovery. Step-drawdown tests are conducted at successively greater discharges for relatively short periods to collect data that will be used to assess aquifer response at various pumping rates. These tests are usually conducted prior to constant-rate tests in order to estimate the maximum sustainable pumping rates.

Constant-rate tests involve pumping a well for a significant length of time at an approximately constant-rate. Constant rates are typically selected based on step-discharge testing results and/or well development information.

Recovery tests involve monitoring the recharge of groundwater to the test wells following the conclusion of the constant rate test. The following procedures will be implemented for conducting the aquifer tests.

2.1 PUMPING TEST PROCEDURES

2.1.1 Pre-Test Data Recording

A Pumping Test Data Form will be completed for each well and each test as described in Section 3.

2.1.2 Instrument Check

The flow meter, transducers, and electronic water level meters will be calibrated or checked to make sure they are working properly before commencement of the aquifer tests. Copies of instrument calibration documents will be filed with the records of the test data. The following checks and calibrations will be performed for pumping test equipment:

- 1) Pressure transducers are rated to specific pressure heads. When selecting pressure transducers, verify pressure head ranges and associated maximum water column heights. Select an appropriate transducer for the range of water level change anticipated in the aquifer test (see operations manual). Record any pertinent information that may have a bearing on test quality.
- 2) The in-line flow meter will be checked on-site using a calibrated volume and stopwatch.
- 3) The accuracy of the transducers will be checked by moving the transducer up and down in the well a known vertical distance and reading the pressure (or feet of water) values recorded at the data logger. The known amount that the transducer is moved up or down should match the value displayed on the data logger. Also, the sign of the value on the data logger will be checked to verify the direction of transducer movement.
- 4) The water level meter will be checked to make sure that there are no lengths of cable cut off, and that the footages are accurate. The probes will be submerged into water to verify that the tone and/or indicator light are functional.

2.1.3 General Setup

- 1) Adequate fuel will be kept on-site to maintain a generator, if used for power, for the duration of the test, and all refuel times will be noted in field notebook.
- 2) The oil level in the generator (if used for power) will be checked periodically (at least twice daily).
- 3) Locate, open, and vent all wells to be tested on that day, unless prohibited by access restrictions. This will allow the water level in the test well to equilibrate with the prevailing barometric pressure. Equilibration of static water levels should be measured with the electronic water level indicator and/or pressure transducer as appropriate. The test wells should equilibrate for at least 30 minutes prior to beginning an aquifer test, and may require more time depending on aquifer characteristics.
- 4) Measure the static water level and total depth of the well to the nearest 0.01 foot with an electronic water level indicator before the test begins. The measuring point shall be the survey point where the surface elevation was measured; otherwise the point of reference will be the rim of the top of casing on the north side of the well. The well must not be recovering or receding as a result of sampling, development, pumping of nearby wells, or related activities. The test wells will be allowed to recover from these activities for a minimum of 24 hours before the start of the aquifer test.
- 5) Install pump and discharge lines in pumping well. The pump will be equipped with a check valve on the discharge line to prevent water in the discharge pipe from reentering the well once pumping ceases. Connect the discharge lines to purge containers, and pump control box to pump cable. Allow the water level in the well equilibrate to static conditions.
- 6) Measure the static water level and total depth in pumping well and observation wells from the surveyed reference point with an electric water level meter.
- 7) Install pressure transducers in pumping well and observation wells at a depth below the maximum drawdown expected during the test. The pumping well transducer may be installed

inside a sounding tube to limit noise in the transducer readings from potential turbulence in the well. If a sounding tube is present, install pressure transducer approximately 6 inches from the bottom of the tube. Transducers are usually installed above the pump. Do not exceed the specified depth range of the transducer. The transducer should be secured so that it does not move during the test.

- 8) The transducers should equilibrate for 5 to 10 minutes before initiating the aquifer test.
- 9) The transducer cable will be connected to the data logger and the data logger turned on. The transducer probe pressure readout (reference level) will be set to zero while the probe is in the water. The depth interval from the static water level will be compared to the transducer probe readout on the data logger to verify that the transducer probe is working properly. The probe may then be referenced to the "appropriate datum" within the data logger. The appropriate datum may be the water level elevation as referenced to mean sea level or the depth of groundwater below the monitoring point.
- 10) A pre-run checkout test will be performed as specified in Section 2.1.2.
- 11) Care must be taken to ensure that the elevation of the transducer does not change once the test has begun. Readings from the transducer may be utilized to determine when the test should be stopped.
- 12) All water generated during the test shall be properly containerized or otherwise disposed of.
- 13) At the conclusion of any test, be sure to "stop", "save", and "download" all data from the transducers and/or data logger prior to removing a transducer from the well.
- 14) Remove the transducer and decontaminate all equipment.
- 15) Aquifer test data acquired from wells will be downloaded from the data logger onto a computer and backup copies created.

2.1.4 Conducting a Step-Discharge Test

A series of 2-hour step-discharge tests may be conducted at each selected test well at pre-selected rates based on well development records. The purpose of these tests is to estimate the optimum sustainable pumping rate for the constant-rate test, and to assess how specific capacity varies with increasing pumping rates. The step-discharge test will involve pumping the test well up to 4 successively increasing discharge rates. Each pumping rate (step) will continue for at least 2 hours, or until water levels generally stabilize.

- 1) Pumping rates for each step may be adjusted in the field.
- 2) Water-level data from select observation wells and the pumping well will be collected continuously on a logarithmic time schedule using a data logger. Water levels will be measured according to the following time schedule for each step:
 - a) 0-10 minutes (min): 1 second (sec) intervals
 - b) 10-15 min: 10 sec intervals
 - c) 15-100 min: 2 min intervals

- d) 100-120 min: 5 min intervals
- e) >120 min: 10 min intervals
- 3) The start time of the data logger will be synchronized with that of the pump. This can easily be done with hand signals or with the delayed start feature on the data logger. Ensure that the pump does not start before the data logger so that the initial water level, H_o, is recorded.
- 4) Start the step test, recording the time the pump is started as test "zero time."
- 5) Monitor the pumping well discharge rate and maintain a constant flow rate by regulating the valve. Monitor the pumping rate approximately every 15 minutes during the step test. Record data on the Aquifer Test Data Forms, or in the field notebook.
- 6) Water levels in the pumping well and observation wells will be measured and recorded to back up the electronic data collected by the pressure transducers and data logger. Manual water-level measurements of the pumping well will be made at 5-minute intervals during the first hour, at 15 minute intervals through the remainder of the test. Measurements at the observation wells may be made every 30 minutes. Measurements will be recorded on the Pumping Test Data Forms.
- 7) At the end of a 2-hour interval (or sooner, if equilibrium conditions are reached early), the pump will be advanced to the next higher rate and the next step will begin. The water level measurement schedule will start over from time = 0.
- 8) The data logger will be downloaded to a laptop computer after step- testing is completed.

2.1.5 Conducting a Constant-Rate Test

A constant-rate test will be conducted to estimate aquifer parameters. The constant-rate test will begin only after the aquifer has recovered to within 95% of pre-step test static conditions. Water levels will be measured at least 3 times, approximately 10-15 minutes apart, to verify that static conditions have been re-established.

The constant-rate test will involve pumping the aquifer at a constant discharge rate for a specified duration and measuring water level drawdown. The pumping rate at which the constant-rate test is conducted will be determined from the results of step-discharge test or from previous site knowledge. Barometric pressure will be recorded several times daily to document changes that may influence groundwater elevations. A detailed list of activities to be performed during the constant-rate test follows:

- 1) Prior to starting the constant-rate test, static water level will be measured in the observation wells and in the test well (to nearest 0.01 foot). Measurements will be made from a surveyed reference point on the well.
- 2) The pumping well and observation wells located within 100 feet of the pumping well will be monitored with a data logger. The data logger will be programmed to record data logarithmically from the test well and observation wells in which transducers have been placed on the following schedule:
 - a) 0-10 min: 1 sec intervals
 - b) 10-15 min: 10 sec intervals

- c) 15-100 min: 2 min intervals
- d) 100-1,000 min: 30 min intervals
- e) 1,000-10,000 min: 200 min intervals
- 3) Water levels may be monitored manually with an electric water level meter at observation wells located within 300 feet of the pumping well at the following approximate intervals:
 - a) 0-10 min: 1 min intervals
 - b) 10-100 min: 10 min intervals
 - c) 100-1,000 min: 100 min intervals
 - d) >1,000 min: 1,000 min intervals

Observation wells greater than 300 feet from the test well may be monitored manually less frequently.

- 4) The start time of the data logger will be synchronized with that of the pump. This can easily be done with hand signals or with the delayed start feature on the data logger. Ensure that the pump does not start before the data logger so that the initial water level, H_o, is recorded.
- 5) Start the constant-rate test, recording the time the pump is started as test "zero time."
- 6) If the initial discharge rate exceeds the predetermined discharge rate, reduce flow by partially closing the valve on the discharge pipe and note the time in the field notebook.
- 7) Monitor the pumping well discharge rate and maintain a constant flow rate by regulating the valve. Monitor the pumping rate every 10 minutes during the first two hours. It is recommended to then monitor the pumping rate at 30-minute to 1-hour intervals, as appropriate, throughout the remainder of the test. Record data on the Pumping Test Data Forms, or in the field notebook.
- 8) During pumping, plot the data (time versus drawdown) on log-log and/or semi-log graph paper or with computer software to assess the progress of the test and to determine when sufficient data have been collected.
- 9) Water levels in the pumping well will be measured and recorded to back up the electronic data collected by the pressure transducers and data logger. Manual water-level measurements in the pumping well are recommended at approximately 5-minute intervals during the first hour, at 15 minute intervals from 1 to 4 hours, and at 1 hour intervals through the remainder of the test. Measurements will be recorded on the Aquifer Test Data Forms.
- 10) Manual water-level measurements in the observation wells are recommended at 15 minute intervals during the first 4 hours, and then at 2 hour intervals through the conclusion of the test. Measurements will be recorded on the Pumping Test Data Forms.
- 11) Samples of groundwater may be collected from the pumping well during the test.
- 12) The data logger will be downloaded to a laptop computer after the constant-rate test is completed.

Aquifer Pumping Tests

There is generally no need to continue a test if water levels have sufficiently stabilized. This normally indicates that sufficient data have been collected. Additional useful information generally will not be gained by continued pumping. When the time versus drawdown data for the most distant observation well begins to plot as a straight line (constant slope) on the semi-log graph paper, the test can be terminated unless delayed yield conditions are anticipated.

Delayed yield conditions may be expected in unconfined aquifers. Pumping tests in unconfined aquifers should be continued until the effects of delayed yield are no longer present and a second Theis-type drawdown begins, if practical.

2.1.6 Conducting a Recovery Test

When the constant-rate test is terminated, the data logger cycle will be terminated and started again to record recovery data. The data logger will be programmed to collect recovery data in a logarithmic mode at the same intervals as those used for the constant-rate test. The start of the data recording will be timed precisely to the shutdown of the pump. The pump will be equipped with a check valve on the discharge line to prevent water in the discharge pipe from reentering the well once pumping ceases.

The recovery test will be terminated when water levels in the observation wells have recovered to within 90% of pre-test static levels or a specified duration. Recorded data will be downloaded from the data logger to a computer disk with file names that reflect the well name and test type (step-discharge, constant-rate, or recovery). Backup disks will also be created for contingency purposes.

2.2 PUMPING TEST DATA ANALYSIS

Data analyses and interpretations from the aquifer tests will be included in the investigation report. Drawdown and recovery data will be compiled and analyzed to:

- Determine hydraulic conductivity, transmissivity, and specific yield or storativity
- Estimate the radius of influence
- Assess whether any hydrogeologic boundaries were encountered (i.e., barrier or recharge boundaries)
- Assess whether any hydraulic communication between aquifer units exists
- Determine the nature and extent of aquifer anisotropy, if appropriate

All analyses will be performed using AQTESOLV[®] for Windows software (Duffield, 2007), Microsoft Excel[®], or similar software. The aquifer test data will be analyzed using the appropriate analytical method(s). Methods may include, but are not limited to, Theis (1935) and Cooper-Jacob (1946). If the hydrogeologic conditions and pumping test data satisfy more than one method of analysis, then results will be presented for each method used.

If hydrogeologic conditions at the site prove to be more complicated than is appropriate for standard modeling methods, a more detailed numerical modeling approach may be undertaken. All numerical modeling results should contain an adequate description of the method or methods utilized.

2.3 REPORTING

Aquifer test data analyses and interpretations will be presented in the investigation report. At a minimum, this portion of the report will include:

- A description of the procedures implemented during testing
- Interpretations of pumping test data
- Tables containing well completion information (e.g., well elevations and screened intervals) and water level data (e.g., initial and final pumping water levels)
- Tables summarizing estimated aquifer property values and water quality parameters collected during the pumping tests
- AQTESOLV[®] reports and graphs, as well as any manually produced graphs and calculations

3.0 DOCUMENTATION

Documentation of the observations and data acquired in the field will provide information on the activities conducted and also provide a permanent record of field activities. Observations and data will be recorded on a Pumping Test Data Form (Attachment 18-1) and in the field logbook.

3.1 FIELD NOTES

The following aquifer test information will be recorded in a bound field logbook using indelible ink:

- Names of test personnel
- Weather conditions (including barometric pressure)
- Date and time of testing
- Test locations, specifying pumping wells and observation wells
- Start and stop time for each test or step conducted
- Equipment used
- Any other pertinent information that may have a bearing on test quality

3.2 FIELD FORMS

A Pumping Test Data Form (Attachment 18-1) will be completed for each well and each test. The following information will be recorded:

- Date of test
- Aquifer test personnel
- Pumping/extraction or observation well identification number
- Location and elevation (if known) of the reference point from which water depth measurements are made (i.e., top of PVC well casing) for each well

- Static water level
- Well depths, screened intervals, well casing diameters, borehole diameters, and filter pack intervals (from well construction logs)
- Aquifer or groundwater zone (lithology) being tested (from well construction logs)
- Start time of test or step
- End time of test or step
- Type of test (step test, constant-rate, or recovery). If a step test is run, specify which step in the series.
- Pumping rate
- Data logger test number
- Manual water level readings and associated times
- Data collected during the test will not be hand copied from the data logger, but will be downloaded onto a computer and backup copies created

4.0 REFERENCES

Cooper, H.H. and C.E. Jacob, 1946. A generalized graphical method for evaluating formation constants and summarizing well field history, Am. Geophys. Union Trans., vol. 27, pp. 526-534.

Duffield, Glenn M. 1996. AQTESOLV for Windows™, User's Guide. HydroSOLVE, Inc.

Theis, C.V., 1935. The relation between the lowering of the piezometric surface and the rate and duration of discharge of a well using groundwater storage, Am. Geophys. Union Trans., vol. 16, pp. 519-524.

Aquifer Pumping Tests

Attachment 18-1 Example of Pumping Test Data Form

PUMPING TEST DATA FORM

Well ID	Personnel	
Location	Static Water Level	
Type of Well	Extraction Well Distance	
Test Date	Total Casing Depth	
Measuring Point Elevation	 Borehole Diameter	
Type of Test	Casing Diameter	
Step Number	 Screened Interval	
Data logger Test Run No.	 Sand Pack Interval	
Pumping Rate	Lithology Tested	
Test Start Time	Test End Time	

Time	Elapsed Time (min)	Water Depth (ft)	Time	Elapsed Time (min)	Water Depth (ft)

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